GENERAL NOTES

Fabrication shall be in accordance with A.S.C. standard practices in compliance with the applicable sections, relating to design requireme and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	Hot Rolled Steel Shapes (W, & C)	A572	Fy = 50 KSI
	Hot Rolled Steel Angles (L)	A36	Fy = 36 KSI
	Steel Pipes	A500	Fy = 42 KSI
	Structural Tubing	A500	Fy = 42 KSI
	Structural Steel Web Plate	A572/A1011	Fy = 50 KSI
	Structural Steel Flange Plates/Bars	A529/A572	Fy = 55 KSI
	Cold Formed Light Gage	A653/A1011	Fy = 55 KSI
	Roof and Wall Sheets	A792/A653	Fv = 50, 80 KSI
	Cable Brace	A475´ – TYPE 1	Extra High Strength
	Rod Brace	A529	Fy = 50 KSI
			MINI TENCH E CEDENCE

Machine Bolts & Nuts High Strength Bolts (1"ø and less) High Strength Bolts (>1"ø to 1 1/2"ø) Anchor Bolts (Not supplied by A.S.C.) Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI A325-TYPE 1 A325-TYPE 1 A36/A307/F1554

PRIMER

PRIMER
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is A.S.C. Gray Oxide color. This paint is not intended for long term exposure to the elements. A.S.C. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. A.S.C. shall not be responsible for any field applied paint and/or coatings. (AISC Code of Standard Practice, Latest Edition).

Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.

GALVANIZED OR SPECIAL COATINGS: See Contract Documents

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :

a) Endwall rafter splice - 5/8" ø x 0"-1 3/4" A325-N
b) Endwall column to rafter connection - 1/2" ø x 0"-1 1/4" A325 MIN.(SEE WALL ELEVATION)
c) Main frame connections - SEE CROSS SECTION

d) Flange Brace connections - 1/2"ø x 0'-1 1/4" A325

NOTE: Washers are not supplied unless noted otherwise on drawing

1.6 A325 BOLT TIGHTENING REQUIREMENTS

All high strength bolts are A325—N unless specifically noted otherwise.

Holes are not slotted and design is bearing connection.

Structural bolts shall be tightened by the turn-of-the-nut method in accordance with the Latest Edition AISC "Specification For Structural Joints" using ASTM A325 or A490 Bolts, when specifically required. A325—N bolts are supplied without washer unless otherwise noted on the drawings.

All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.

CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE— Under roof panels & base of wall panels
OUTSIDE — Between roof panels & ridge cap
— Between wall panels & eave/gable trim

<u>ERECTION NOTE:</u>
All bracing, strapping, & bridging shown and provided by A.S.C. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

Analysis Procedure Used

Other Loads/Requirements

1.9 ERECTION AND UNLOADING NOT BY A.S.C.

1.10 SHORTAGES

Any claims or shortages by buyer must be made to A.S.C. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
Claims for correction of alleged misfits will be disallowed unless A.S.C. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of A.S.C.

BUYER/END USE CUSTOMER RESPONSIBILITIES

2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release A.S.C. to fabricate upon receiving such.

2.2 Armstrong Steel Corp (hereafter referred to as A.S.C.) standard specifications apply unless stipulated otherwise in the Contract Documents. A.S.C. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.

2.3 In case of discrepancies between A.S.C. structural steel plans and plans for other trades, A.S.C. plans shall govern. (Section. 3 AISC Code of Standard Practices, Latest Edition)

Approval of A.S.C. drawings and calculations indicates that A.S.C. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the A.S.C. design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)

Once the BUYER/END USE CUSTOMER has signed A.S.C. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and

2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by A.S.C. and A.S.C. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or A.S.C. assumptions will govern (AISC Code of Standard Practice, Latest Edition)



PHONE: 800-345-4610 www.armstrongsteel.com

JOB NO.: 57249

CUSTOMER: END USER: END USE : LOCATION PH. NO.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY A.S.C. IS INCLUDED.

THAN ARMSTRONG ARE SPECIFICALLY

FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER

DESIGN LOADS:			BUILDING	<u>DESCRIPTI</u>	ON:
Design Code / Wind Code Building Risk Category Enclosure	: IBC-18 : II-Normal : Closed		Width (ft) Length (ft) Eave Ht. c		: 60 : 100 t) : 16
Dead Load (psf)	: 2.50		Eave Ht. c	ıt FSW (fi	r) :16
Collateral Load (psf) Wind Load	: 8.00		Roof Slope		: 4.0:12 : 4.0:12
Ultimate Wind Speed, (Vult) (mph)	:142.00		Roof Slope Bay Spacii		
Wind Exposure	: C				
Internal Pressure Coefficient, GCpi	: 0.18/-0.18		COVERING		
Wall Panel Design Wind Pressure (psf)	: 47.00/-50.90		Roof Pane		
Live Load			Panel T Panel C	, ,	: 26 Ga. R—Loc : Galvalume Plus
Primary Framing (psf)	: 20.00		Trim Co		. Galvalattie Flas
Trib. Area Reduction	: No				m :Galvalume Plus
Secondary Framing (psf)	: 20.00			, ==	
Snow Load	. 0. 00		Wall Panel	& Trims	
Ground Snow Load, Pg (psf) Roof Snow Load, Pf (psf)	: 0.00 : 0.00		Panel T		:26 Ga. R-Loc
Sloped Roof Snow Load, Ps (psf)	: 0.00		Panel C		:Galvalume Plus
Snow Exposure Factor, Ce	: 1.00		Trim Co		0.1.1. 51
Snow Importance Factor, Is	: 1.00			er Trims	
Thermal Factor, Ct	: 1.00			ing Trims Trim	: Galvalume Plus : Galvalume Plus
Sloped Factor, Cs	: 1.00		buse	111111	. Garvaranne i lus
Seismic Load					
Seismic Importance Factor, le	: 1.00				
Site Class	:D	.01 _	0.019		
Mapped Spectral Response Acceleration Spectral Response Coefficients	: Ss = 0.060 : Sds = 0.063	:S1 = :Sd1 =			
Seismic Design Category	: A	.501 –	0.027		
Basic Force Resisting Systems Used	:Steel System Detailed For R :Rigid Frames :Braced Frames	Resistanc (OMF)	e		
Total Design Base Shear, V (kips)	:Longitudinal = :Transverse =		•		
Response Modification Factors, R	:Rigid Frames :SW X—Bracing		$\Omega = 3.0$ $\Omega = 3.0$		
Seismic Response Coefficient, Cs	:Rigid Frames :SW X—Bracing			IMPI STE REC	LING OF THIS DRAWING DOES NOT LY OR CONSTITUTE THAT ARMSTRONG EL ENGINEER IS THE ENGINEER OF ORD OR THE DESIGN PROFESSIONAL

