

GENERAL NOTES

1.1	Fabrication shall be in accordance with A.S.C. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".		
1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	Hot Rolled Steel Shapes (W, & C)	A572	Fy = 50 KSI
	Hot Rolled Steel Angles (L)	A36	Fy = 36 KSI
	Steel Pipes	A500	Fy = 42 KSI
	Structural Tubing	A500	Fy = 42 KSI
	Structural Steel Web Plate	A572/A1011	Fy = 50 KSI
	Structural Steel Flange Plates/Bars	A529/A572	Fy = 55 KSI
	Cold Formed Light Gage	A653/A1011	Fy = 55 KSI
	Roof and Wall Sheets	A792/A653	Fy = 50, 80 KSI
	Cable Brace	A475 - TYPE 1	Extra High Strength
	Rod Brace	A529	Fy = 50 KSI
		MIN. TENSILE STRENGTH	
	Machine Bolts & Nuts	A307	Fu = 60 KSI
	High Strength Bolts (1"Ø and less)	A325-TYPE 1	Fu = 120 KSI
	High Strength Bolts (>1"Ø to 1 1/2"Ø)	A325-TYPE 1	Fu = 105 KSI
	Anchor Bolts (Not supplied by A.S.C.)	A36/A307/F1554	Fu = 60 KSI

1.3 **PRIMER**
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is A.S.C. Gray Oxide color. This paint is not intended for long term exposure to the elements. A.S.C. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. A.S.C. shall not be responsible for any field applied paint and/or coatings. (AISC Code of Standard Practice, Latest Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.

1.4 **GALVANIZED OR SPECIAL COATINGS:**
See Contract Documents

1.5 **ALL BOLTS ARE 1/2"Ø x 0'-1 1/4" A307 EXCEPT :**
a) Endwall rafter splice - 5/8"Ø x 0'-1 3/4" A325-N
b) Endwall column to rafter connection - 1/2"Ø x 0'-1 1/4" A325 MIN.(SEE WALL ELEVATION)
c) Main frame connections - SEE CROSS SECTION
d) Flange Broce connections - 1/2"Ø x 0'-1 1/4" A325

NOTE: Washers are not supplied unless noted otherwise on drawing

1.6 **A325 BOLT TIGHTENING REQUIREMENTS**
All high strength bolts are A325-N unless specifically noted otherwise. Holes are not slotted and design is bearing connection. Structural bolts shall be tightened by the turn-of-the-nut method in accordance with the Latest Edition AISC "Specification For Structural Joints " using ASTM A325 or A490 Bolts, when specifically required. A325-N bolts are supplied without washer unless otherwise noted on the drawings.
All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.

1.7 **CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:**
INSIDE- Under roof panels & base of wall panels
OUTSIDE - Between roof panels & ridge cap
- Between wall panels & eave/gable trim

1.8 **ERECTION NOTE:**
All bracing, strapping, & bridging shown and provided by A.S.C. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.9 **ERECTION AND UNLOADING NOT BY A.S.C.**

1.10 **SHORTAGES**
Any claims or shortages by buyer must be made to A.S.C. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 **CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)**
Claims for correction of alleged misfits will be disallowed unless A.S.C. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of A.S.C.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- 2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release A.S.C. to fabricate upon receiving such.
- 2.2 Armstrong Steel Corp (hereafter referred to as A.S.C.) standard specifications apply unless stipulated otherwise in the Contract Documents. A.S.C. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.
- 2.3 In case of discrepancies between A.S.C. structural steel plans and plans for other trades, A.S.C. plans shall govern. (Section. 3 AISC Code of Standard Practices, Latest Edition)
- 2.4 Approval of A.S.C. drawings and calculations indicates that A.S.C. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the A.S.C. design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)
- 2.5 Once the BUYER/END USE CUSTOMER has signed A.S.C. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.
- 2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by A.S.C. and A.S.C. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or A.S.C. assumptions will govern (AISC Code of Standard Practice, Latest Edition)



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JOB NO. : 57243

CUSTOMER :

END USER :

END USE :

LOCATION :

_____ :

_____ :

PH. NO. :

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

Design Code / Wind Code	: IBC 18 / IBC 18
Building Risk Category	: II - Normal
Enclosure	: Closed
Dead Load (psf)	: 2.65
Collateral Load (psf)	: 6.00
Wind Load	
Ultimate Wind Speed, (Vult) (mph)	: 138.0
Wind Exposure	: C
Internal Pressure Coefficient, GCpi	: 0.18 /-0.18
Wall Panel Design Wind Pressure (psf)	: 44.7 /-48.5
Live Load	
Primary Framing (psf)	: 20.00
Trib. Area Reduction	: No
Secondary Framing (psf)	: 20.00
Snow Load	
Ground Snow Load, Pg (psf)	: 5.00
Roof Snow Load, Pf (psf)	: 5.00
Sloped Roof Snow Load, Ps (psf)	: 5.00
Snow Exposure Factor, Ce	: 1.0000
Snow Importance Factor, Is	: 1.00
Thermal Factor, Ct	: 1.20
Sloped Factor, Cs	: 0.860

Seismic Load

Seismic Importance Factor, Ie	: 1.00
Site Class	: D
Mapped Spectral Response Acceleration	: Ss = 1.067 :S1 = 0.333
Spectral Response Coefficients	: Sds = 0.763 :Sd1 = 0.437
Seismic Design Category	: D
Basic Force Resisting Systems Used	: Steel System Not Specifically Detailed For Resistance
	: Rigid Frames (OMF)
	: Braced Frames (OCBF/OMF)
Total Design Base Shear, V (kips)	: Longitudinal = 7.44
	: Transverse = 8.28

Response Modification Factors, R	: Rigid Frames = 3.00	Ω = 3.00
	: SW X-Bracing = 3.00	Ω = 2.00
	: SW Wind Bent = 3.00	Ω = 3.00

Seismic Response Coefficient, Cs	: Rigid Frames = 0.2546
	: SW X-Bracing = 0.2546
	: SW Wind Bent = 0.2546

Analysis Procedure Used : Equivalent Lateral Force Procedure
Other Loads/Requirements

BUILDING DESCRIPTION:

Width (ft)	: 40
Length (ft)	: 48
Eave Ht. at BSW (ft)	: 18
Eave Ht. at FSW (ft)	: 18
Roof Slope at BSW	: 5.0:12
Roof Slope at FSW	: 5.0:12
Bay Spacing (ft)	: 2 at 24

COVERING AND TRIMS:

Roof Panels & Trims

Panel Type	: 26 Ga. R-Loc
Panel Color	: Galvalume Plus
Trim Colors	
Gable/Eave Trim	: Burnished Slate 40 yr
Eave Gutter	: Burnished Slate 40 yr

Wall Panel & Trims

Panel Type	: 26 Ga. R-Loc
Panel Color	: Charcoal 40 yr
Trim Colors	
Corner Trims	: Burnished Slate 40 yr
Opening Trims	: Burnished Slate 40 yr
Downspouts	: Charcoal 40 yr
Base Trim	: Charcoal 40 yr

Drawing Index

Drawing Name	Page(s)
Drawing Cover	COVER
3D Reference	3D REF
Anchor Bolt Plan	1
Anchor Bolt Details	2
Anchor Bolt Reactions	3
Rigid Frame	4,5
Front Sidewall	6
Back Sidewall	7
Left Endwall	8
Right Endwall	9
Roof Plan	10
Details	11-14

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT ARMSTRONG STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY A.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ARMSTRONG ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

BUYER/END USE CUSTOMER RESPONSIBILITIES CONTINUED

- 2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that A.S.C. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that A.S.C. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by A.S.C.
- 2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with A.S.C. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (AISC Code of Standard Practice, Latest Edition.)
- 2.9 Armstrong Steel Corp is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete. Unless otherwise provided in the Order Documents, A.S.C. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site. It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Latest MBMA Low Rise Building Systems Manual)
- 2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to A.S.C. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (AISC Code of Standard Practice Latest Edition)
- 2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (AISC Code of Standard Practice Latest Edition)
- 2.12 **WARNING** In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto Galvalume should be avoided.
- 2.13 **SAFETY COMMITMENT** Armstrong Steel Corp has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of A.S.C. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.
- 2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.
- 2.15 It is recommended by Factory Mutual (Reference: B2.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.

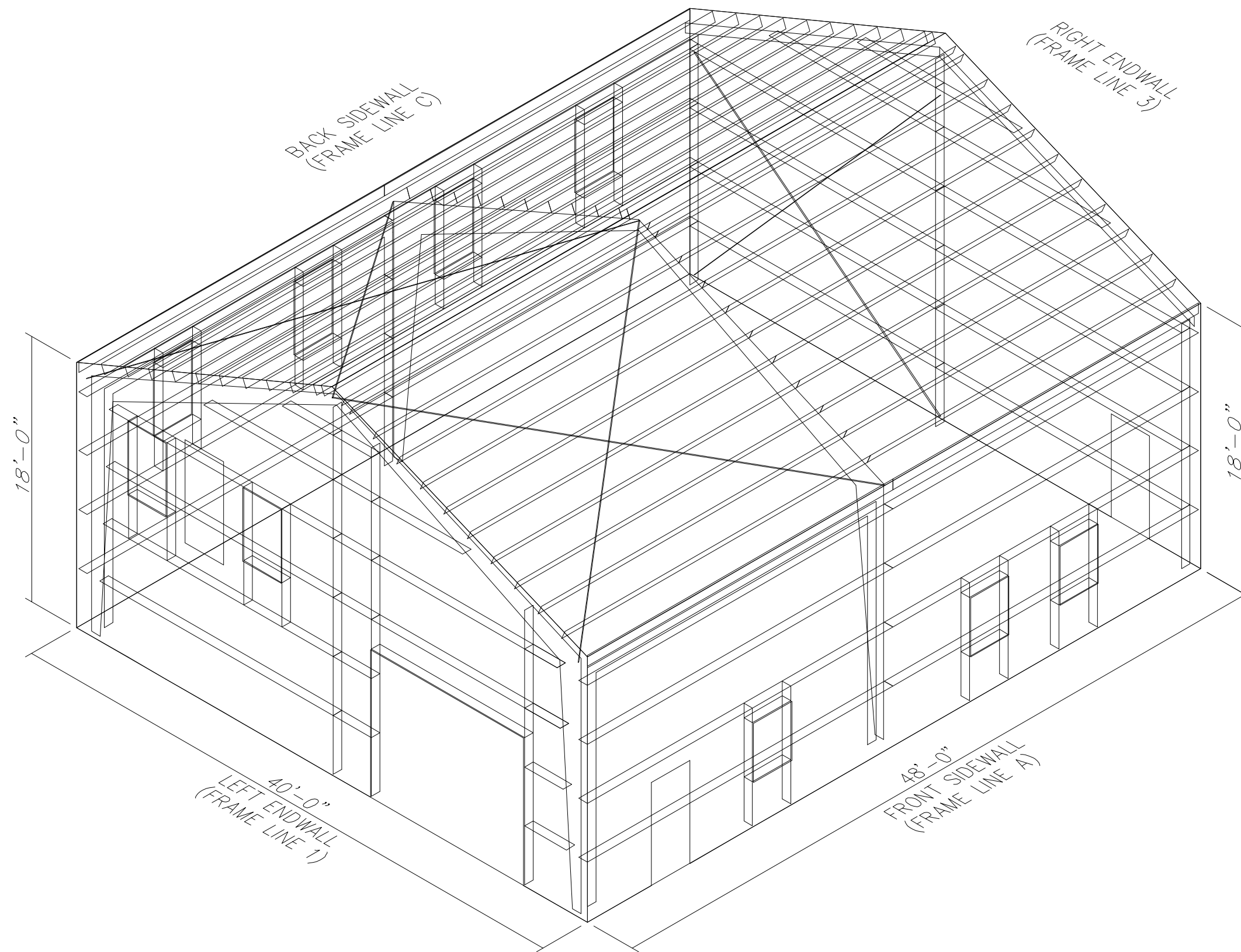
ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

NOTE:
For Snow/Ice Removal Procedure, Refer to Metal Building System Manual 2002 Edition, Section A8.4, Page XI-A8-2.

Drawing Status


- APPROVAL:** **REVISED APPROVAL:**
These drawings, being for approval, are by definition not final, and are for conceptual representation only. their purpose is to confirm proper interpretation of the project documents. Only drawings issued "Construction" can be considered as complete.
- PERMIT:** **REVISED PERMIT:**
These drawings, being for permit, are by definition not final. Only drawings issued "Construction" can be considered as complete.
- CONSTRUCTION:**
Final drawings to be used in the erection of the building.

JOB NO : 57243



NOTE:
 3D IS A GENERAL REPRESENTATION OF BUILDING.
 SOME MEMBERS MAY CHANGE IN FINAL ERECTION DRAWINGS

ELEVATION	PAGE
FRONT SIDEWALL (FRAME LINE A)	6 OF 14
BACK SIDEWALL (FRAME LINE C)	7 OF 14
LEFT ENDWALL (FRAME LINE 1)	8 OF 14
RIGHT ENDWALL (FRAME LINE 3)	9 OF 14

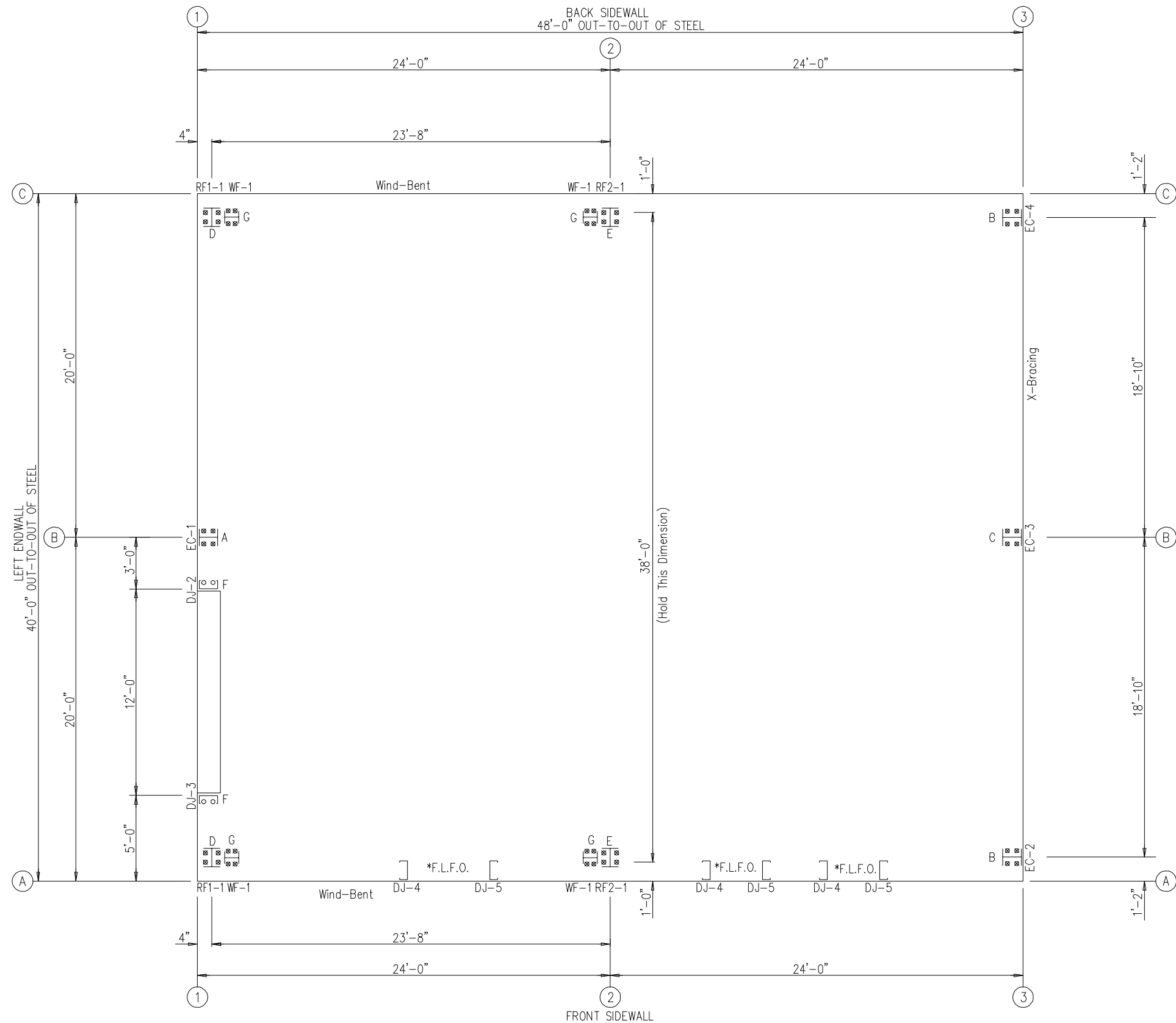


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DESCRIPTION	ISOMETRIC VIEW
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57243	ENG. BY: MZ
DWG. NO.: 3D REFERENCE	DATE: 11/19/21
	ISSUE: C

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
4	Jamb	5/8"	A307
16	Endwall	3/4"	A307
16	Frame	3/4"	A307
16	WindCol	3/4"	A307



ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)