: Equivalent Lateral Force Procedure

<u>Drawing Index</u>	
<u>Drawing Name</u>	Page(s)
Drawing Cover	
3D Reference	3D REF.
Anchor Bolt Plan	1
Anchor Bolt Details	2
Anchor Bolt Reactions	3
Rigid Frame	4
Front Sidewall	5
Back Sidewall	6
Left Endwall	7
Right Endwall	8
Roof Plan	9
Details	10-12

BUYER/END USE CUSTOMER RESPONSIBILITIES CONTINUED

2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that A.S.C. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that A.S.C. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished

2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with A.S.C. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (AISC Code of Standard Practice,

2.9 Armstrong Steel Corp is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the

concrete.

Unless otherwise provided in the Order Documents, A.S.C.
does not design and is not responsible for the design, material
and construction of the foundation or foundation embedments. The
END USE CUSTOMER should assure himself that adequate provisions are made
in the foundation design for loads imposed by column reactions of the building,
other imposed loads, and bearing capacity of the soil and other conditions of the
building site.
It is recommended that the anchorage and foundation of the building be
designed by a Registered Professional Engineer experienced in the design
of such structures. (Latest MBMA Low Rise Building Systems Manual)

2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to A.S.C. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (AISC Code of Standard Practice Latest Edition)

1 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (AISC Code of Standard Practice Latest Edition)

2.12 <u>WARNING</u> In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto Galvalume should be avoided.

2.13 <u>SAFETY COMMITMENT</u> Armstrong Steel Corp has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of A.S.C.

It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should diways be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees.

Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.

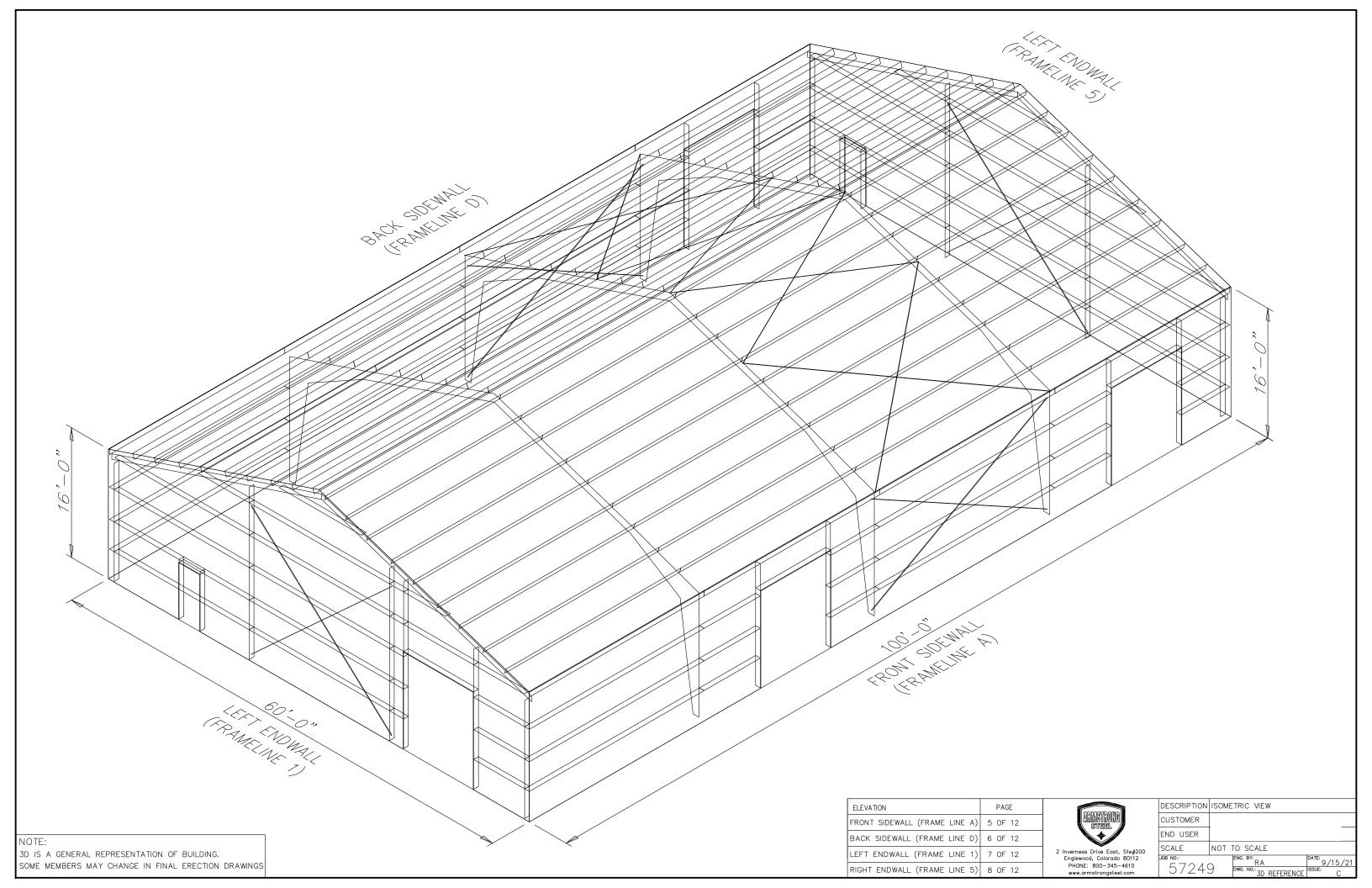
Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.