*Field Located Framed Opening

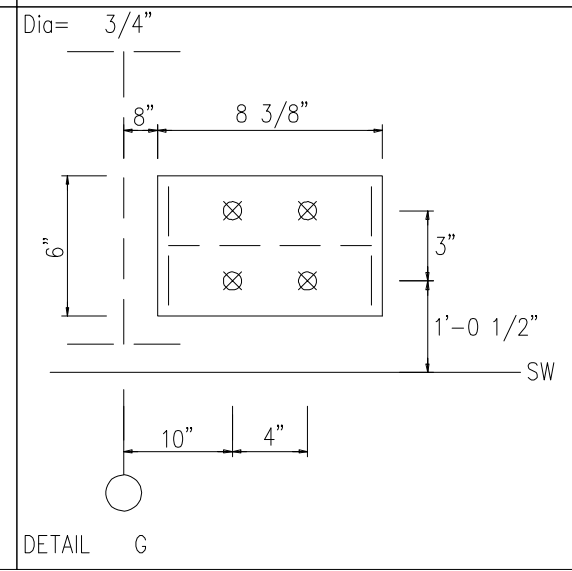
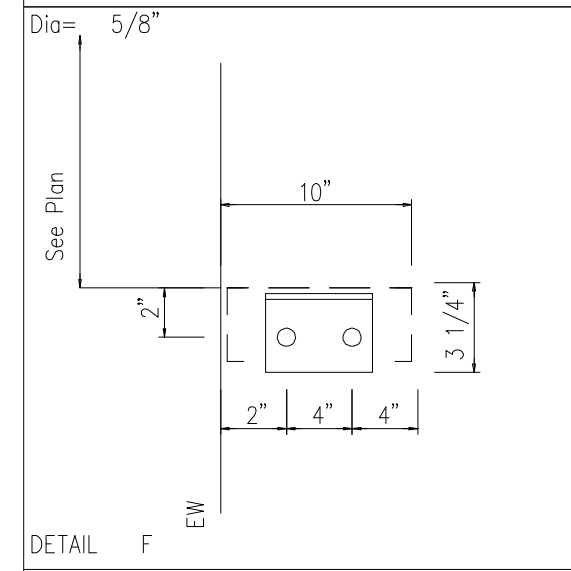
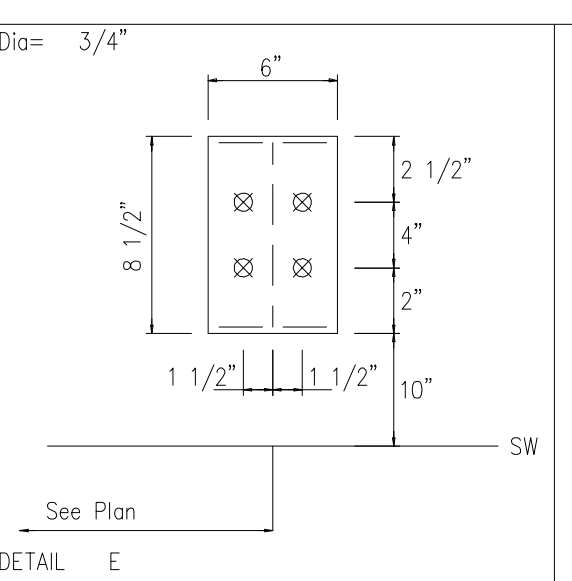
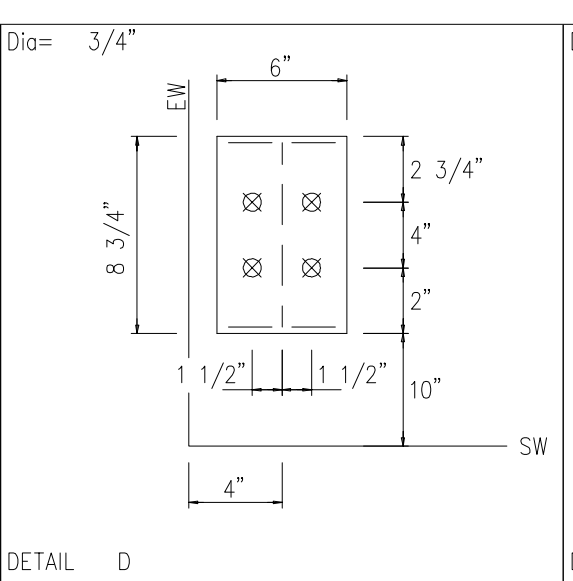
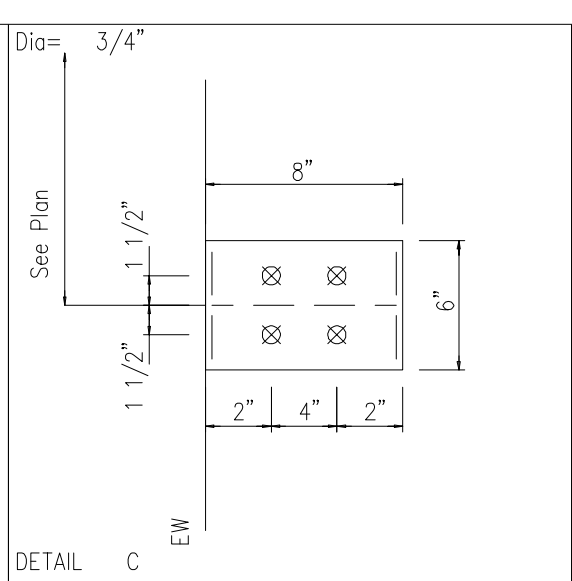
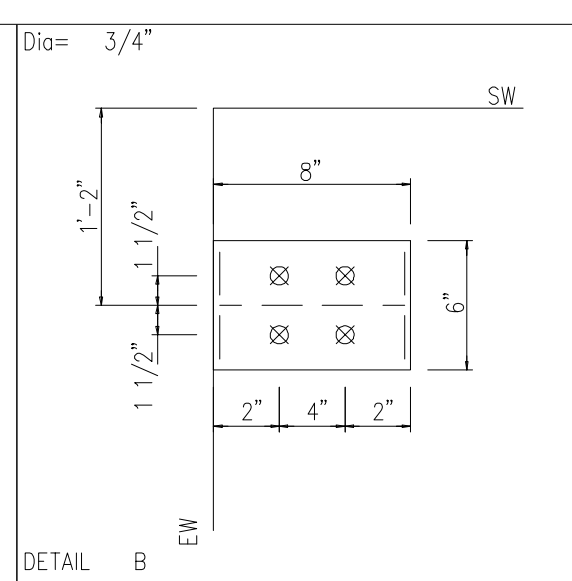
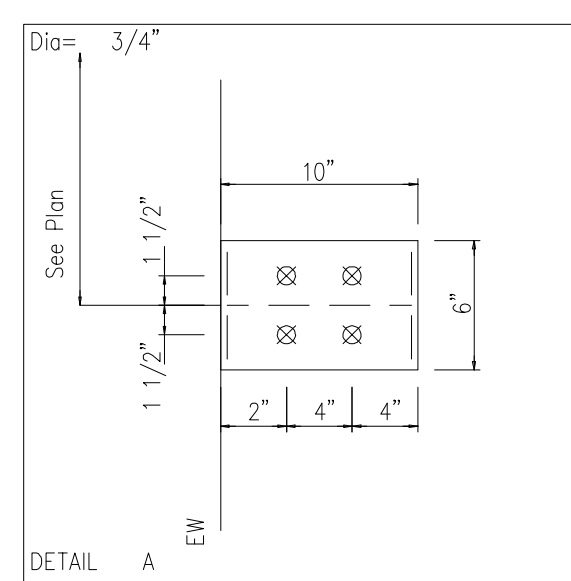
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ



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DESCRIPTION	ANCHOR BOLT PLAN		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	ENG. BY:	DATE:	
57243	MZ	11/19/21	
DWG. NO.:	1 OF 14	ISSUE:	C

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.



NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

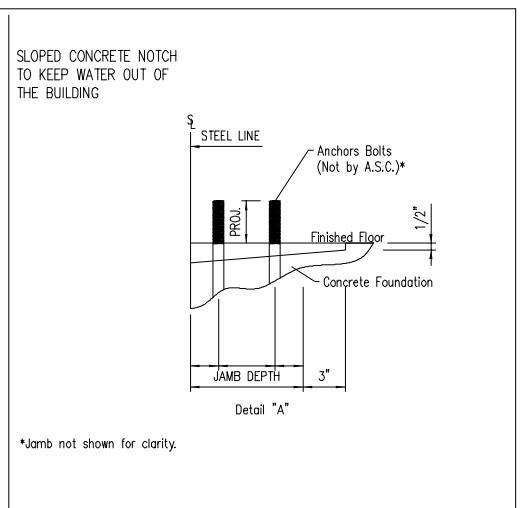
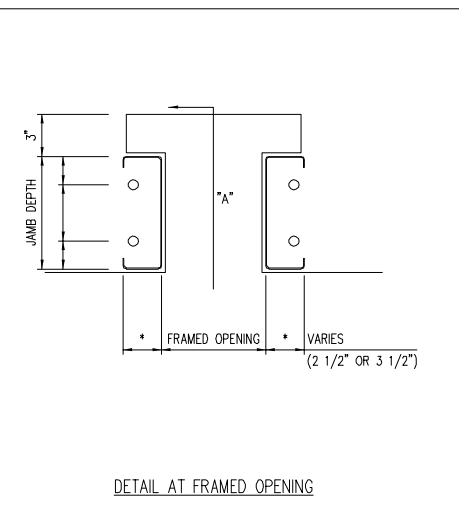
ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING MANUFACTURER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

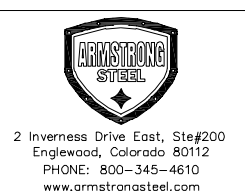
NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.

Anchor Bolt Diameter	Projection
1/2"	1 1/2"
5/8"	2"
3/4"	2 1/2"
7/8"	3 1/2"
1"	3 1/2"
1 1/8"	3 1/2"
1 1/4"	3 1/2"

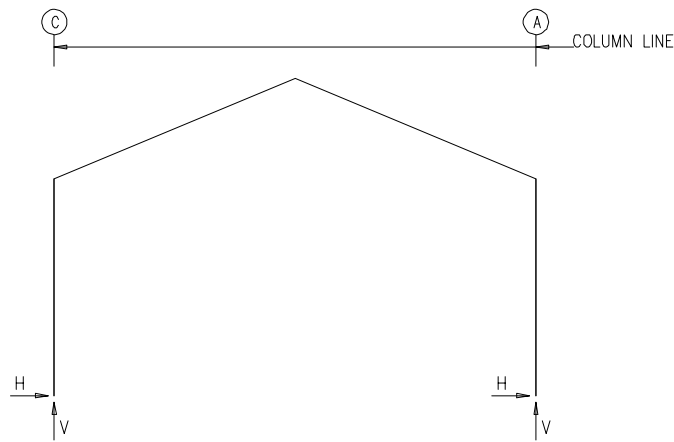
ANCHOR BOLT PROJECTION



ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ



DESCRIPTION	ANCHOR BOLT DETAILS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57243	ENG. BY: MZ
DWG. NO.: 2 OF 14	DATE: 11/19/21
	ISSUE: C



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	C	3	3.6	4.5	6	-4.2	-3.0	4	0.750	6.000	8.750	0.500	0.0
		1	2.4	8.0	4	-3.7	-5.1						
1	A	7	4.2	-3.0	2	-3.6	4.5	4	0.750	6.000	8.750	0.500	0.0
		1	-2.4	8.0	5	3.7	-5.1						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2	C	3	8.1	10.7	6	-8.3	-5.2	4	0.750	6.000	8.500	0.500	0.0
		1	5.9	18.1	4	-7.2	-10.1						
2	A	7	8.3	-5.2	2	-8.1	10.7	4	0.750	6.000	8.500	0.500	0.0
		1	-5.9	18.1	5	7.2	-10.1						

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind Press		Live Vert	Snow	Wind_Left1		Wind_Right1		Wind_Left2	Wind_Right2	Wind Press
			Horz	Vert			Horz	Vert	Horz	Vert			
1	B	0.3	-8.9	9.9									
3	A	0.5	0.6	1.9	0.5	0.0	-2.5	0.0	-4.3	0.0	-0.3	0.0	-3.1
3	B	1.5	1.9	5.9	1.5	-4.8	-15.5	0.0	-1.1	-4.8	-13.2	0.0	-8.5
3	C	0.5	0.6	1.9	0.5	0.0	3.9	4.8	-8.7	0.0	6.0	4.8	-3.1

Frm Line	Col Line	Wind Suct Horz	Wind_Long1		Wind_Long2		Seis_Left		Seis_Right		E2UNB_SL_L-		E2UNB_SL_R-	
			Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
3	A	3.6	0.0	-5.9	0.0	-3.5	0.0	0.3	0.0	-0.3	0.0	0.5	0.0	0.1
3	B	9.4	0.0	-4.2	-1.7	-9.3	-2.8	-4.8	0.0	3.6	0.0	1.2	0.0	1.2
3	C	3.6	1.7	-5.7	0.0	-3.1	0.0	4.5	2.8	-3.3	0.0	0.1	0.0	0.5

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	B	10	5.9	0.2	11	-5.4	0.2	4	0.750	6.000	10.00	0.375	0.0
		8	0.0	0.4									
3	A	12	2.1	-3.3	13	-1.9	-3.3	4	0.750	6.000	8.000	0.375	0.0
		1	0.0	3.0	12	2.1	-3.3						
3	B	14	5.7	-8.4	11	-5.1	-4.6	4	0.750	6.000	8.000	0.375	0.0
		9	0.0	10.1	14	5.7	-8.4						
3	C	15	2.1	-4.9	13	-1.9	-3.1	4	0.750	6.000	8.000	0.375	0.0
		16	1.6	5.3	15	2.1	-4.9						

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	C	0.3	1.3	0.5	1.6	1.6	5.0	0.4	1.3	-6.6	-9.7	3.5	-4.8
1	A	-0.3	1.3	-0.5	1.6	-1.6	5.0	-0.4	1.3	-3.5	-4.8	6.6	-9.7

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	C	-7.4	-6.3	2.7	-1.4	1.9	-6.8	0.6	-6.7	-1.4	-1.3	1.4	1.3
1	A	-2.7	-1.4	7.4	-6.3	-0.6	-6.7	-1.9	-6.8	-1.4	1.3	1.4	-1.3

Frame Line	Column Line	F1UNB_SL_L-		F1UNB_SL_R-	
		Horz	Vert	Horz	Vert
1	C	0.3	1.2	0.3	0.7
1	A	-0.3	0.7	-0.3	1.2

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	C	0.7	2.3	1.3	3.9	3.9	11.9	1.0	3.0	-12.6	-19.1	7.1	-9.8
2	A	-0.7	2.3	-1.3	3.9	-3.9	11.9	-1.0	3.0	-7.1	-9.8	12.6	-19.1

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	C	-14.5	-11.0	5.2	-1.7	4.3	-16.2	1.3	-15.9	-2.5	-2.3	2.5	2.3
2	A	-5.2	-1.7	14.5	-11.0	-1.3	-15.9	-4.3	-16.2	-2.5	2.3	2.5	-2.3

Frame Line	Column Line	F2UNB_SL_L-		F2UNB_SL_R-	
		Horz	Vert	Horz	Vert
2	C	0.8	2.8	0.8	1.7
2	A	-0.8	1.7	-0.8	2.8

WIND BENT REACTIONS

Loc	Wall Line	Col Line	Wind(k)		Seismic(k)		Bolt(in) Qty	Dia	Base_Plate(in)		
			Horz	Vert	Horz	Vert			Width	Length	Thick
F_SW	A	1	3.1	4.4	1.9	2.6	4	0.750	6.000	8.375	0.500
F_SW	A	2	3.1	4.4	1.9	2.6	4	0.750	6.000	8.375	0.500
B_SW	C	2	3.1	4.4	1.9	2.6	4	0.750	6.000	8.375	0.500
B_SW	C	1	3.1	4.4	1.9	2.6	4	0.750	6.000	8.375	0.500

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	Wind		Seismic		Panel_Shear (lb/ft)		Note
			Horz	Vert	Horz	Vert	Wind	Seis	
L_EW	1								(h)
F_SW	A	1,2							(a)
R_EW	3	B,C	4.8	6.2	2.2	2.8			
B_SW	C	1,2							(a)

(a) Wind bent in bay
(h) Rigid frame at endwall

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
4	Jamb	5/8"	A307
16	Endwall	3/4"	A307
16	Frame	3/4"	A307
16	WindCol	3/4"	A307

NOTES FOR REACTIONS

Building reactions are based on the following building data:

- Width (ft) = 40.0
- Length (ft) = 48.0
- Eave Height (ft) = 18.0/18.0
- Roof Slope (rise/12) = 5.0/5.0
- Dead Load (psf) = 2.7
- Collateral Load (psf) = 6.0
- Live Load (psf) = 20.0
- Snow Load (psf) = 5.0
- Wind Speed (mph) = 138.0
- Wind Code = IBC-18
- Exposure = C
- Closed/Open = C
- Importance Wind = 1.00
- Importance Seismic = 1.00
- Seismic Zone = D
- Seismic Coeff (Fa*Ss) = 1.15

ID	Description
1	Dead+Collateral+Live
2	Dead+Collateral+0.75Live+0.45Wind_Left1
3	Dead+Collateral+0.75Live+0.45Wind_Right1
4	0.6Dead+0.6Wind_Left1
5	0.6Dead+0.6Wind_Right1
6	0.6Dead+0.6Wind_Left2
7	0.6Dead+0.6Wind_Right2
8	1.11Dead+1.11Collateral+0.75Seismic_Right
9	1.08Dead+1.08Collateral+0.75Live+0.53Seismic_Right
10	0.6Dead+0.6Wind_Right2+0.6Wind_Suction
11	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
12	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
13	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
14	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
15	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
16	Dead+Collateral+0.75Live+0.45Wind_Left2+0.45Wind_Suction

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ

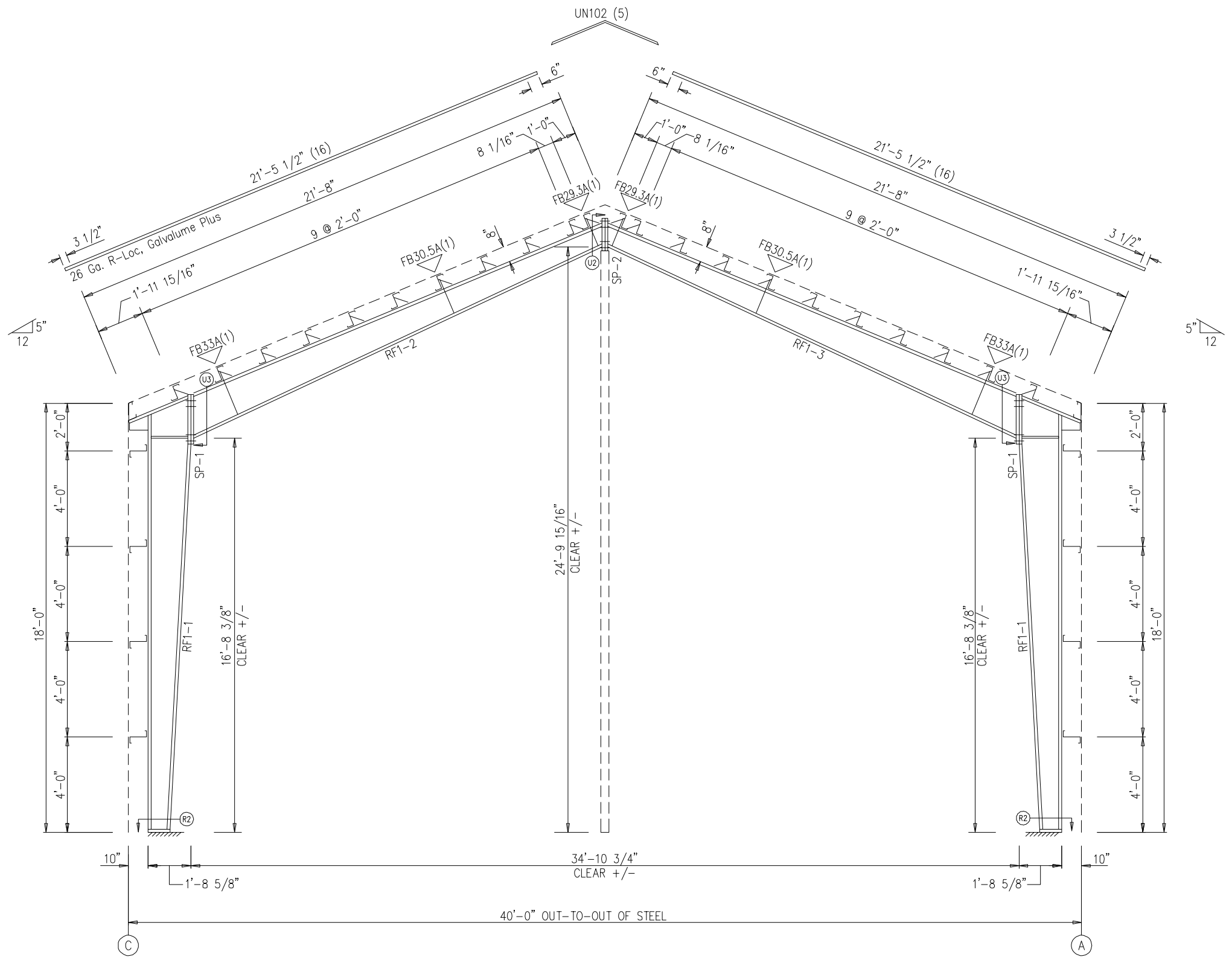
2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ANCHOR BOLT REACTIONS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57243	ENG. BY: MZ
DWG. NO.: 3 OF 14	DATE: 11/19/21
	ISSUE: C

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325	0.750	2.00
SP-2	4	4	0	A325	0.625	1.75

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start/End	Thick	Length	Thick	W x Thk x Length	W x Thk x Length
RF1-1	8.0/20.0	0.135	18'-3 3/16"		5 x 1/4" x 17'-6 3/4"	5 x 5/16" x 9'-5 3/16"
RF1-2	18.0/ 8.0	0.135	19'-5 1/4"		5 x 1/4" x 2'-8 11/16"	5 x 3/8" x 6'-10 3/4"
RF1-3	8.0/18.0	0.135	19'-5 1/4"		5 x 1/4" x 18'-9 5/8"	5 x 1/4" x 19'-2"

▽ FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 A - B316



RIGID FRAME ELEVATION: FRAME LINE 1

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ

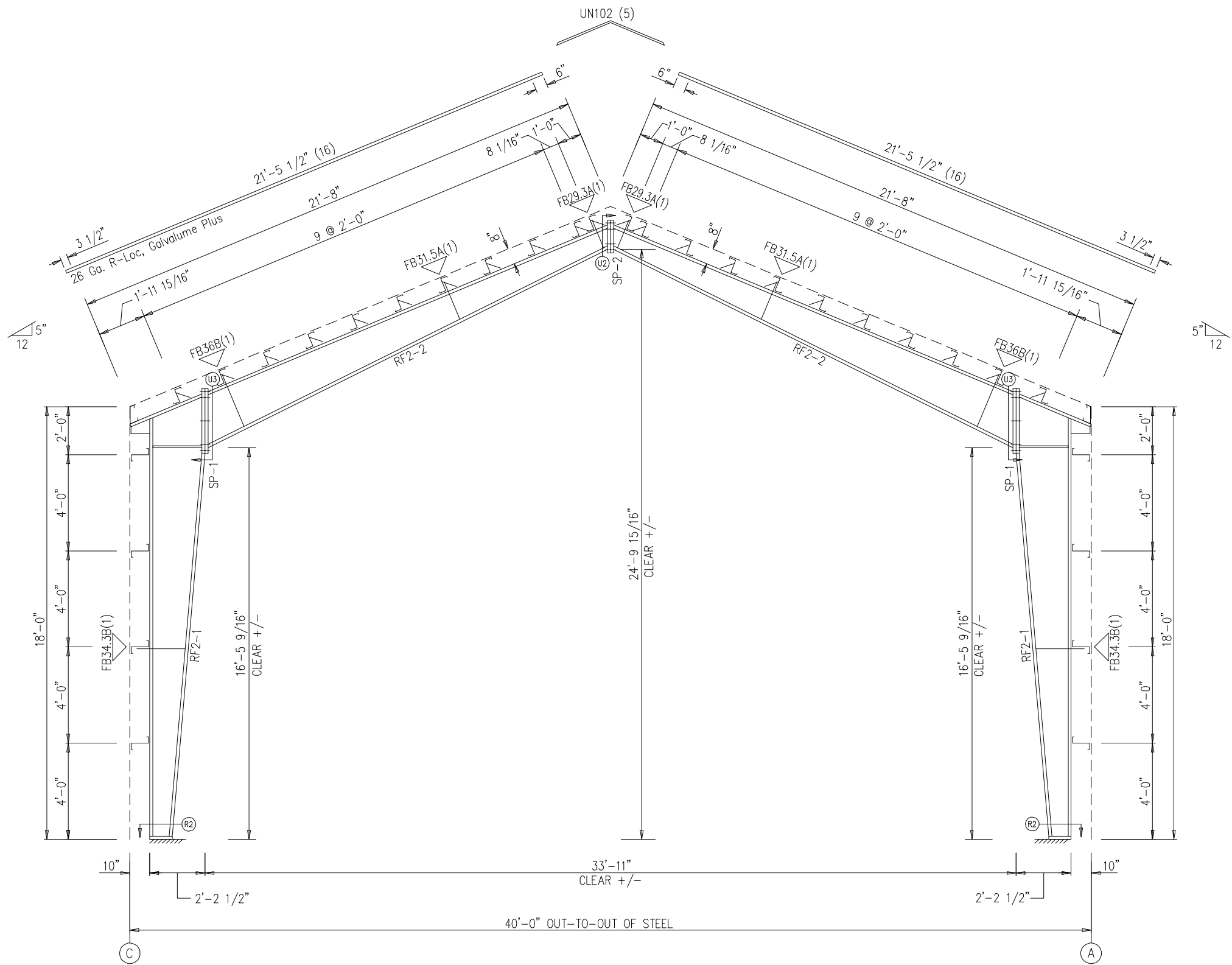
2 Inverness Drive East, Ste#200
 Englewood, Colorado 80112
 PHONE: 800-345-4610
 www.armstrongsteel.com

DESCRIPTION	RIGID FRAME ELEVATION		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	57243	ENG. BY:	MZ
DWG. NO.:	4 OF 14	DATE:	11/19/21
ISSUE:			C

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	2	A325	0.750	2.25
SP-2	4	4	0	A325	0.625	2.00

MEMBER TABLE								
Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length	W x Thk x Length	
RF2-1	8.0	25.7	0.135	15'-9 13/16"	5 x 1/4" x 17'-6 3/4"	5 x 1/4" x 16'-1 5/8"		
	25.7	26.0	0.188	2'-7 7/8"	5 x 1/4" x 3'-3 3/16"			
RF2-2	23.0	8.0	0.135	19'-0 7/16"	5 x 1/4" x 18'-2 3/4"	5 x 1/4" x 18'-9 7/16"		

▽ FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 B - B314
 A - B316



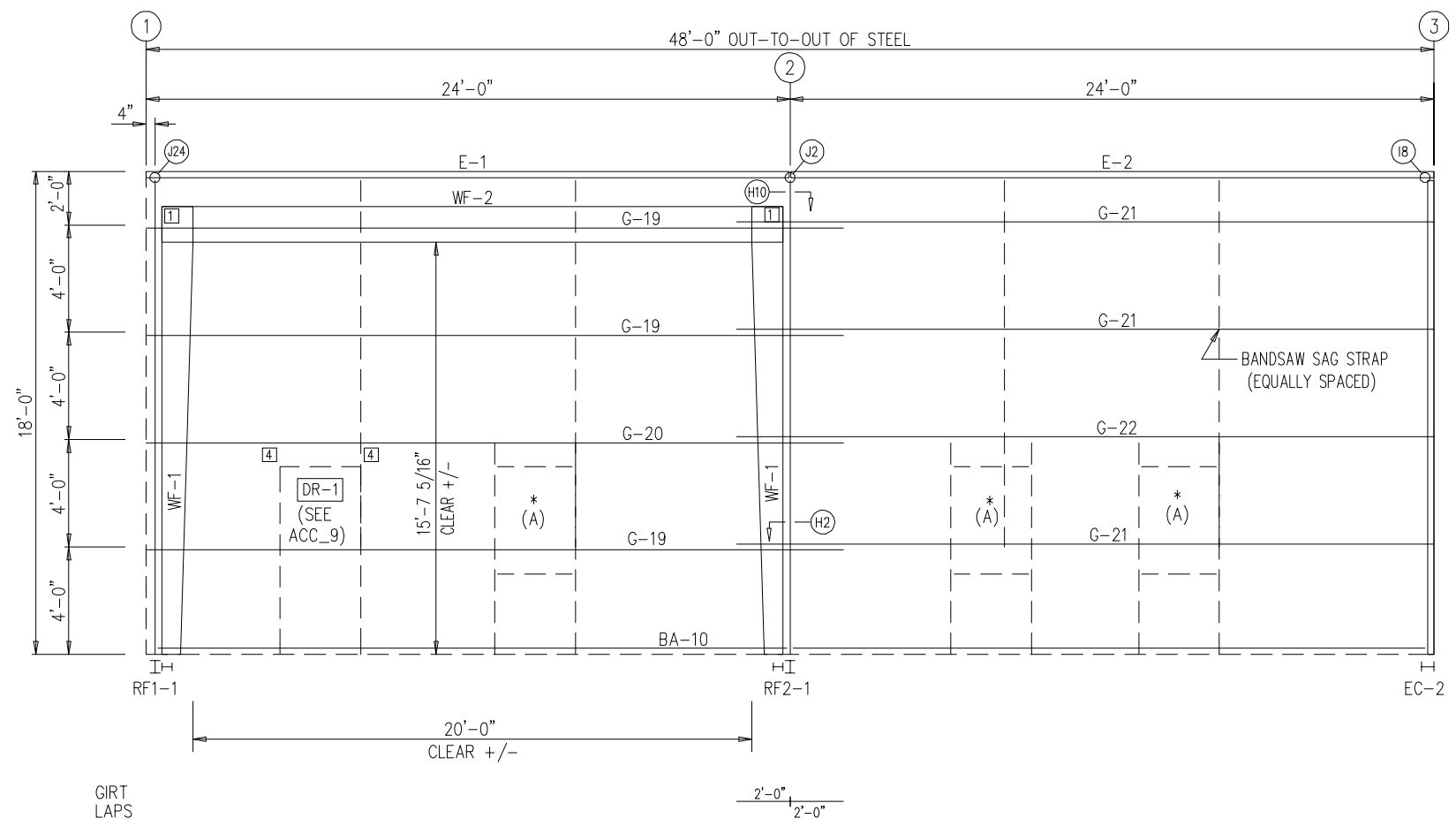
RIGID FRAME ELEVATION: FRAME LINE 2

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

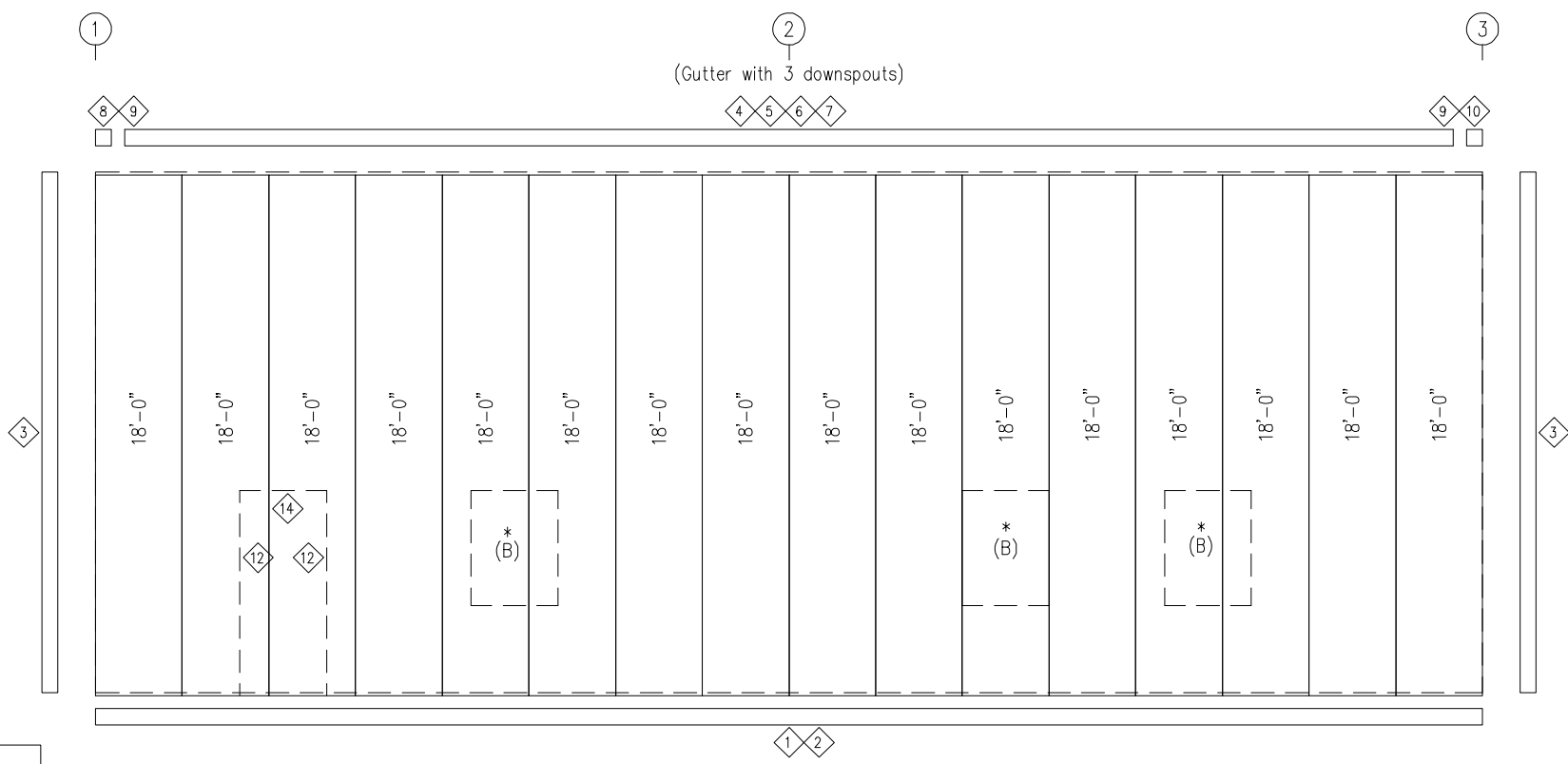
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ

2 Inverness Drive East, Ste#200
 Englewood, Colorado 80112
 PHONE: 800-345-4610
 www.armstrongsteel.com

DESCRIPTION	RIGID FRAME ELEVATION		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	ENG. BY:	DATE:	
57243	MZ	11/19/21	
DWG. NO.:	OF	ISSUE:	
5	14	C	



FRONT SIDEWALL FRAMING: FRAME LINE A



FRONT SIDEWALL SHEETING & TRIM: FRAME LINE A

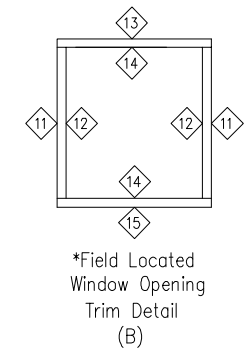
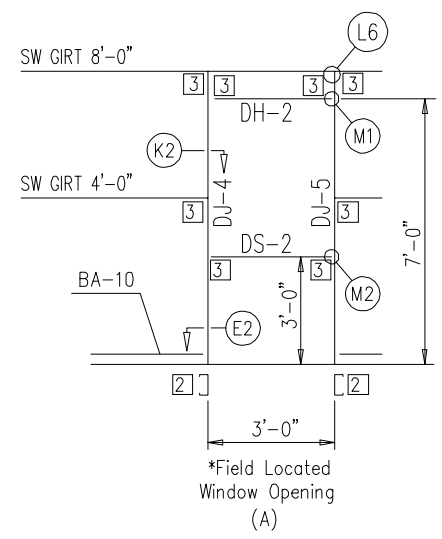
PANELS: 26 Ga. R-Loc - Charcoal 40 yr