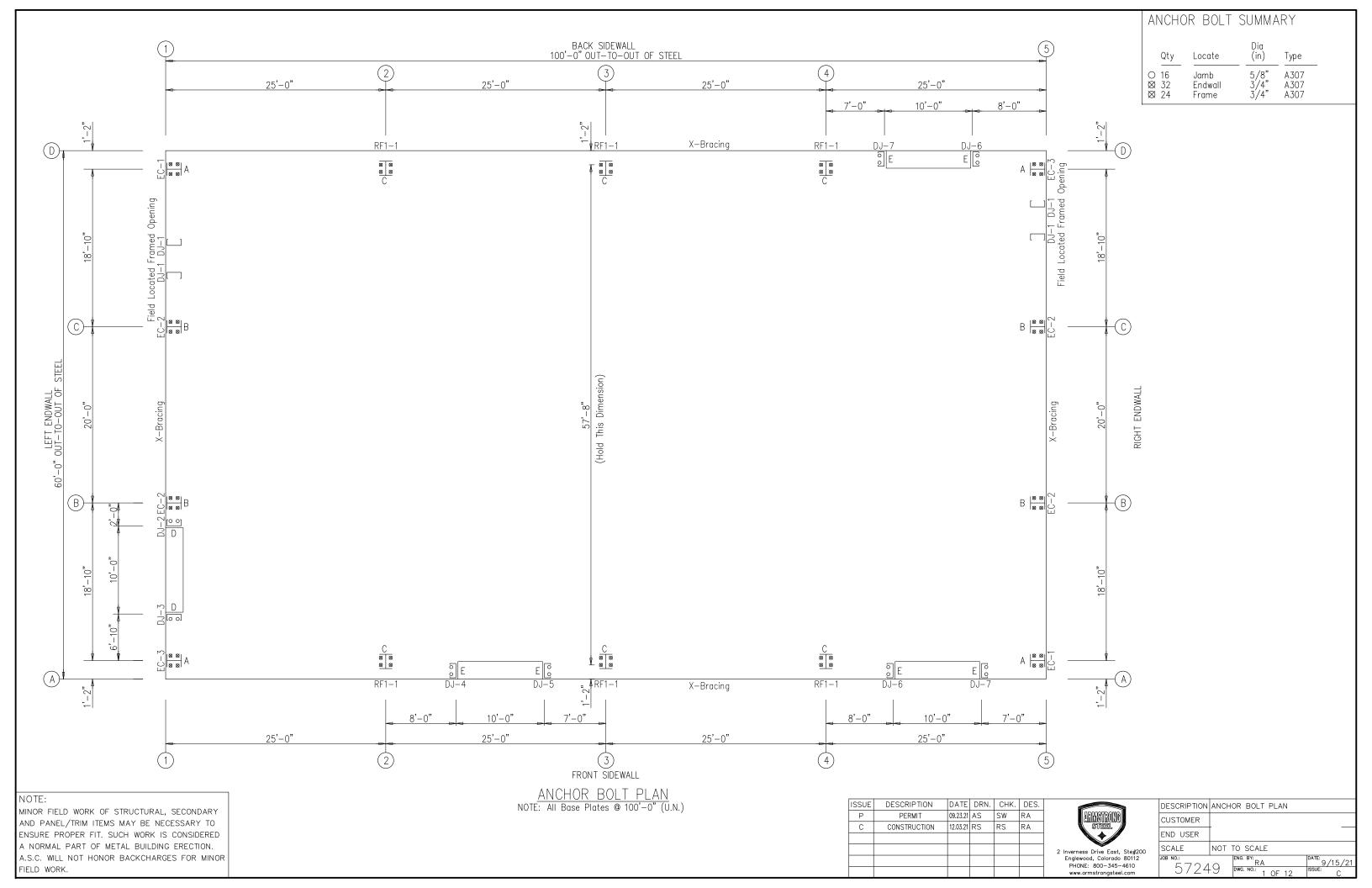
2.15 It is recommended by Factory Mutual (Reference: B2.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.