TRIM TABLE				
FRAME LINE A				
ID	QUAN	PART	LENGTH	DETAIL
1	1	BA6102	10'-2"	TRIM_2
2	2	BA6	20'-4"	TRIM_2
3	2	OU6	18'-2"	TRIM_30
4	2	Q760F6	16'-2"	TRIM_63
5	1	Q760F6	18'-2"	TRIM_63
6	2	Q1906	16'-2"	TRIM_63
7	1	Q1906	18'-2"	TRIM_63
8	1	Q761L6	6"	
9	2	AR9626	8 1/16"	TRIM_62
10	1	Q761R6	6"	
11	6	AR3716	4'-7"	TRIM_50
12	8	JA6	7'-2"	TRIM_50
13	3	AR3716	3'-7"	TRIM_51
14	7	HE6	3'-6"	TRIM_51
15	3	AR3716	3'-7"	TRIM_51

BOLT TABLE				
FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	5/8"	2 1/4"
WF-1 - RF1-1	8	A325	5/8"	1 1/2"
WF-1 - RF2-1	8	A325	5/8"	1 1/2"

MEMBER TABLE			
FRAME LINE A			
QUAN	MARK	PART	LENGTH
2	WF-1	BEAM	16'-11 5/16"
1	WF-2	W16631	19'-11 1/8"
3	DJ-4	10X25C16	7'-8"
3	DJ-5	10X25C16	7'-8"
3	DH-2	10X25C16	2'-11"
3	DS-2	10X25C16	2'-11"
1	E-1	08536DU5	23'-11"
1	E-2	08536DU5	23'-11"
3	G-19	10X25Z16	25'-11 1/2"
1	G-20	10X25Z14	25'-11 1/2"
3	G-21	10X25Z16	25'-11 1/2"
1	G-22	10X25Z14	25'-11 1/2"

CONNECTION PLATES		
FRAME LINE A		
ID	QUAN	MARK/PART
1	2	BC-49
2	6	BC-05
3	24	BC-01
4	2	BC-500

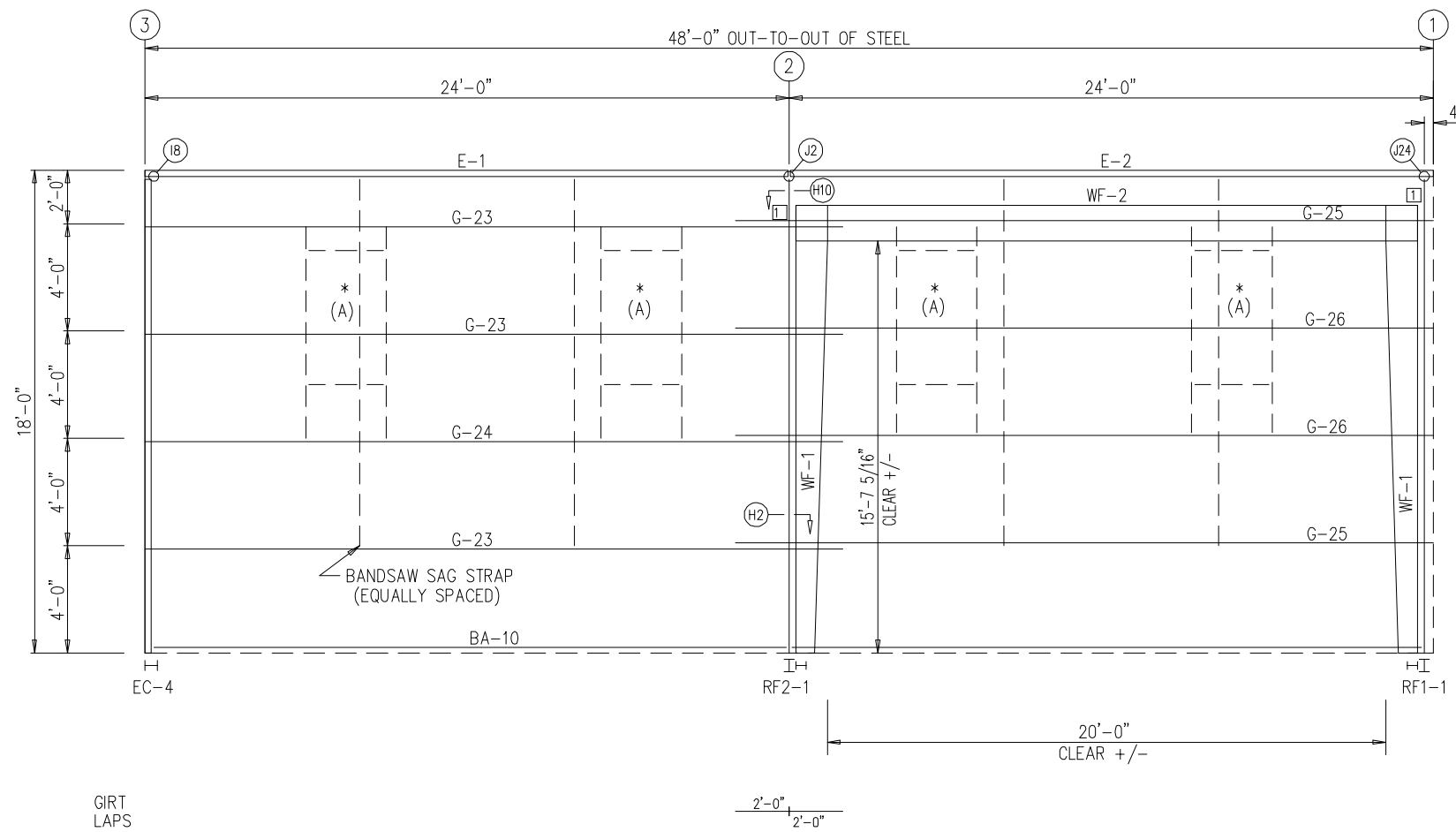


NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

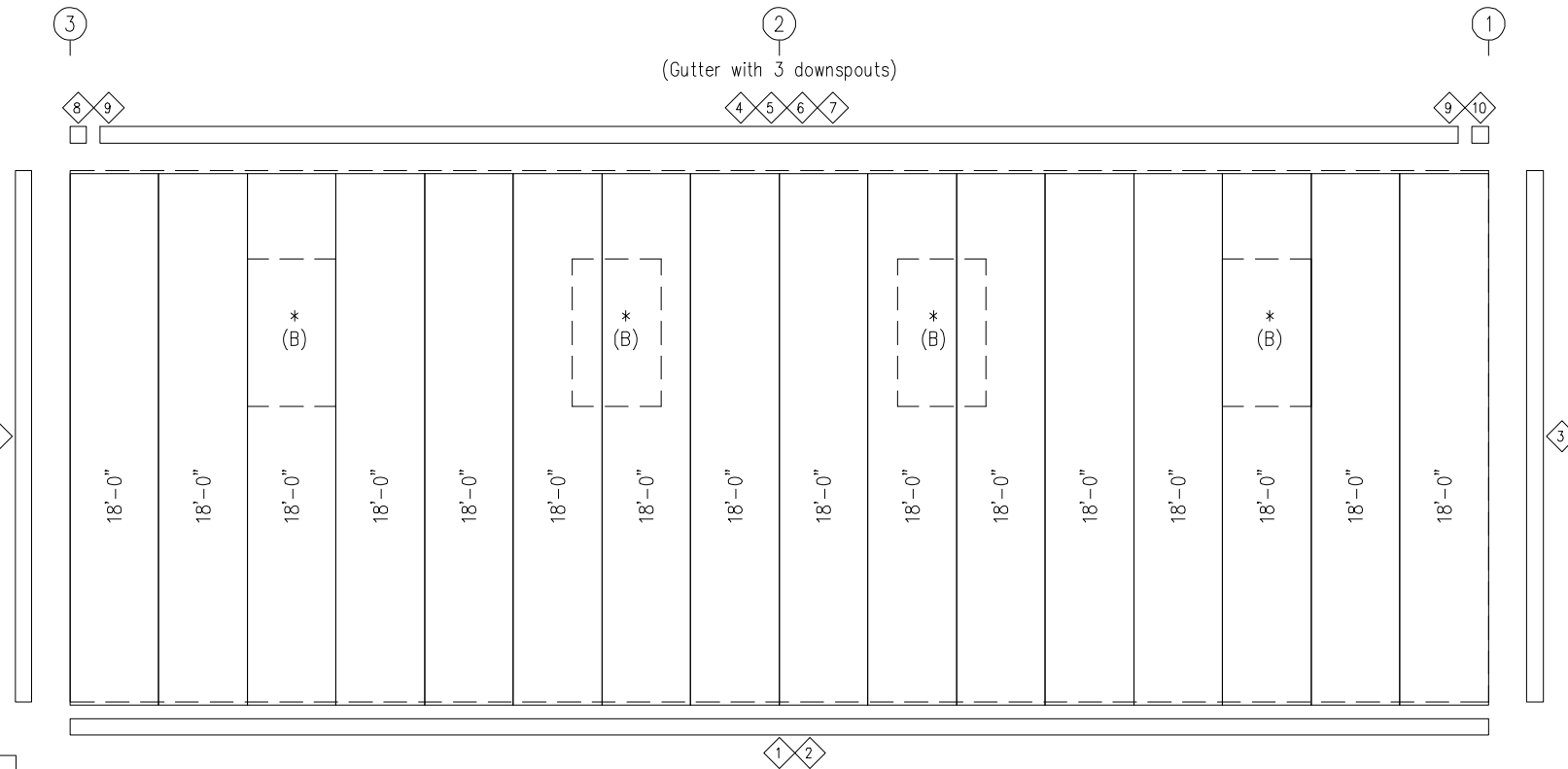
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	SIDEWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57243	DATE: 11/19/21
ENG. BY: MZ	ISSUE: C
DWG. NO.: 6 OF 14	



BACK SIDEWALL FRAMING: FRAME LINE C



BACK SIDEWALL SHEETING & TRIM: FRAME LINE C

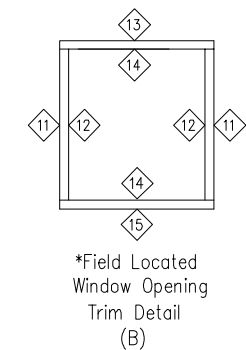
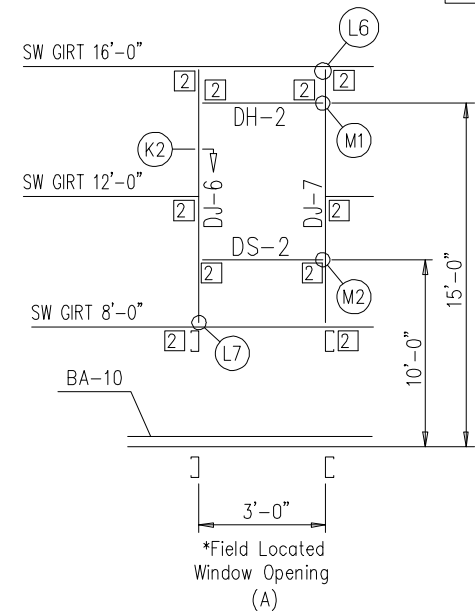
PANELS: 26 Ga. R-Loc - Charcoal 40 yr

TRIM TABLE FRAME LINE C				
ID	QUAN	PART	LENGTH	DETAIL
1	1	BA6102	10'-2"	TRIM_2
2	2	BA6	20'-4"	TRIM_2
3	2	OU6	18'-2"	TRIM_30
4	2	Q760F6	16'-2"	TRIM_63
5	1	Q760F6	18'-2"	TRIM_63
6	2	Q1906	16'-2"	TRIM_63
7	1	Q1906	18'-2"	TRIM_63
8	1	Q761L6	6"	
9	2	AR9626	8 1/16"	TRIM_62
10	1	Q761R6	6"	
11	8	AR3716	5'-7"	TRIM_50
12	8	JA6	7'-2"	TRIM_50
13	4	AR3716	3'-7"	TRIM_51
14	8	HE6	3'-6"	TRIM_51
15	4	AR3716	3'-7"	TRIM_51

BOLT TABLE FRAME LINE C				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	5/8"	2 1/4"
WF-1 - RF2-1	8	A325	5/8"	1 1/2"
WF-1 - RF1-1	8	A325	5/8"	1 1/2"

MEMBER TABLE FRAME LINE C			
QUAN	MARK	PART	LENGTH
2	WF-1	BEAM	16'-11 5/16"
1	WF-2	W16631	19'-11 1/8"
4	DJ-6	10X25C16	7'-4"
4	DJ-7	10X25C16	7'-4"
4	DH-2	10X25C16	2'-11"
4	DS-2	10X25C16	2'-11"
1	E-1	08536DU5	23'-11"
1	E-2	08536DU5	23'-11"
3	G-23	10X25Z16	25'-11 1/2"
1	G-24	10X25Z14	25'-11 1/2"
2	G-25	10X25Z16	25'-11 1/2"
2	G-26	10X25Z14	25'-11 1/2"

CONNECTION PLATES FRAME LINE C		
ID	QUAN	MARK/PART
1	2	BC-49
2	40	BC-01

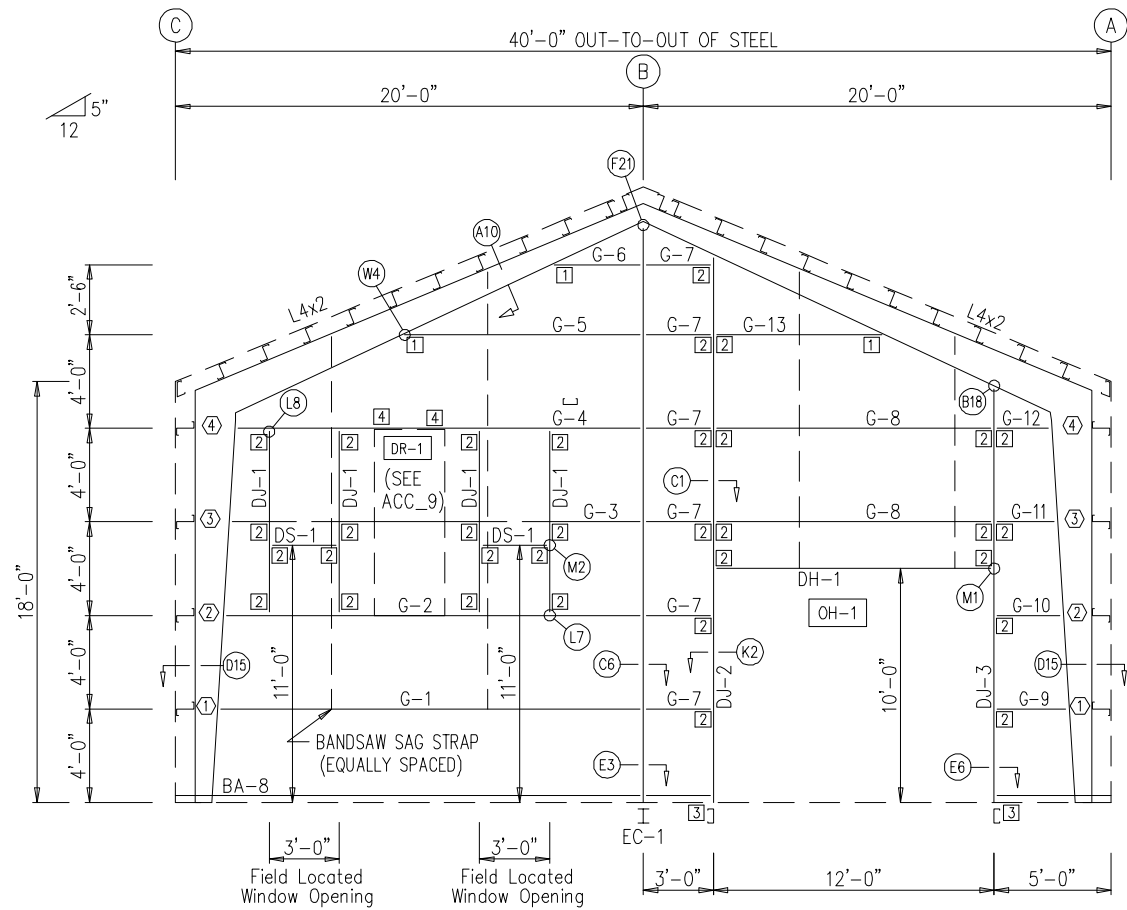


NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

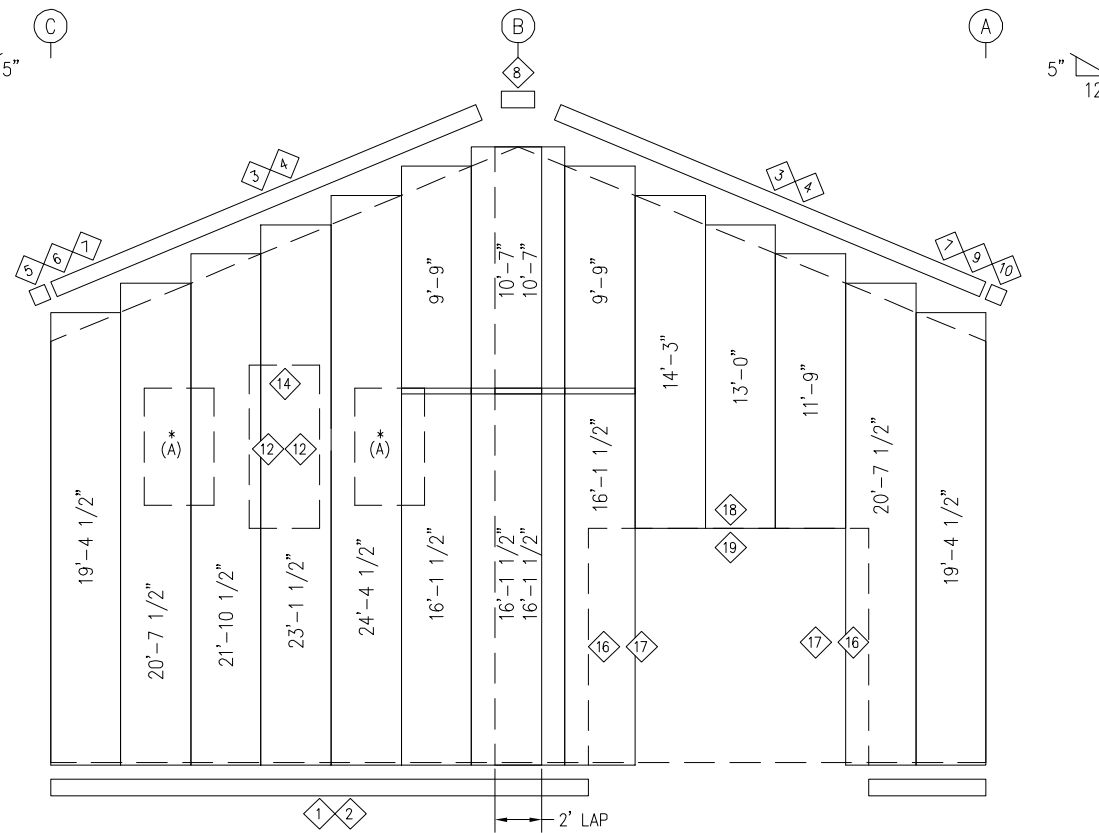
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.18.23	KAC	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	SIDEWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.:	57243
ENG. BY:	MZ
DATE:	11/19/21
DWG. NO.:	7 OF 14
ISSUE:	C

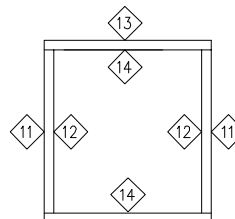


LEFT ENDWALL FRAMING: FRAME LINE 1



LEFT ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. R-Loc - Charcoal 40 yr



*Field Located Window Opening Trim Detail (A)

TRIM TABLE FRAME LINE 1				
ID	QUAN	PART	LENGTH	DETAIL
1	2	BA6102	10'-2"	TRIM_2
2	1	BA6	20'-4"	TRIM_2
3	2	Q7646102	10'-2"	TRIM_66
4	2	Q7646	12'-2"	TRIM_66
5	1	Q765L6	6"	
6	1	AR963L6	9 1/8"	
7	2	AR9626	8 1/16"	
8	1	Q7676	1'-4"	TRIM_100
9	1	Q765R6	6"	
10	1	AR963R6	9 1/8"	
11	4	Q3706	5'-7"	TRIM_50
12	6	JA6	7'-2"	TRIM_50
13	2	AR3806	3'-7"	TRIM_51
14	5	HE6	3'-6"	TRIM_51
15	2	Q3706	3'-7"	TRIM_51
16	2	AR3716	12'-2"	TRIM_50
17	2	JA6102	10'-2"	TRIM_50
18	1	AR3716	12'-2"	TRIM_51
19	1	HE6	12'-3"	TRIM_51

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	1/2"	1 1/4"
Jamb/Raf	2	A325	1/2"	1 1/4"

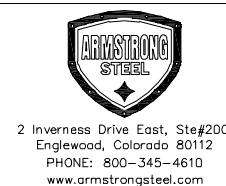
MEMBER TABLE FRAME LINE 1			
QUAN	MARK	PART	LENGTH
1	EC-1	W10541	24'-5 3/8"
4	DJ-1	8X25C16	7'-7 1/2"
1	DJ-2	10X35C12	23'-1 1/4"
1	DJ-3	10X35C12	17'-5 3/4"
1	DH-1	10X25C16	11'-11"
2	DS-1	8X25C16	2'-11"
1	G-1	8X25Z14	17'-10"
1	G-2	8X35Z12	17'-7 1/16"
1	G-3	8X25Z16	17'-4 1/16"
1	G-4	8X25C16	17'-1 1/8"
1	G-5	8X25Z16	9'-8 1/16"
1	G-6	8X25Z16	4'-4"
6	G-7	8X25Z16	2'-4"
2	G-8	8X25Z16	11'-11"
1	G-9	8X25Z16	2'-10"
1	G-10	8X25Z16	2'-7 1/16"
1	G-11	8X25Z16	2'-4 1/16"
1	G-12	8X25Z16	2'-1 1/8"
1	G-13	8X25Z16	6'-11 9/16"

CONNECTION PLATES FRAME LINE 1			
ID	QUAN	MARK/PART	
1	3	BC-15F	
2	33	BC-01	
3	2	BC-05	
4	2	BC-500	

4" ZEE TABLE	
ID	LENGTH
1	10"
2	1'-0"
3	1'-2"
4	1'-6"

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ



DESCRIPTION	ENDWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57243	DATE: 11/19/21
ENG. BY: MZ	ISSUE: C
DWG. NO.: 8 OF 14	

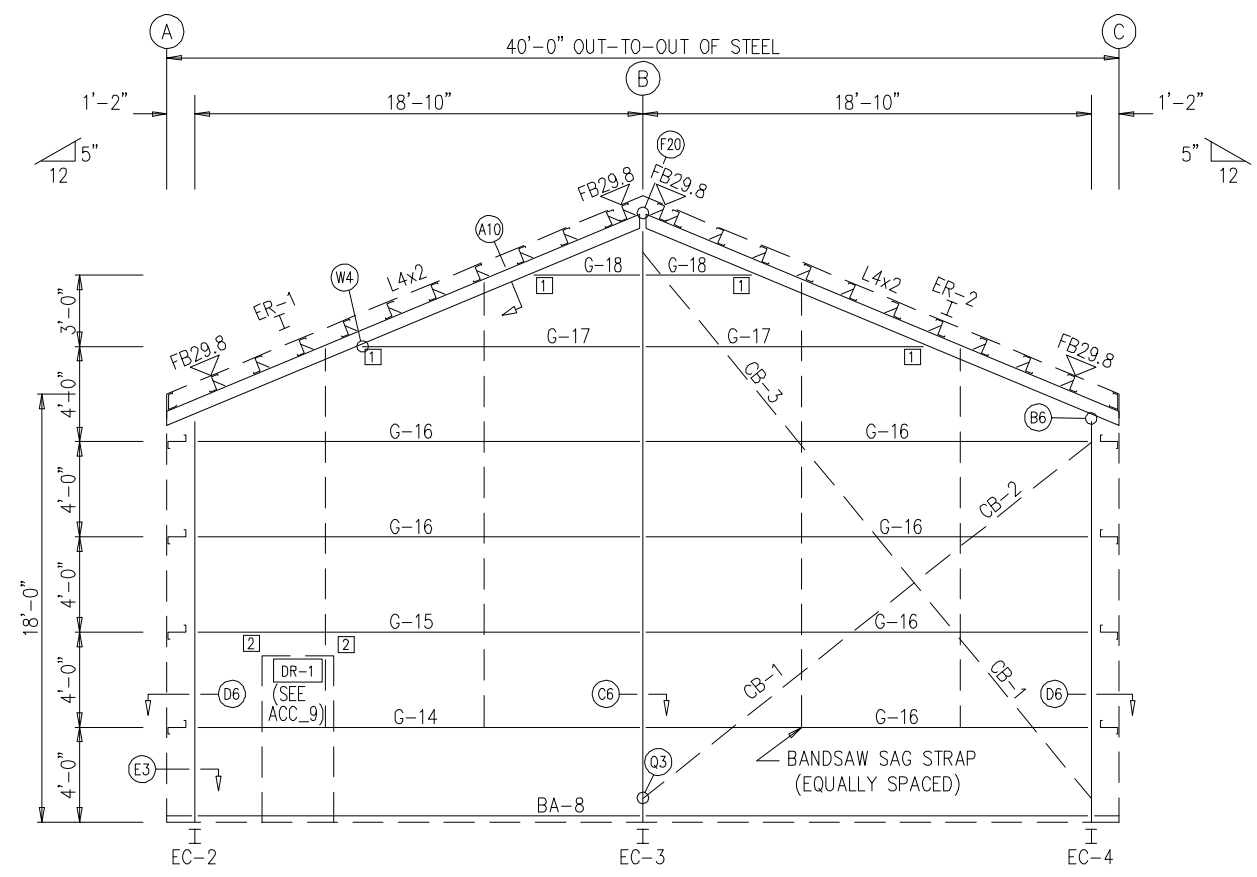
TRIM TABLE FRAME LINE 3				
ID	QUAN	PART	LENGTH	DETAIL
1	2	BA6	20'-4"	TRIM_2
2	2	Q7646102	10'-2"	TRIM_66
3	2	Q7646	12'-2"	TRIM_66
4	1	Q765L6	6"	
5	1	AR963L6	9 1/8"	
6	2	AR9626	8 1/16"	
7	1	Q7676	1'-4"	TRIM_100
8	1	Q765R6	6"	
9	1	AR963R6	9 1/8"	
10	2	JA6	7'-2"	TRIM_50
11	1	HE6	3'-6"	TRIM_51

BOLT TABLE FRAME LINE 3				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	8	A325	5/8"	1 3/4"
Cor_Column/Raf	4	A325	1/2"	1 1/4"
EC-3/ER-2	4	A325	5/8"	1 1/2"

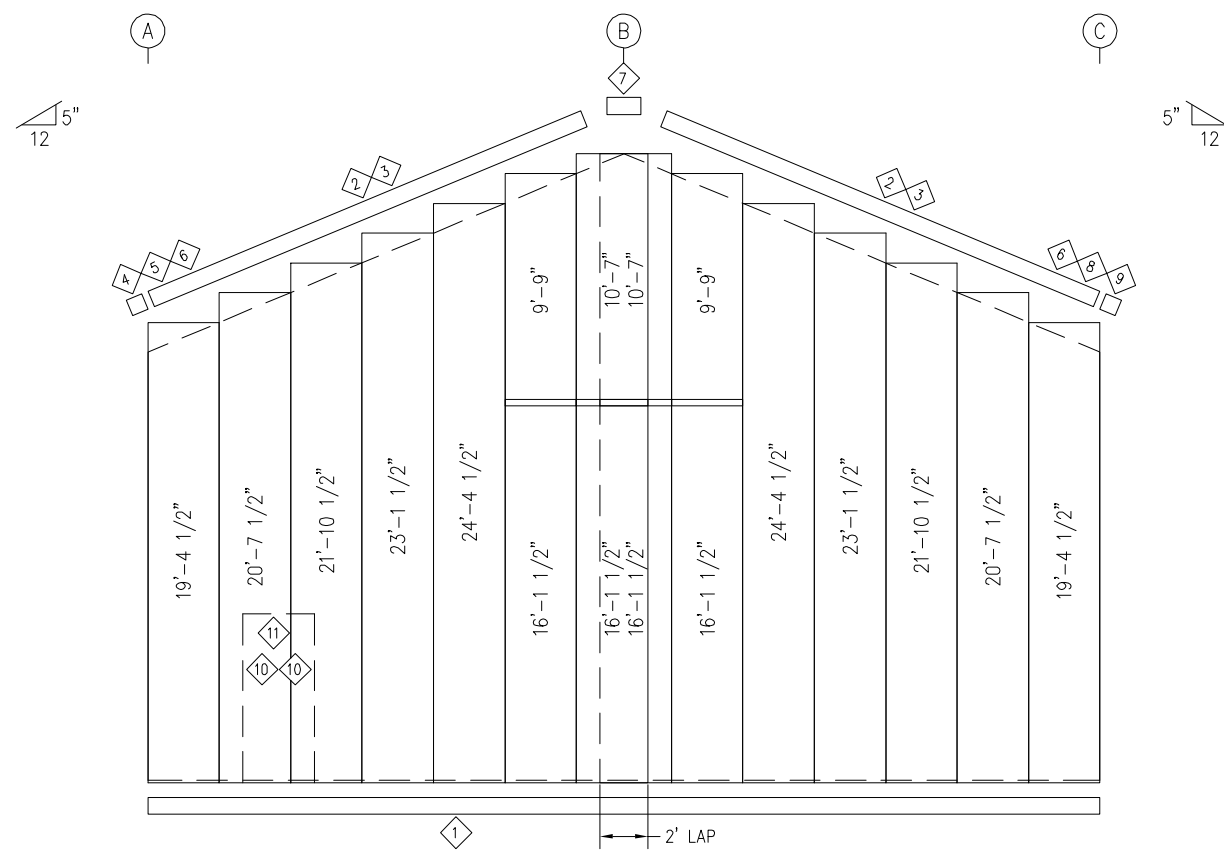
MEMBER TABLE FRAME LINE 3			
QUAN	MARK	PART	LENGTH
1	EC-2	W08531	16'-8 5/16"
1	EC-3	W08561	24'-5 1/4"
1	EC-4	W08531	16'-8 5/16"
1	ER-1	W10531	21'-7 7/16"
1	ER-2	W10531	21'-7 7/16"
1	G-14	8X25Z16	18'-2"
1	G-15	8X25Z12	18'-2"
6	G-16	8X25Z14	18'-2"
2	G-17	8X25Z16	10'-7 5/8"
2	G-18	8X25Z16	3'-5 3/16"
2	CB-1	BR5/8	20'-0"
1	CB-2	BR5/8	4'-4 1/2"
1	CB-3	BR5/8	9'-9 1/4"

FLANGE BRACE TABLE FRAME LINE 3			
ID	QUAN	MARK	LENGTH
1	4	FB29.8	2'-5 3/4"

CONNECTION PLATES FRAME LINE 3		
ID	QUAN	MARK/PART
1	4	BC-15F
2	2	BC-500



RIGHT ENDWALL FRAMING: FRAME LINE 3



RIGHT ENDWALL SHEETING & TRIM: FRAME LINE 3

PANELS: 26 Ga. R-Loc - Charcoal 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

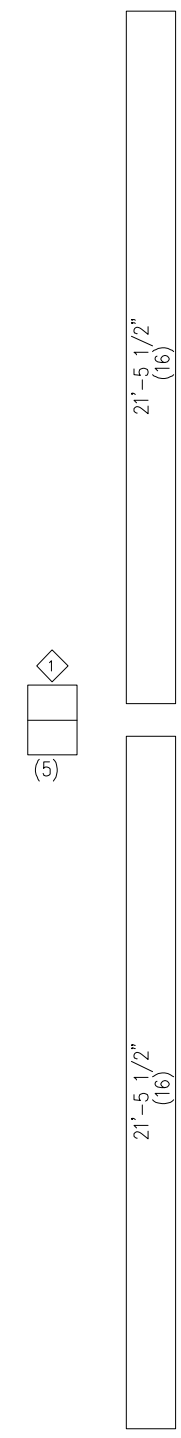
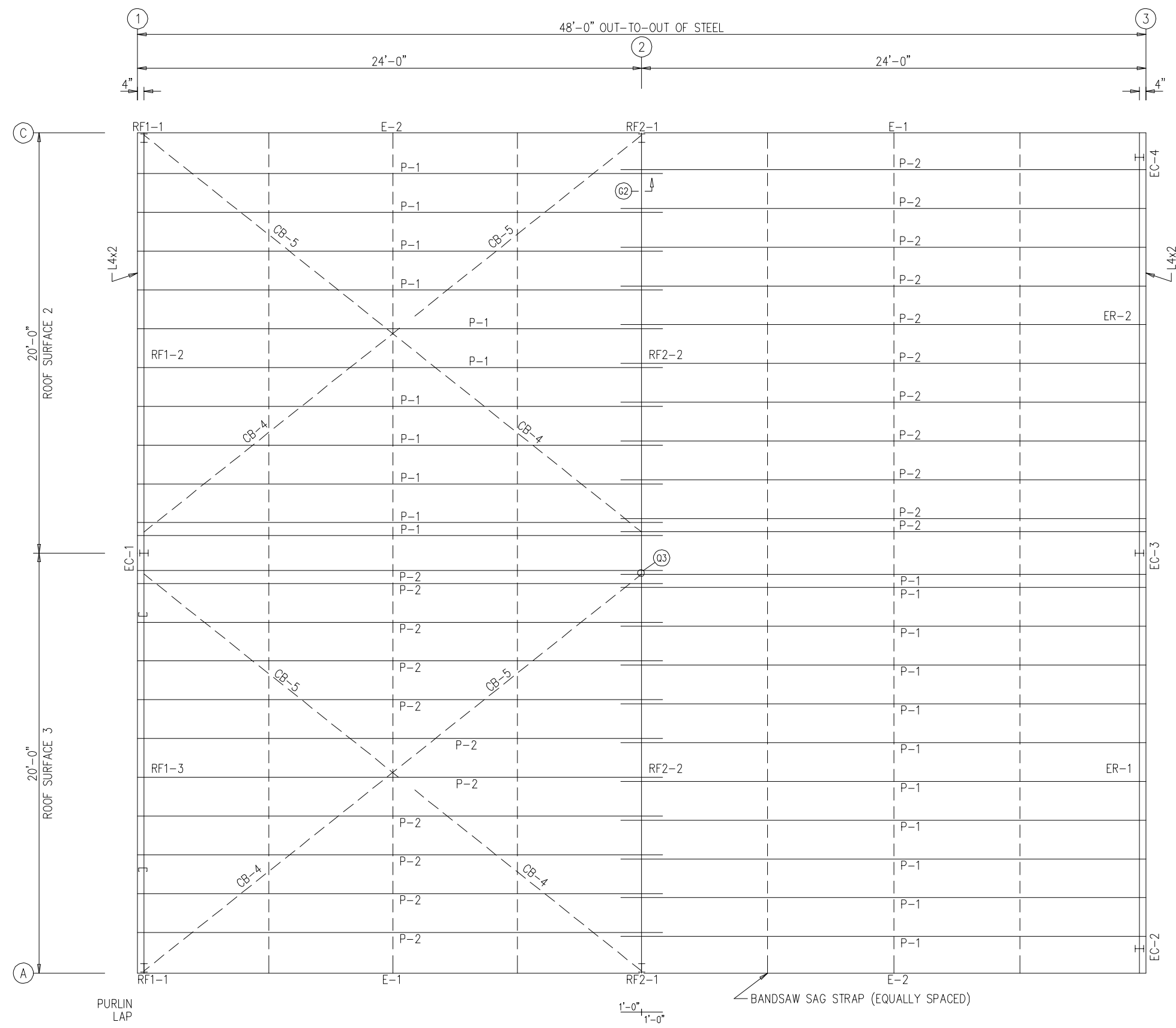
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ

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DESCRIPTION	ENDWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.:	57243
ENG. BY:	MZ
DWG. NO.:	9 OF 14
DATE:	11/19/21
ISSUE:	C

TRIM TABLE				
ROOF PLAN				
◇ID	QUAN	PART	LENGTH	DETAIL
1	5	UN6102	10'-2"	TRIM_101

MEMBER TABLE				
ROOF PLAN				
QUAN	MARK	PART	LENGTH	
22	P-1	8x25Z16	24'-11 1/2"	
22	P-2	8x25Z16	24'-11 1/2"	
2	E-1	08536DU5	23'-11"	
2	E-2	08536DU5	23'-11"	
4	CB-4	BR1/2	10'-8 1/4"	
4	CB-5	BR1/2	20'-0"	



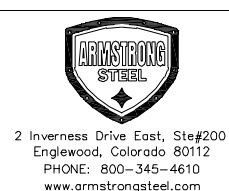
ROOF SHEETING

PANELS: 26 Ga. R-Loc Galvalume Plus

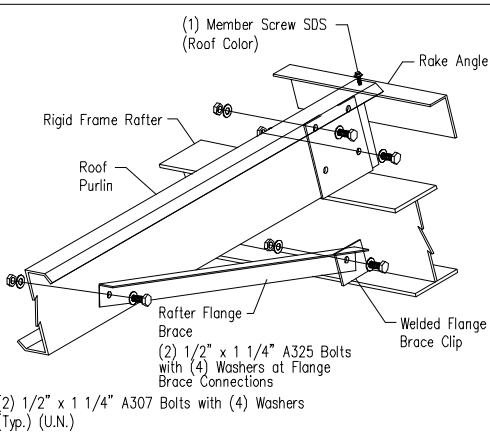
ROOF FRAMING PLAN

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

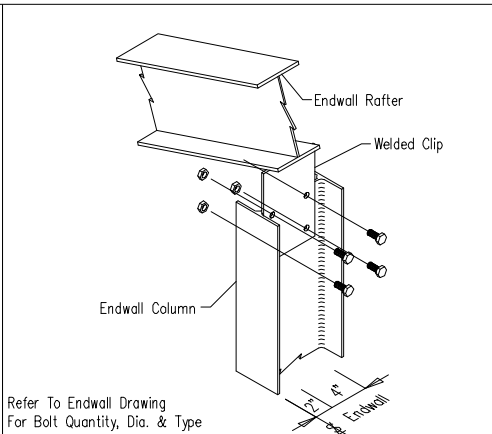
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ



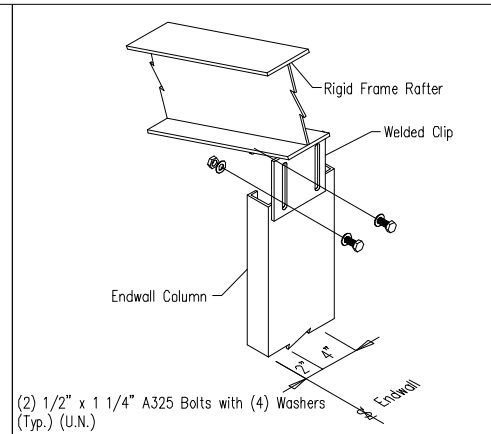
DESCRIPTION	ROOF FRAMING & SHEETING		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	57243	ENG. BY:	MZ
DATE:	11/19/21	DWG. NO.:	10 OF 14
ISSUE:	C		



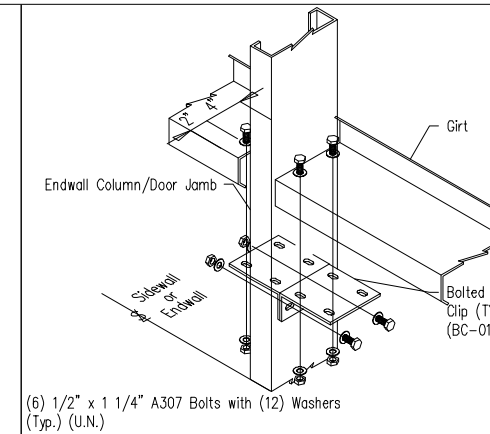
A10 ROOF PURLIN TO RIGID FRAME RAFTER



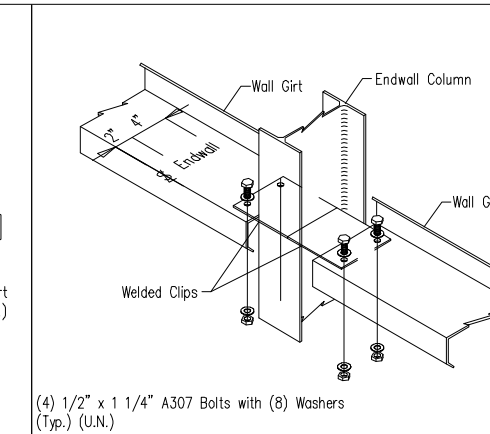
B6 BUILT-UP ENDWALL COLUMN TO ENDWALL RAFTER



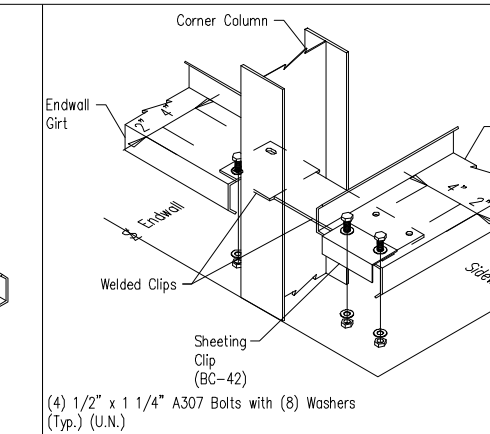
B18 CEE ENDWALL COLUMN TO RIGID FRAME RAFTER



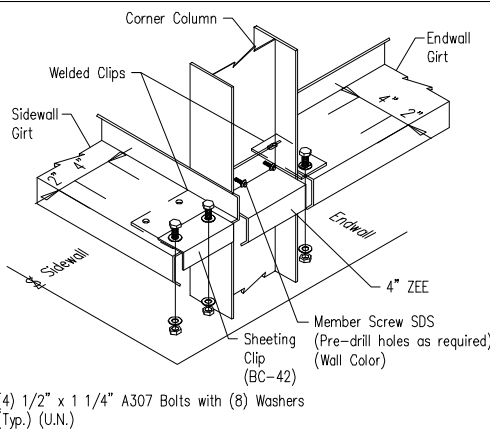
C1 WALL GIRTS TO ENDWALL COLUMN/DOOR JAMB



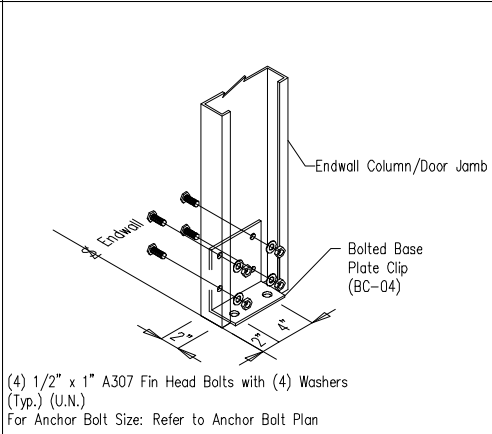
C6 WALL GIRTS TO ENDWALL COLUMN



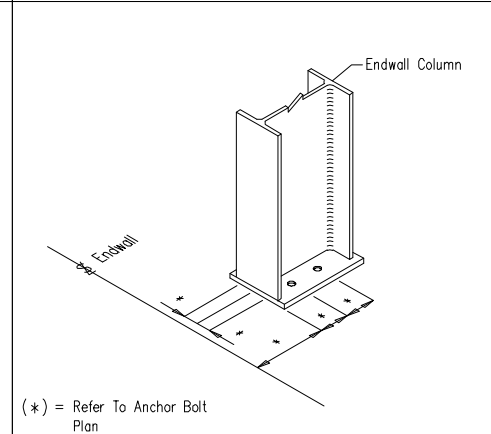
D6 WALL GIRTS TO ENDWALL CORNER COLUMN



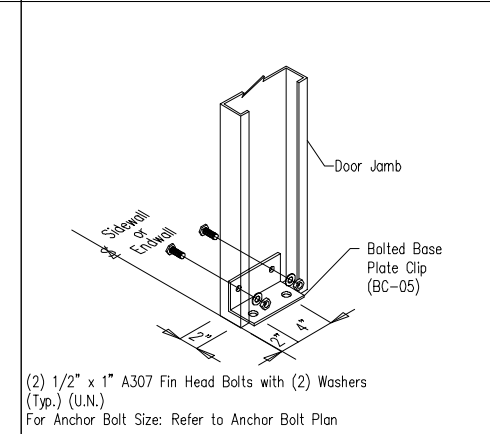
D15 WALL GIRTS TO ENDWALL CORNER COLUMN



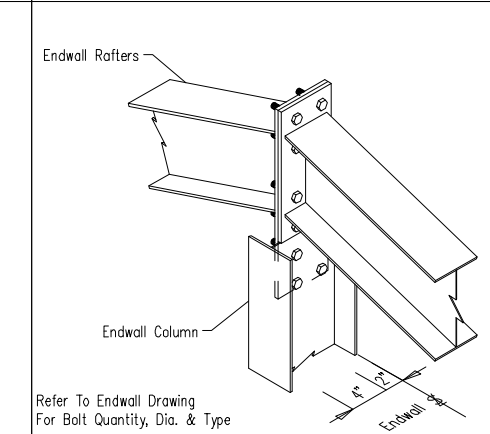
E2 BASE PLATE FOR ENDWALL COLUMN/DOOR JAMB



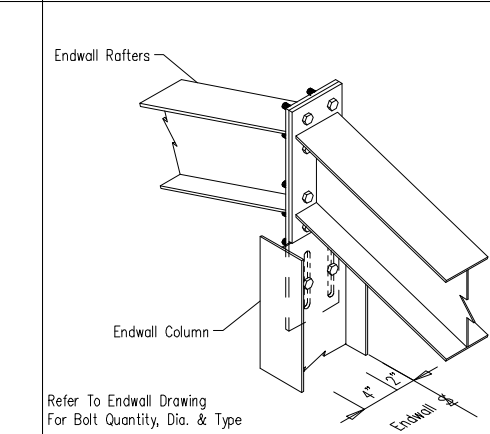
E3 ANCHOR BOLTS AT ENDWALL COLUMNS



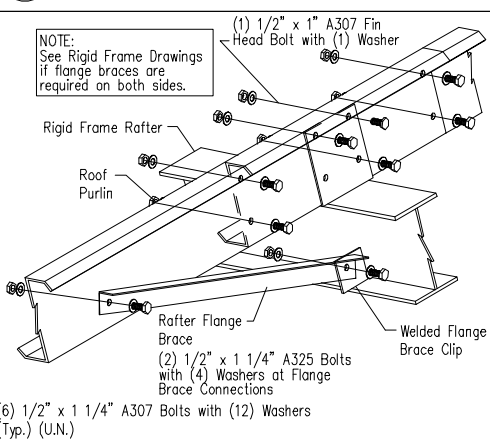
E6 BASE PLATE FOR DOOR JAMB



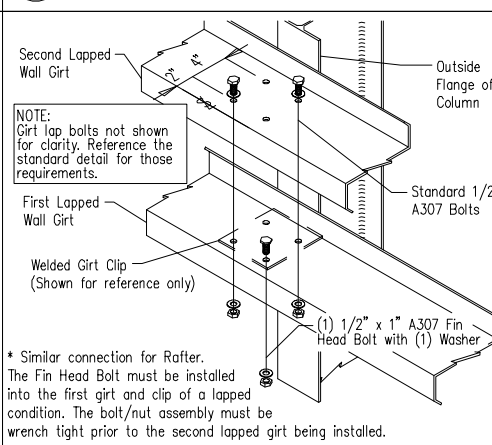
F20 RAFTER SPLICE AT PEAK WITH COLUMN



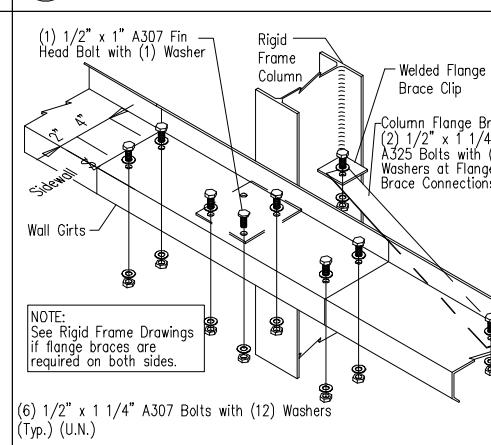
F21 RAFTER SPLICE SLOT CONNECTION WITH COLUMN



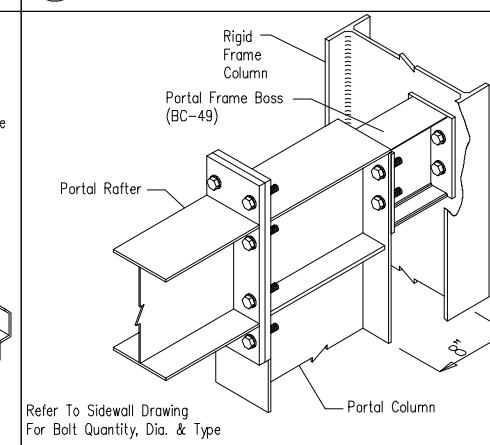
G2 ROOF PURLIN TO INTERIOR RIGID FRAME



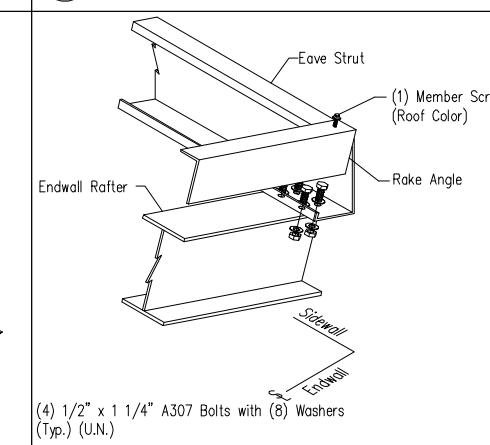
H0 BYPASS LAPPED WALL GIRTS



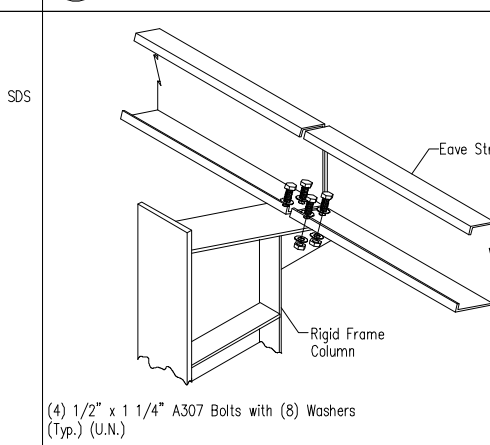
H2 WALL GIRTS TO RIGID FRAME COLUMN



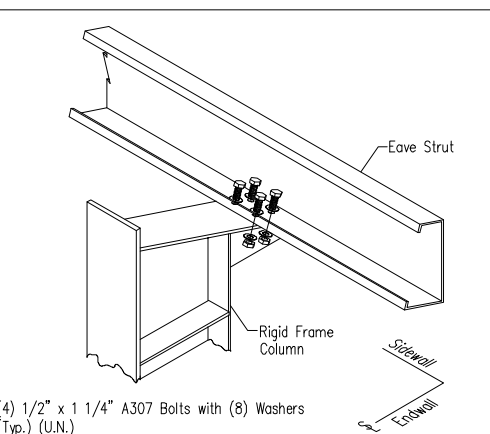
H10 PORTAL FRAME TO RIGID FRAME COLUMN



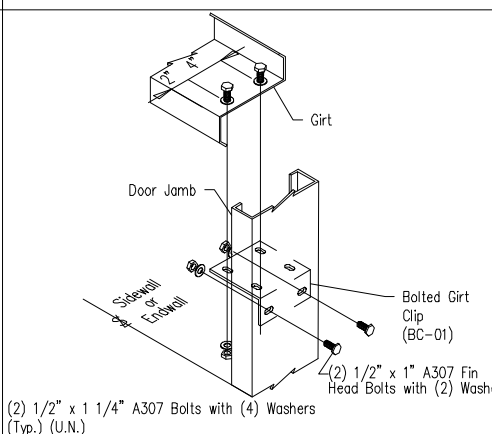
I8 EAVE STRUT TO ENDWALL RAFTER



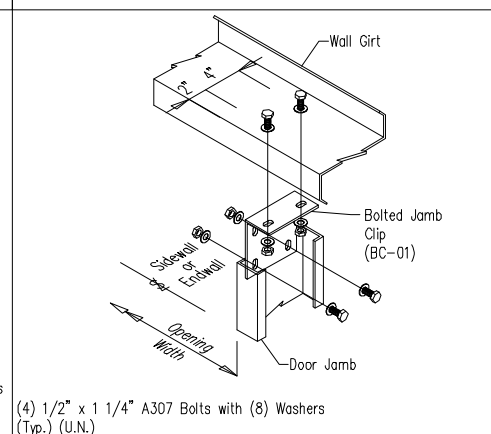
J2 LOWSIDE EAVE STRUT TO BYPASS RIGID FRAME