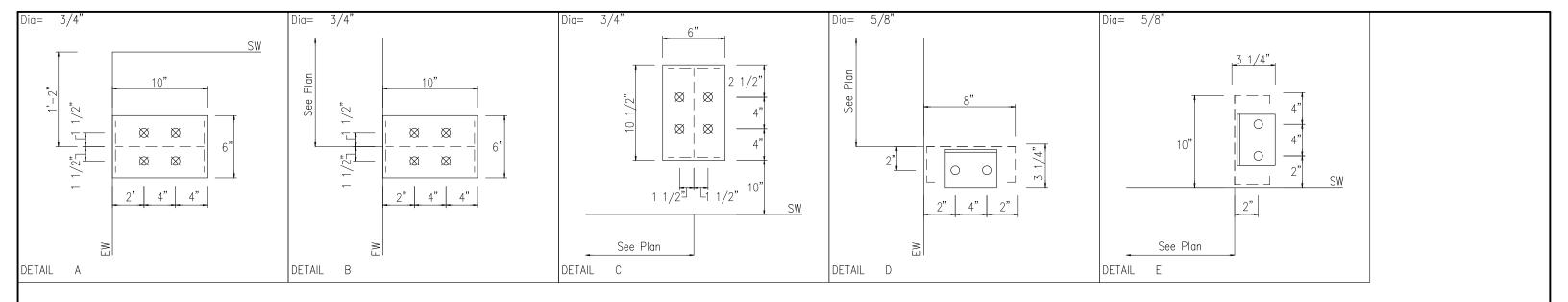
ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20
NOTE: For Snow/Ice Rem	oval Procedure, Refer to Metal Buildi	ing System Manual 2002 Edition,

	80	24.40	12.20
	NOTE: For Snow/Ice Rem- Section A8.4, Page	oval Procedure, Refer to Metal Buildi XI-A8-2,	ng System Manual 2002 Edition,
D	rawing Status	·	
	final, and purpose is documents.	rings, being for approve are for conceptual repr	
	final. Only	REVISED Ings, being for permit, drawings issued "Constas complete."	PERMIT: are by definition not ruction" can be
	CONSTRUCT Final drawi		erection of the building.

JOB NO : 57249







NOTE:

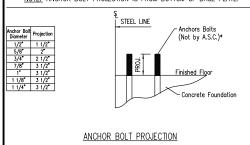
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION.

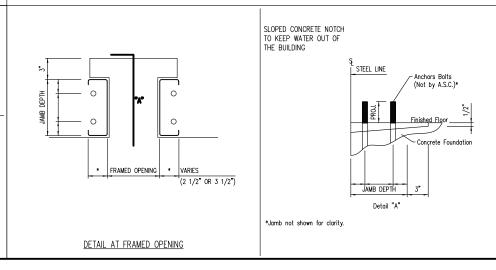
A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING MANUFACTURER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

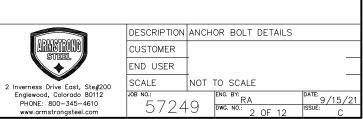
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED MAD REPORTED BY OTHERS.

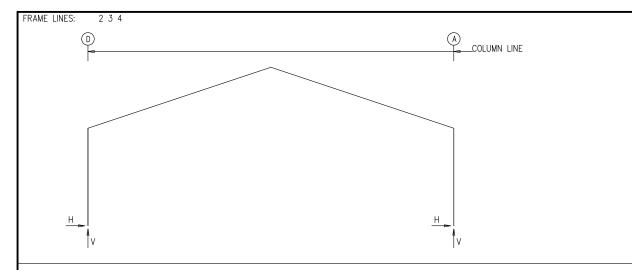
NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
Р	PERMIT	09.23.21	AS	SW	RA
С	CONSTRUCTION	12.03.21	RS	RS	RA





H		FRAME:			DELOTION		LIOD DOLT		- DI 1 T					
T,	NGID	FINAIVIE.		MAXIMUM	REACTION	is, anci	HOK BOLI:	S, & BAS	E PLAIE	:5				
	Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id) Hmin H	V Vmin	Bolt Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Grout (in)
	2*	D	1	12.8	24.1	2 4	-9.0 -0.9	-13.7 -15.0	4	0.750	6.000	10.50	0.500	0.0
	2*	Α	3 1	9.0 -12.8	-13.7 24.1	1 5	-12.8 0.9	24.1 -15.0	4	0.750	6.000	10.50	0.500	0.0
	2*	Frame lin	ies:	2 3 4										

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft) = 60.00

Length (ft) = 100.00

Eave Height (ft) = 16.00/16.00

Roof Slope (rise/12) = 4.00/4.00

Dead Load (psf) = 2.50

Collateral Load (psf) = 8.00

Live Load (psf) = 20.00

Ultimate Wind Speed (vult) (mph) = 142.00

Wind Code = IBC-18

Exposure = C

Closed/Open = C

Importance Wind = 1.00

Importance Seismic = 1.00

Seismic Zone = A

Seismic Coeff (Fa*Ss) = 0.09

ID Description

1	Dead+Collateral+Live
2	0.6Dead+0.6Wind_Left1
3	0.6Dead+0.6Wind_Right1
4	0.6Dead+0.6Wind_Long1L
5	0.6Dead+0.6Wind_Long2L
6	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
7	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
8	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
9	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
10	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
11	0.6Dead+0.6Wind Suction+0.6Wind Long2L

ANCHO	R BOLT	SUMMA	ιRΥ	
Qty	Locate	Dia (in)	Туре	
○ 16 ⋈ 32 ⋈ 24	Jamb Endwall Frame	5/8" 3/4" 3/4"	A307 A307 A307	

BUILDING BRAC	ING REAC	TIONS			
Wall — Col Loc Line — Line L_EW 1 C,B F_SW A 3,4 R_EW 5 B,C B_SW D 4,3	# Red Wind Horz Ver 3.4 3. 9.4 5. 3.4 3. 9.4 5.	7 0.2 2 0.8 7 0.2	0.2 0.5 0.2 0.5	Panel_ (lb, Wind	

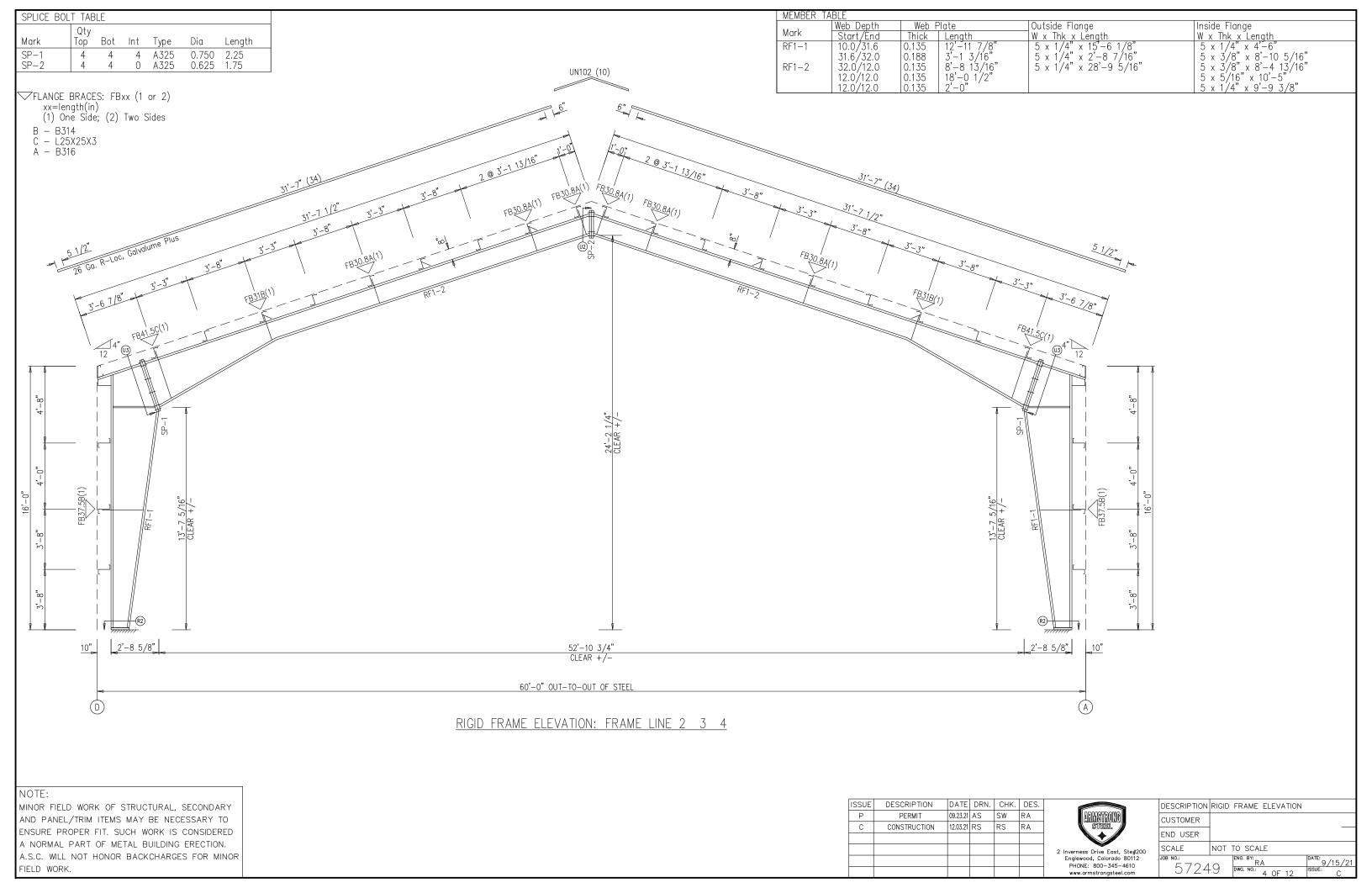
RIGI	D FRAM	ME:	BAS	IC COLUM	N REACT	ONS (k))								
Frame Line 2* 2*	e Column Line D A	Horiz 1.3 –1.3	-Dead Vert 2.8 2.8	Collo Horiz 3.4 -3.4	teral— Vert 6.3 6.3	Horiz 8.1 -8.1	-Live Vert 15.0 15.0	Wind Horiz -16.3 3.6	I_Left1- Vert -25.5 -19.5	-Wind_ Horiz -3.6 16.3	Right1- Vert -19.5 -25.5	Wind Horiz -14.5 1.8	l_Left2- Vert -14.8 -8.7		
Frame Line 2* 2*	e Column Line D A	-Wind_ Horiz -1.8 14.5	Right2- Vert -8.7 -14.8	Wind Horiz -2.8 6.0	_Long1- Vert -27.7 -25.0	Wind Horiz -6.0 2.8	L_Long2- Vert -25.0 -27.7	-Seism Horiz -0.2 -0.2	ic_Left Vert -0.1 0.1	Seismic Horiz 0.2 0.2	_Right Vert 0.1 -0.1	-Seism Horiz 0.0 0.0	ic_Long Vert -0.5 -0.5		
2*	Frame lin	nes:	2 3 4	ļ.											