J24 LOWSIDE EAVE STRUT TO BYPASS RIGID FRAME



K2 WALL GIRTS TO DOOR JAMB

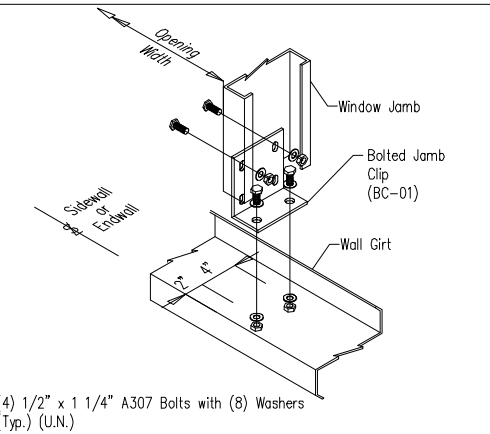


L6 DOOR JAMB TO WALL GIRTS

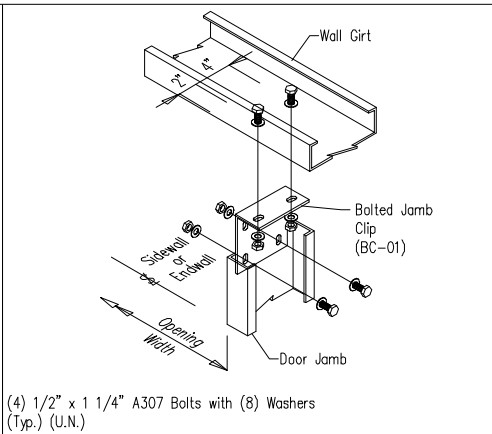
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ

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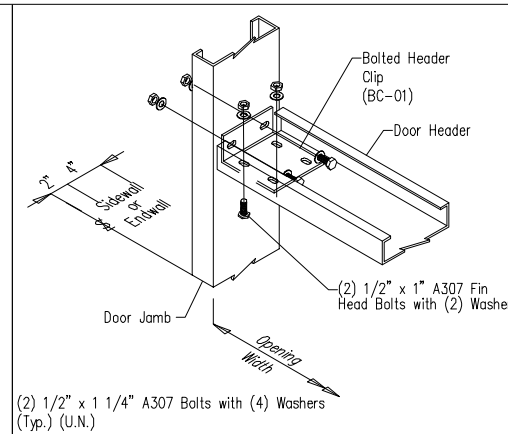
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57243	ENG. BY: MZ DATE: 11/19/21
	DWG. NO.: 11 OF 14 ISSUE: C



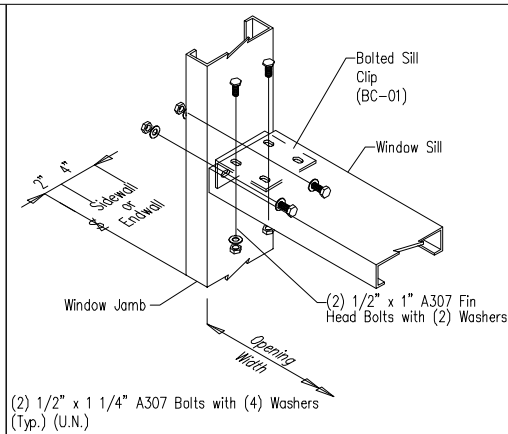
L7 WINDOW JAMB TO WALL GIRT



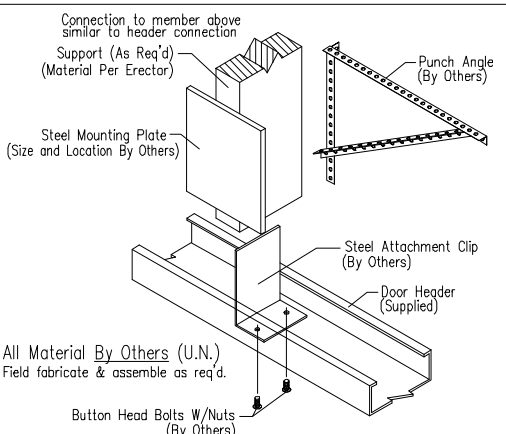
L8 DOOR JAMB TO WALL GIRT



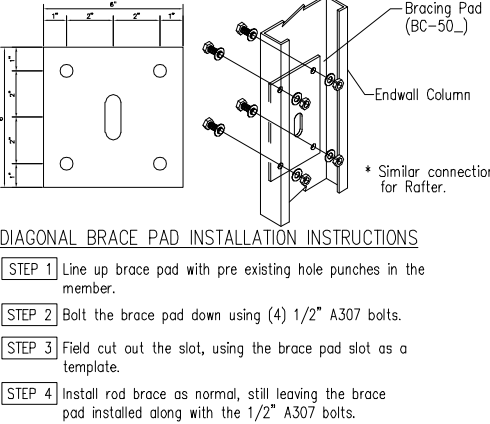
M1 HEADER TO JAMB



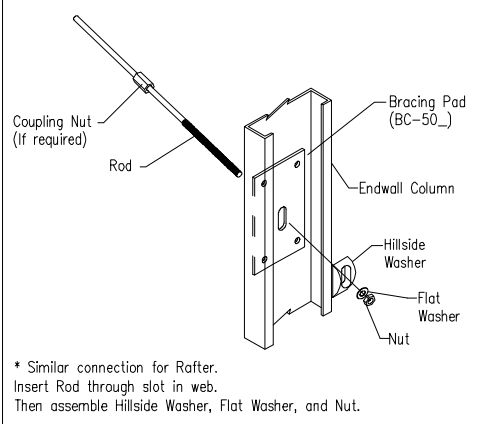
M2 SILL TO JAMB



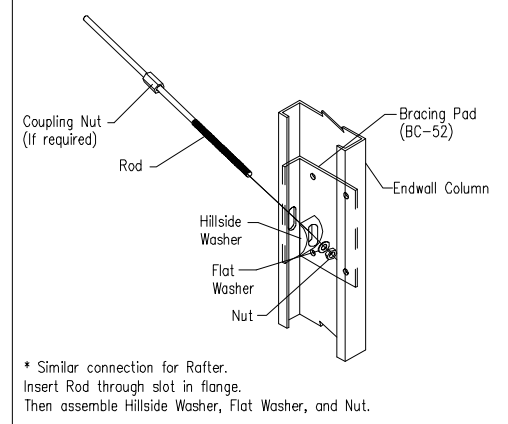
OH O. H. DOOR TORSION BAR BEARING SUPPORT



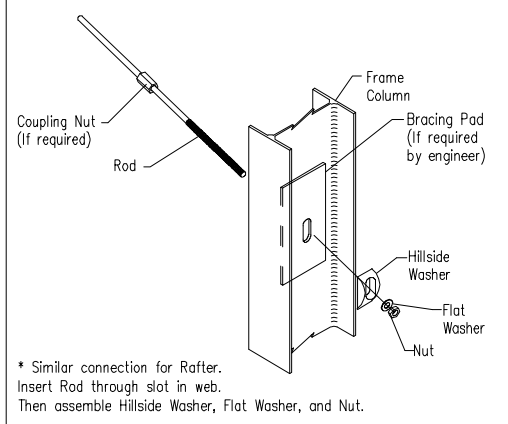
Q3 DIAGONAL BRACE PAD TO WEB OF CEE COLUMN



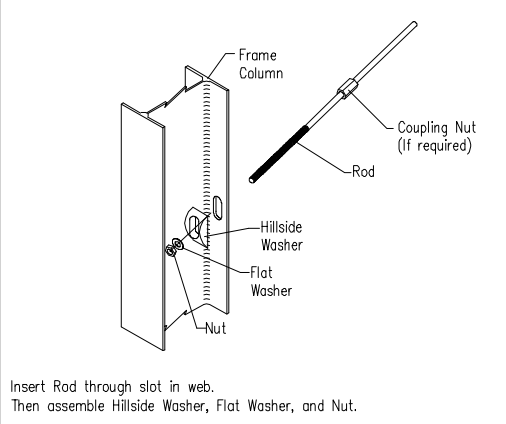
U2 DIAGONAL ROD BRACE TO WEB OF CEE COLUMN



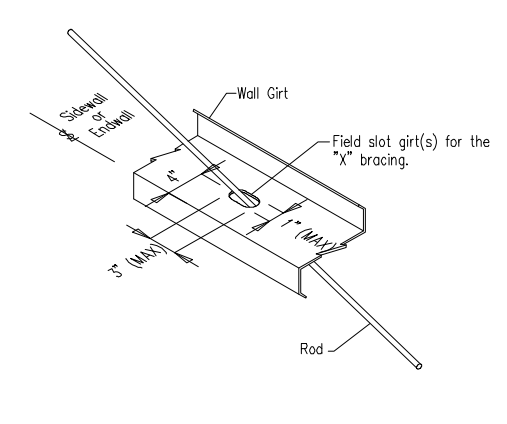
U3 DIAGONAL ROD BRACE TO FLANGE OF CEE COLUMN



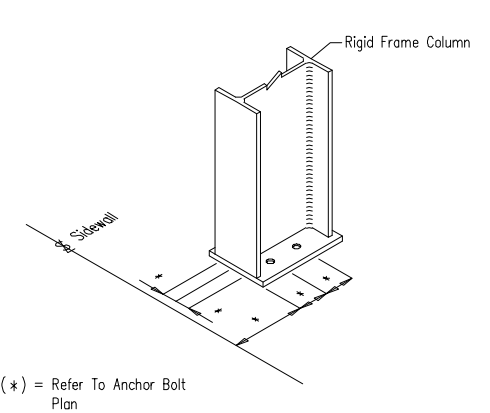
W4 DIAGONAL ROD BRACE TO WEB OF FRAME COLUMN



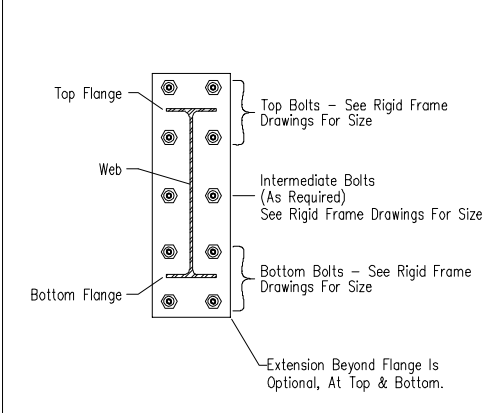
W4 DIAGONAL ROD BRACE TO FLANGE OF FRAME COLUMN



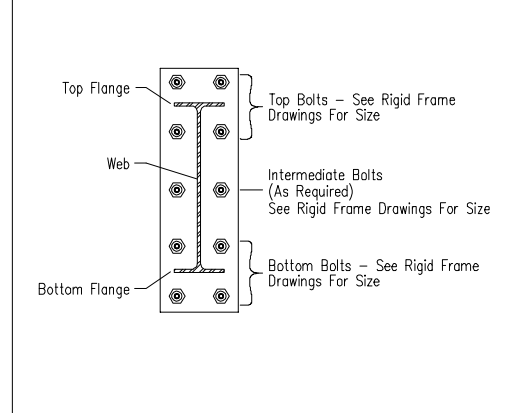
W4 DIAGONAL ROD BRACE AT FLUSH WALL GIRT



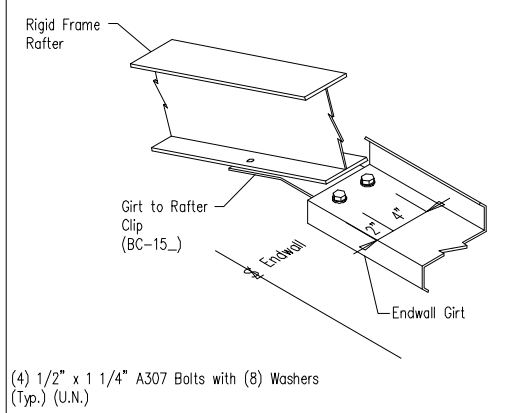
R2 ANCHOR BOLTS AT SIDEWALL COLUMNS



U2 BOLTS FOR RIGID FRAME RAFTER AT BUILDING PEAK



U3 BOLTS FOR RIGID FRAME RAFTER TO COLUMN CONNECTION

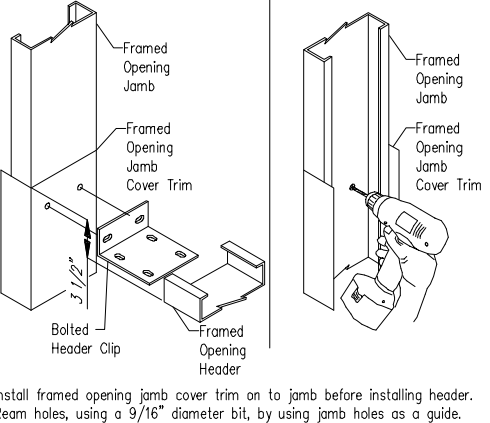
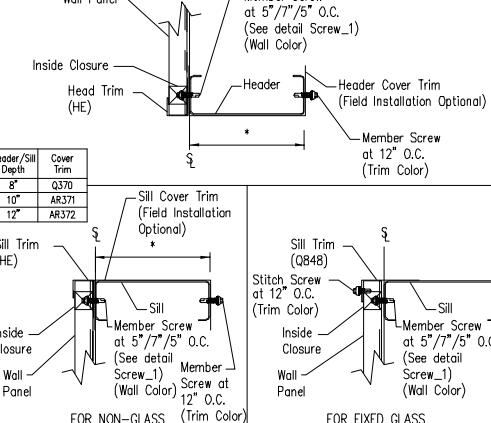
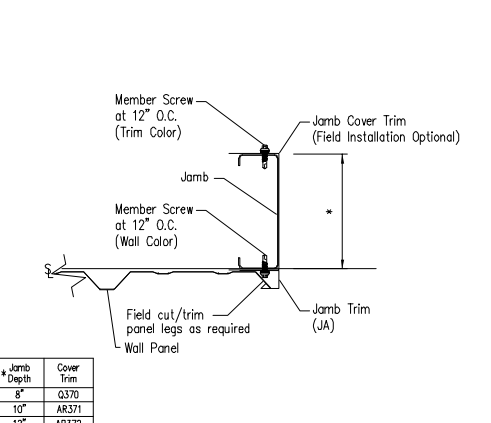
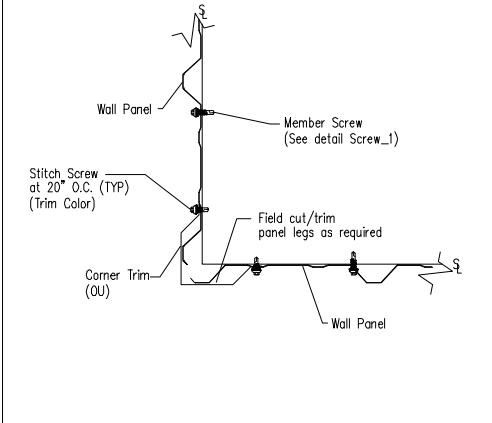
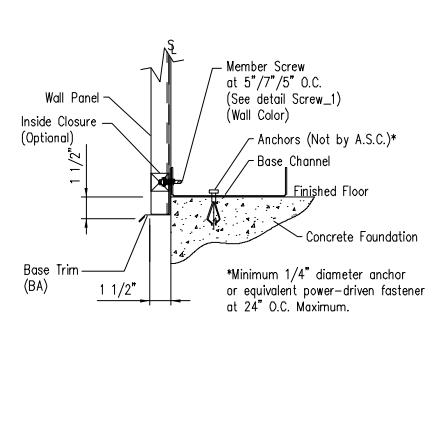


W4 ENDWALL GIRT TO RIGID FRAME RAFTER

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ

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END USER	
SCALE	NOT TO SCALE
JOB NO.: 57243	ENG. BY: MZ
	DATE: 11/19/21
	DWG. NO.: 12 OF 14
	ISSUE: C



BASE CHANNEL DETAIL WITH TRIM

TRIM_2

OUTSIDE CORNER DETAIL

TRIM_30

FRAMED OPENING JAMB TRIM DETAIL

TRIM_50

FRAMED OPENING HEAD & SILL TRIM DETAILS

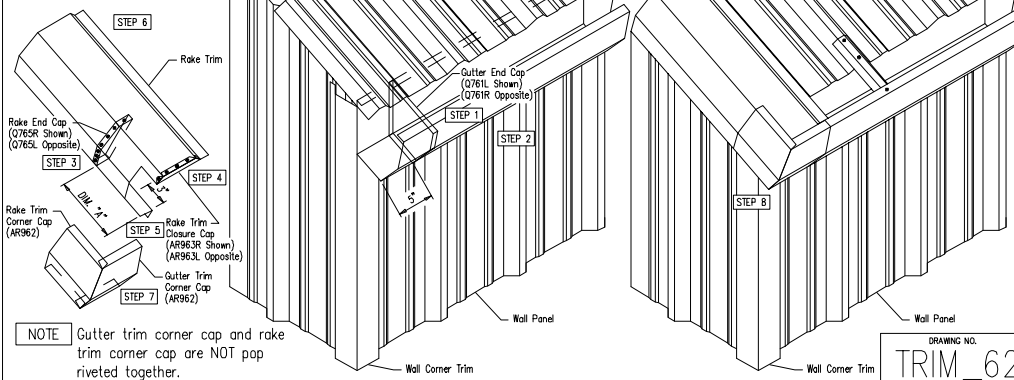
TRIM_51

COVER TRIM INSTALLATION INSTRUCTIONS

TRIM_52

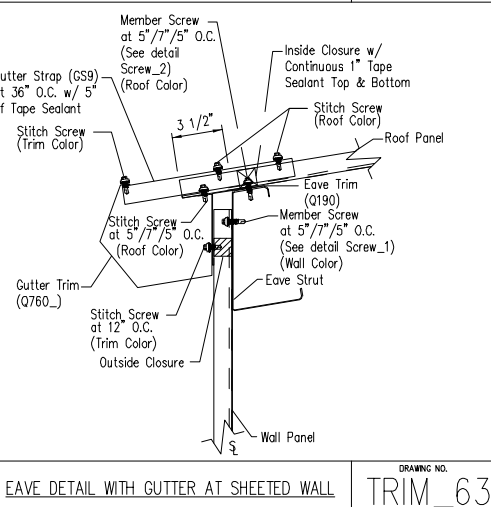
- GUTTER CORNER TRIM INSTALLATION INSTRUCTIONS**
- STEP 1** Install gutter end cap, 5" into gutter trim, using (14) pop rivets. If done correctly the gutter end cap will be below the low leg of the rake trim that sits on top of the roof panel.
 - STEP 2** Install gutter trim on bottom of the roof panel with the back of the gutter flush against the wall panel. Be sure the end of the gutter trim is flush with the wall corner trim.
 - STEP 3** Install rake end cap, into rake trim using (8) pop rivets. Use chart to determine how far the rake end cap is positioned into the rake trim.
 - STEP 4** Install rake trim closure cap, flush with the end of the rake trim using (5) pop rivets.
 - STEP 5** Field cut/notch the face of the rake trim by 3". This is to prevent the rake trim from sticking out past the gutter trim upon final assembly.
 - STEP 6** Install rake trim. Be sure the end of the rake trim with the rake trim closure cap slides into the gutter trim.
 - STEP 7** Install the gutter trim corner cap to the gutter trim using (7) pop rivets. Repeat for rake trim.
 - STEP 8** Field cut/notch the bottom legs of the rake trim. Horizontal leg flush with the gutter trim corner cap. Vertical leg flush with the wall corner trim.