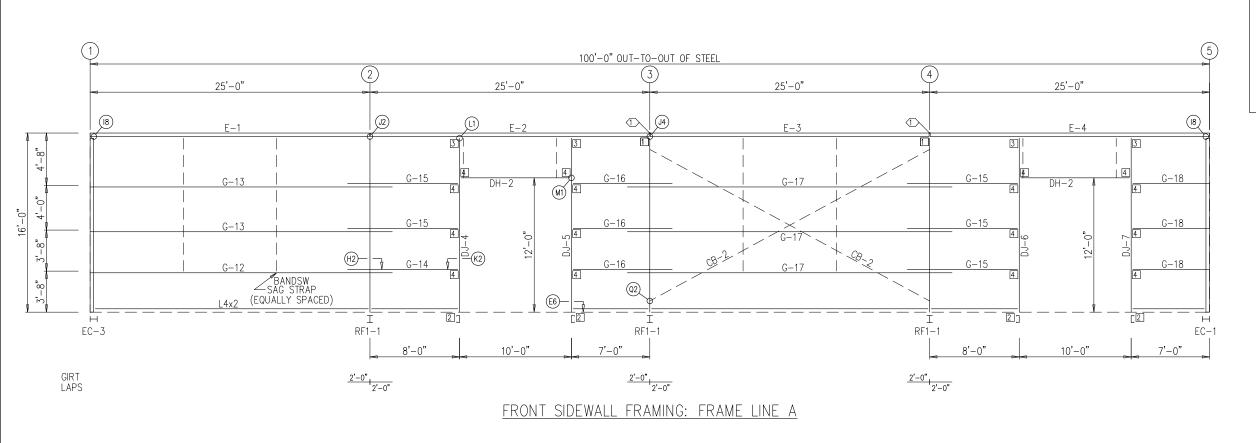
1D W 4	ALL CO	LUMN:		BASIC CO	LUMN R	EACTIONS	S (k)						14 0° 1	Mr. I
ne ((((((Col Dec Line Ver D 0.5 C 1.2 B 1.2 A 0.5 A 0.5 B 1.2 C 1.2 D 0.5	t	Collat Vert 0.9 2.3 2.3 0.9 0.9 2.3 2.3	Live Vert 2.1 5.5 5.5 2.1 2.1 5.5 5.5 2.1	Wind Horz 0.0 -3.4 0.0 0.0 0.0 -3.4 0.0 0.0	_Left1 Ver -4.8 -16.0 -3.6 -4.9 -4.8 -16.0 -3.6 -4.9	t Horz 3 0.0 0.0 0.0 3.4 0.0 0.0 0.0 0.0 3.4 0.0 0.0 3.4 0.0 0.0 0.0 3.4	_Right1 Ver -4.5 -3.0 -16.0 -4.5 -3.0 -16.0 -4.5	rt Hor 9 0.0 6 -3.4 0 0.0 8 0.0 9 0.0 6 -3.4 0 0.0	-2. -12. -0. -2. -2. -12. -12. -12.	rt Hor. 5 0.0 8 0.0 5 3.4 5 0.0 5 0.0 8 0.0 5 3.4	d_Right2 z Ver: -2.5 -0.5 -12.8 -2.5 -0.5 -12.8 -2.5	-2.9 -7.9 -7.9 -2.9 -2.9 -7.9	Wind Suct Horz 3.4 8.7 8.7 3.4 8.7 8.7 3.4
ne [((((((Col Win Line Hou C 0.0 B 2.4 A 0.0 A 0.0 B 0.0 C 2.4 D 0.0	-6. -8. -8. -4. -6. -8.	rt Ho 5 0.0 1 -2. 7 0.0 2 0.0 5 0.0 1 -2. 7 0.0	-4.2 4 -8.7 0 -8.1 0 -6.5 0 -4.2 4 -8.7 0 -8.1	Ho 0. -0 0. 0. -0	0 (.2 -1 0 (0 (0 (.2 -1)) 2 –))	t Vert 0.0 0.3 -0.3 0.0 0.0 0.3 -0.3					
NDW#	ALL CO	LUMN:		MAXIMUM			0.0 0.)	0.0					
Frm Line	ALL CO Col Line	LUMN: Load			REACTIO		0.0 0.	O , & BAS	0.0		e_Plate(in) Length	Thick	Grout (in)	
Frm	Col	Load Id — 6	Coli Hmax H	MAXIMUM umn_React V Vmax -3.6	REACTIO ions(k) Load Id The control 7	NS, ANCI	$0.0 \qquad 0.$ HOR BOLTS $\frac{V}{Vmin}$ $-\frac{3.6}{Vmin}$	O , & BAS Boll	0.0 SE PLATES t(in)	Base	e_Plate(in) Length - 10.00			
Frm Line	Col Line	Load Id - 6 1 8	Coli Hmax H 2.0 0.0 5.2	MAXIMUM umn_React V Vmax -3.6 3.5 -8.8	REACTIO ions(k) Load Id 7 6	Hmin H -1.8 2.0 -4.7	0.0 0. HOR BOLTS V Vmin -3.6 -3.6 -4.5	O & BAS Bolt Qty	0.0 SE PLATES t(in) Dia	Base Width	Length ´		(in)	
Frm Line —	Col Line D	Load Id 6 1 8 1 10	Coli Hmax H 2.0 0.0 5.2 0.0 5.2	MAXIMUM wmn_React Vmax -3.6 3.5 -8.8 9.1 -8.8	REACTIO ions(k) Load Id 7 6 9 8 7	Hmin H -1.8 2.0 -4.7 5.2 -4.7	0.0 0. HOR BOLTS V Vmin -3.6 -3.6 -4.5 -8.8 -4.5	, & BAS Bolt Qty 4	0.0 SE PLATES t(in) Dia 0.750	Base Width 6.000	Length 1 10.00	0.375	0.0	
Frm Line 1	Col Line D	Load Id 6 1 8 1	Coll Hmax H 2.0 0.0 5.2 0.0 5.2 0.0 2.0	MAXIMUM wmn_React v	reactions(k) Load Id 7 6 9 8 7 10 9	Hmin H -1.8 2.0 -4.7 5.2 -4.7 5.2 -1.8	0.0 0. HOR BOLTS V Vmin -3.6 -3.6 -4.5 -8.8 -4.5 -8.8 -3.6	Boli Qty 4	0.0 SE PLATES t(in) Dia 0.750 0.750	Base Width 6.000	10.00 10.00	0.375 0.375	0.0 0.0	
Frm Line 1	Col Line D C	Load Id 6 1 8 1 10 1 11 1 6	Column H 2.0 0.0 5.2 0.0 5.2 0.0 2.0 2.0 2.0 2.0	MAXIMUM V	REACTIONs(k) Load Id 769887110991117	Hmin H -1.8 2.0 -4.7 5.2 -4.7 5.2 -1.8 2.0 -1.8	0.0 0. HOR BOLTS V Vmin -3.6 -3.6 -4.5 -8.8 -4.5 -8.8 -3.6 -3.6 -3.6 -3.6	Bolt Qty 4 4 4	0.0 SE PLATES t(in) Dia 0.750 0.750 0.750	Base Width 6.000 6.000	10.00 10.00 10.00	0.375 0.375 0.375	0.0 0.0 0.0	
Frm Line 1 1 1	Col Line D C B	Load IId 6 1 8 1 10 1 11 1 1	Col Hmax H 2.0 0.0 5.2 0.0 5.2 0.0 2.0 0.0 2.0 0.0 5.2	MAXIMUM vmn_React	REACTIO ions(k) Load Id 7 6 9 8 7 10 9 11 7 6 9	Hmin H -1.8 2.0 -4.7 5.2 -4.7 5.2 -1.8 2.0 -1.8 2.0 -4.7	0.0 0. HOR BOLTS V Vmin -3.6 -3.6 -4.5 -8.8 -4.5 -8.8 -3.6 -3.6 -3.6 -3.6 -3.6 -4.5	Bolt Qty 4 4 4	0.0 SE PLATES t(in) Dia 0.750 0.750 0.750 0.750	Base Width 6.000 6.000 6.000	10.00 10.00 10.00 10.00	0.375 0.375 0.375 0.375	(in) 0.0 0.0 0.0 0.0	
Frm Line 1 1 1 5	D C B A A	Load Id	2.0 0.0 5.2 0.0 5.2 0.0 2.0 0.0 2.0 0.0	MAXIMUM V	REACTIOns(k) Load Id 7 6 9 8 7 10 9 11 7 6	Hmin H 2.0 -4.7 5.2 -4.7 5.2 -1.8 2.0 -1.8 2.0	0.0 0. HOR BOLTS V Vmin -3.6 -3.6 -4.5 -8.8 -4.5 -8.8 -3.6 -3.6 -3.6 -3.6 -3.6	Bolt Qty 4 4 4 4	0.0 SE PLATES t(in) Dia 0.750 0.750 0.750 0.750 0.750 0.750	Base Width 6.000 6.000 6.000 6.000	10.00 10.00 10.00 10.00 10.00	0.375 0.375 0.375 0.375 0.375	(in) 0.0 0.0 0.0 0.0 0.0 0.0	

SSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	
Р	PERMIT	09.23.21	AS	SW	RA	
С	CONSTRUCTION	12.03.21	RS	RS	RA	



DESCRIPTION	ANCHOR BOLT REACTIONS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
јов no.: 5724	PMG. NO.: 7 OF 12 ISSUE: 0



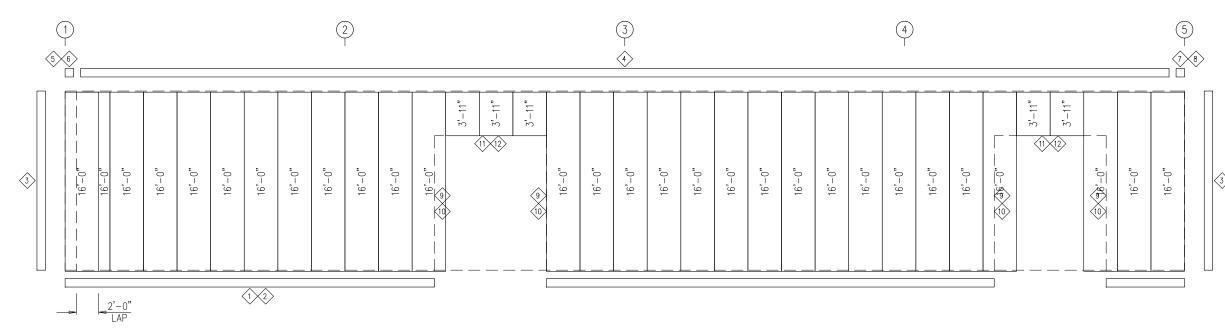


TRIM	TABLE			
FRAM	E LINE	A		
◇ID	QUAN	PART	LENGTH	DETAIL
1	4	BA6	20'-4"	TRIM_1
2	1	BA6102	10'-2"	TRIM_1
3	2 5	006	16'-2"	TRIM_30
4	5	Q7726	20'-4"	TRIM_61
5	1	Q773L6	6"	
6	1	AR961L6	7 7/16"	TRIM_60
7	1	Q773R6	6"	
8	1	AR961R6	7 7/16" 14'–2"	TRIM_60
9	4	AR3716	14'-2"	TRIM_50
10	4	JA6	12'-2"	TRIM_50
11	2	AR371610	10'-2"	TRIM_51
12	2	HF6	10'-3"	TRIM 51