Slope	Dim. "A"
1/2 : 12	6 3/4"
1 : 12	7"
2 : 12	7 1/2"
3 : 12	8"
4 : 12	8 9/16"
5 : 12	9 1/16"
6 : 12	9 5/8"



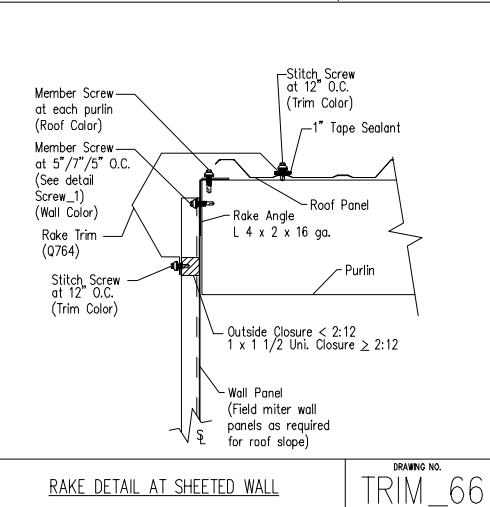
NOTE Gutter trim corner cap and rake trim corner cap are NOT pop riveted together.

TRIM_62



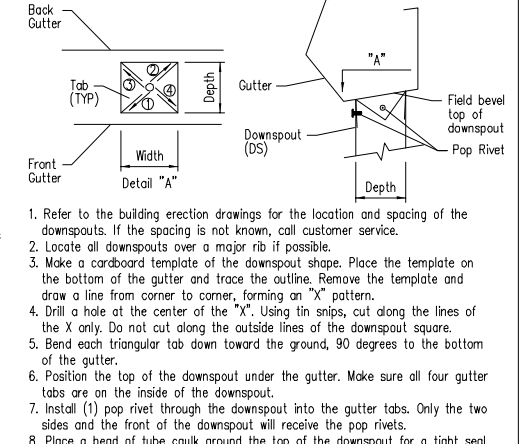
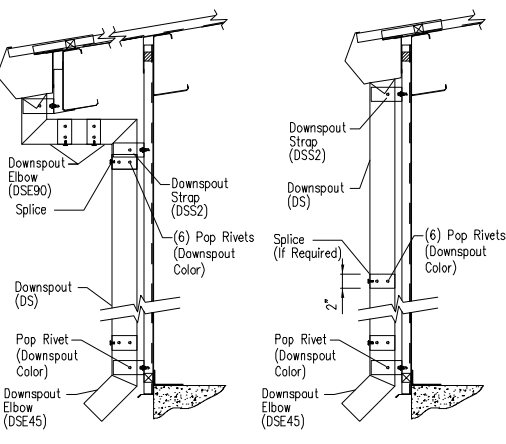
EAVE DETAIL WITH GUTTER AT SHEETED WALL

TRIM_63



RAKE DETAIL AT SHEETED WALL

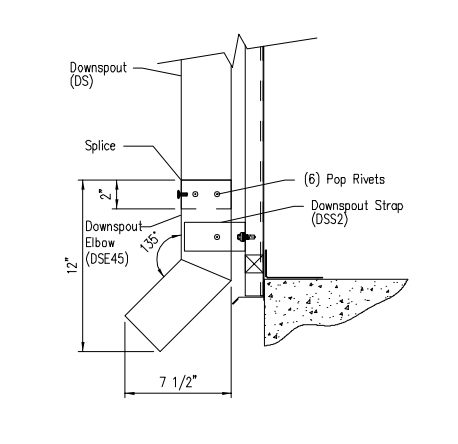
TRIM_66



- Refer to the building erection drawings for the location and spacing of the downspouts. If the spacing is not known, call customer service.
- Locate all downspouts over a major rib if possible.
- Make a cardboard template of the downspout shape. Place the template on the bottom of the gutter and trace the outline. Remove the template and draw a line from corner to corner, forming an "X" pattern.
- Drill a hole at the center of the "X". Using tin snips, cut along the lines of the X only. Do not cut along the outside lines of the downspout square.
- Bend each triangular tab down toward the ground, 90 degrees to the bottom of the gutter.
- Position the top of the downspout under the gutter. Make sure all four gutter tabs are on the inside of the downspout.
- Install (1) pop rivet through the downspout into the gutter tabs. Only the two sides and the front of the downspout will receive the pop rivets.
- Place a bead of tube caulk around the top of the downspout for a tight seal.

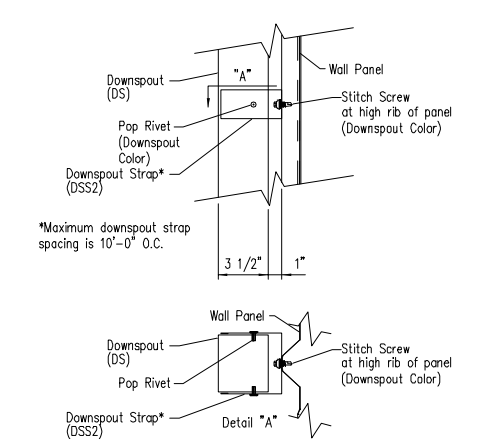
STANDARD DOWNSPOUT TO GUTTER ATTACHMENT DETAIL

TRIM_90



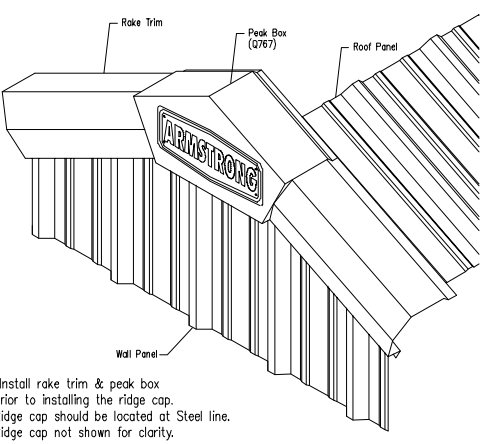
3 1/2" X 4" DOWNSPOUT ELBOW DETAIL

TRIM_91



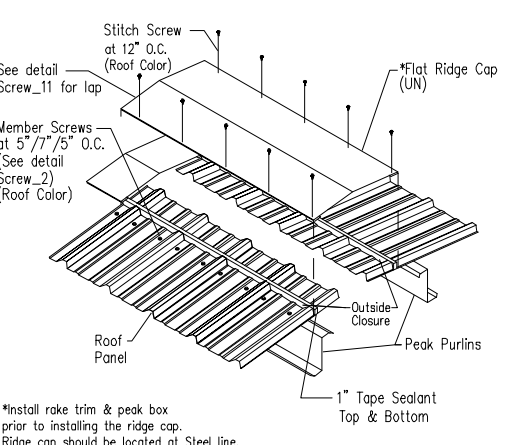
3 1/2" X 4" DOWNSPOUT STRAP DETAIL

TRIM_92



STANDARD PEAK BOX DETAIL

TRIM_100



*Install rake trim & peak box prior to installing the ridge cap. Ridge cap should be located at Steel line.

RIDGE DETAIL WITH FLAT RIDGE

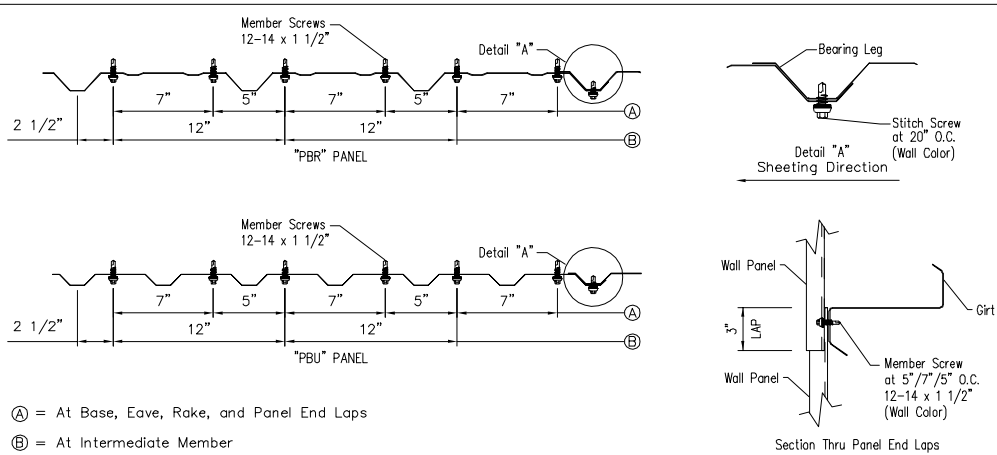
TRIM_101

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ



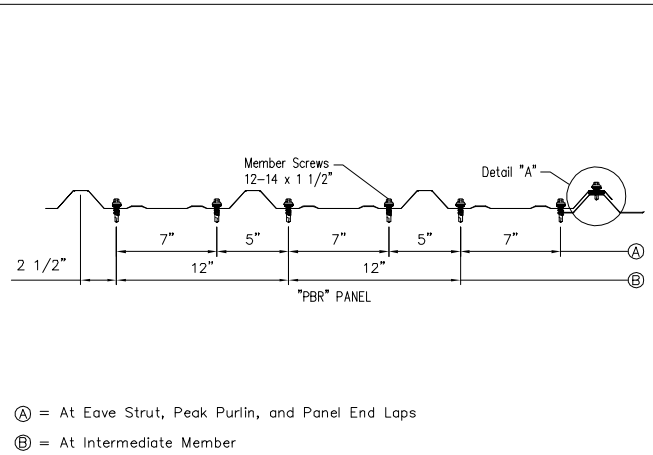
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DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	I
END USER	I
SCALE	NOT TO SCALE
JOB NO.: 57243	ENG. BY: MZ
	DWG. NO.: 13 OF 14
	DATE: 11/19/21
	ISSUE: C



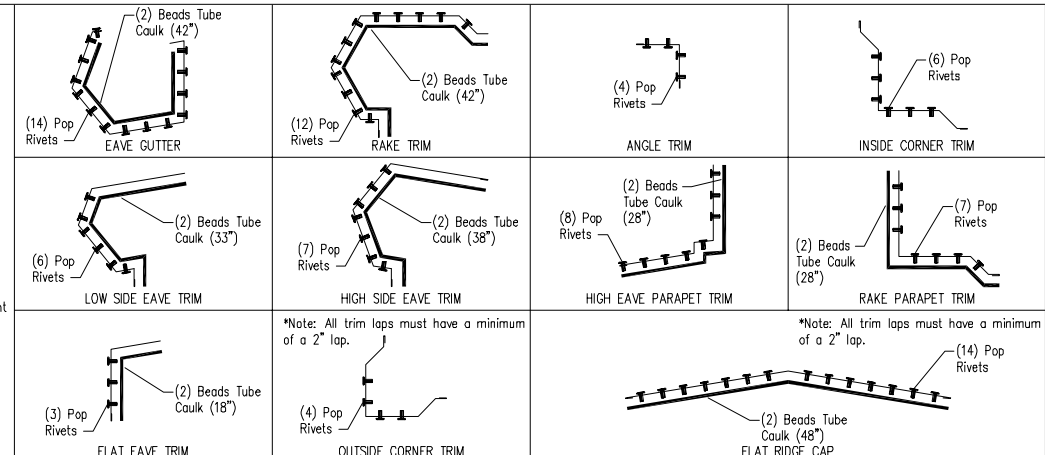
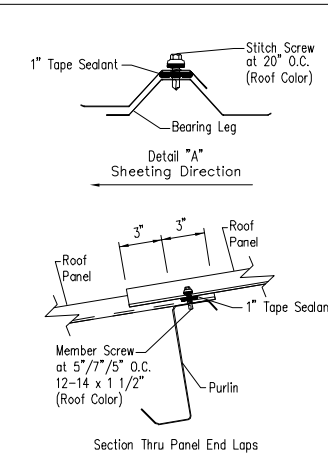
FASTENER LOCATION FOR WALL PANELS

DRAWING NO. SCREW_1



FASTENER LOCATION FOR ROOF PANELS

DRAWING NO. SCREW_2

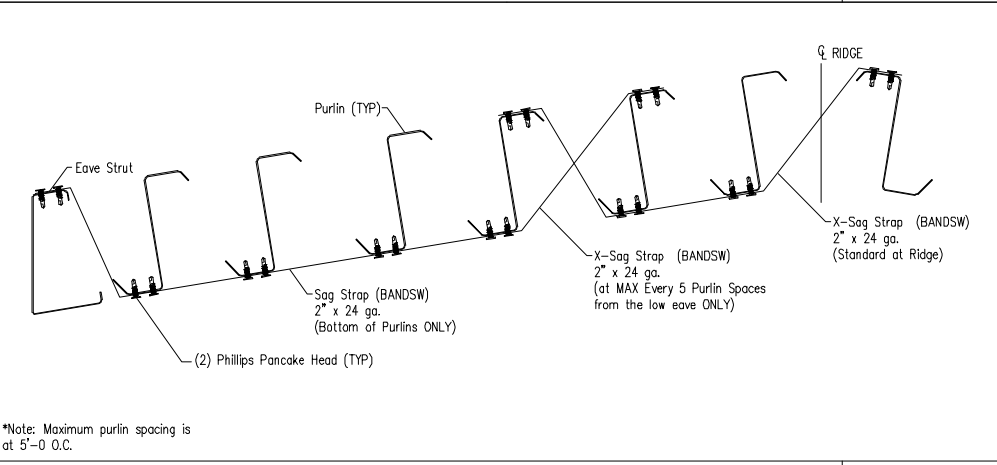


TRIM LAPS

DRAWING NO. SCREW_10

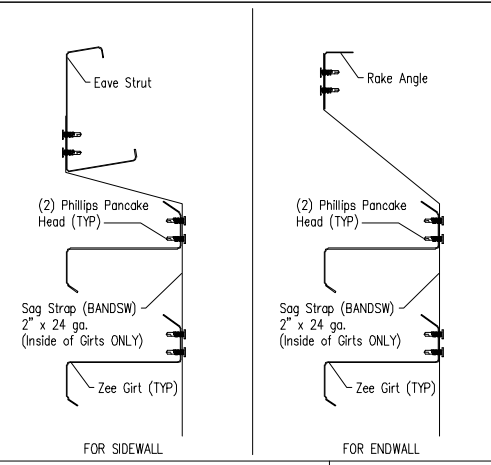
TRIM LAPS

DRAWING NO. SCREW_11



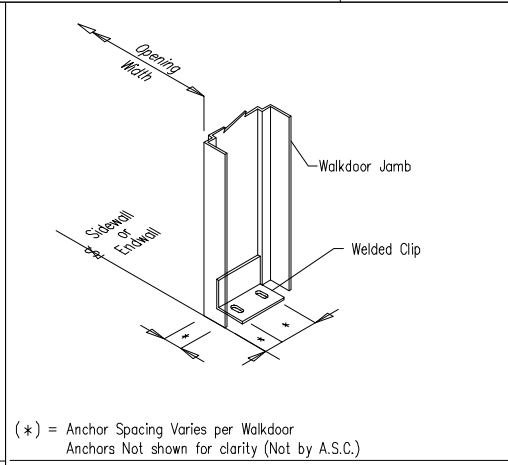
TYPICAL SAG STRAP AT GABLED ROOF

DRAWING NO. SCREW_15

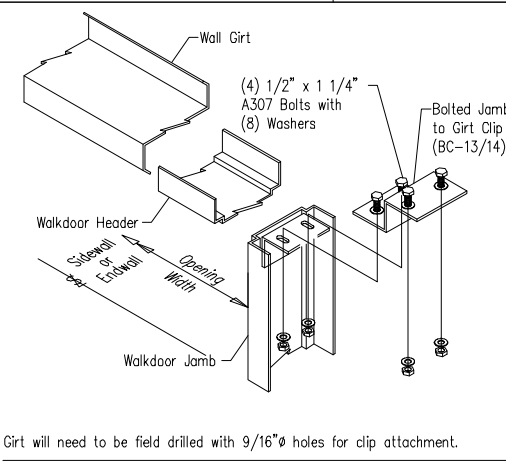


TYPICAL SAG STRAP AT WALLS

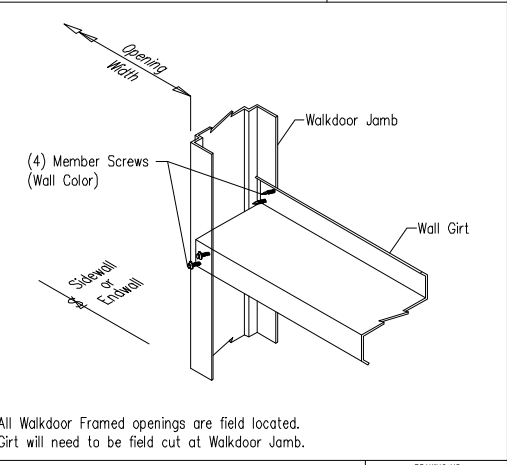
DRAWING NO. SCREW_17



BASE DETAIL FOR WALKDOOR JAMB



WALKDOOR JAMB TO WALL GIRTS



WALL GIRTS TO WALKDOOR JAMB

DRAWING NO. ACC_9

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	11.19.21	MN	DP	MZ
C	CONSTRUCTION	01.16.23	KAC	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57243	ENG. BY: MZ
	DWG. NO.: 14 OF 14
	DATE: 11/19/21
	ISSUE: C