MEMBER	TABLE		
FRAME L	INE A		
QUAN	MARK	PART	LENGTH
1			
	DJ-4	10X25C16	15'-3"
1	DJ-5	10X25C16	15'-3"
1	DJ-6	10X25C14	15'-3"
1	DJ-7	10X25C14	15'-3"
2	DH-2	10X25C16	9'-11"
1	E-1	08536DU4	24'-11"
1	E-2	08536DU4	24'-11"
1	E-3	08536DU4	24'-11"
1	E-4	08536DU4	24'-11"
1	G-12	10X35Z16	26'-11 1/2"
2	G-13	10X25Z14	26'-11 1/2"
1	G-14	10X35Z16	9'-8"
5	G-15	10X25Z16	9'-8"
3	G-16	10X25Z16	8'-8"
3 3 3	G-17	10X25Z16	29'-0"
	G-18	10X25Z16	6'-7 1/2"
2	CB-2	GS1724	29'-3"'

	SPECIAL B	OLTS				
	○ ID	QUAN	TYPE	DIA	LENGTH	WASH
Γ	1	4	A307	1/2"	1 1/4"	2

CONNECTION PLATES						
	FRAME LINE A					
	MARK/PART	QUAN				
	BC-44	2	1			
	BC-05	4	2			
	BC-37E	4	3			
	BC-01	16	4			
	A MARK/PART BC-44 BC-05 BC-37E	E LINE QUAN 2 4 4				



FRONT SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. R-Loc - Galvalume Plus

MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION.

A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISS	SUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
F	>	PERMIT	09.23.21	AS	SW	RA
(0	CONSTRUCTION	12.03.21	RS	RS	RA



DESCRIPTION	SIDEWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.:	ENG. BY: RA DATE: 9/15/21
1 3//4	- Y DWG. NO.: F OF 10 ISSUE:

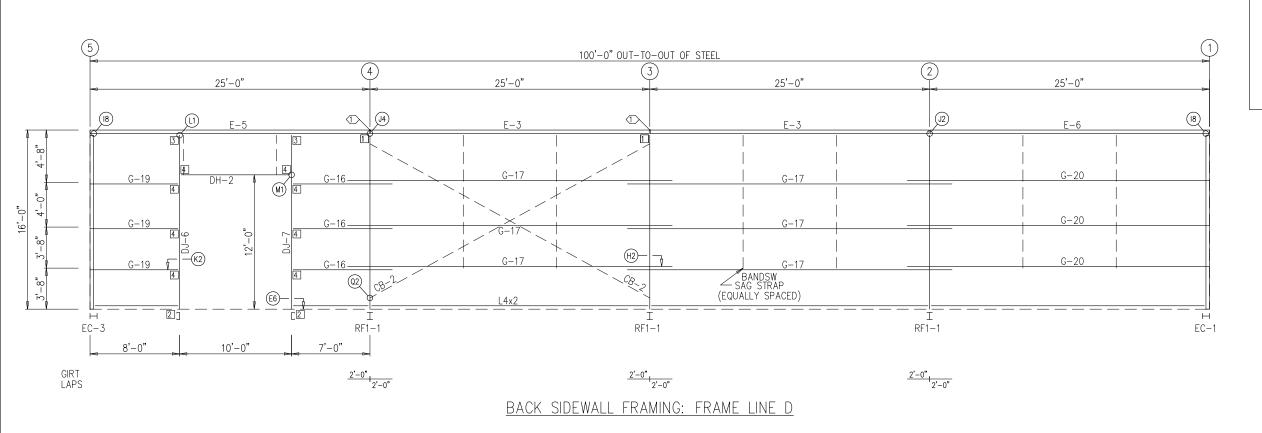
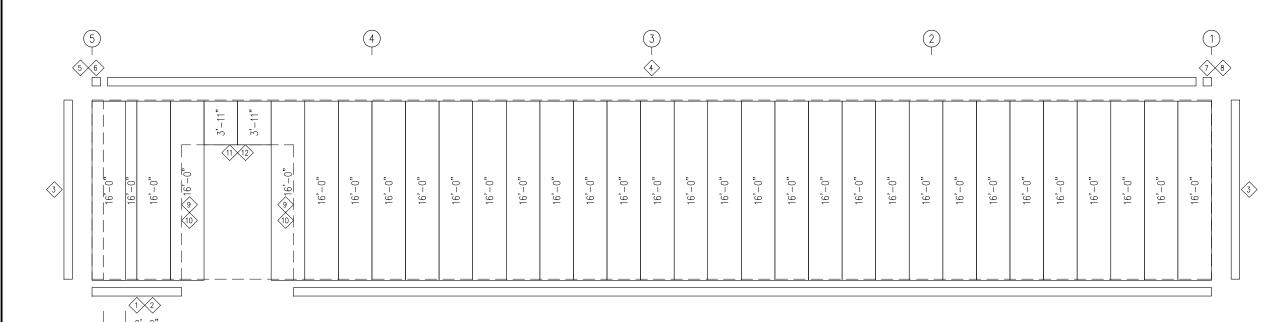


	TABLE	D		
	E LINE	U		
◇ID	QUAN	PART	LENGTH	DETAIL
1	4	BA6	20'-4"	TRIM_1
2	2	BA6102	10'-2"	TRIM_1
2 3	2	006	16'-2"	TRIM_30
4	2 5	Q7726	20'-4"	TRIM_61
5 6	1	Q773L6	16"	_
6	1	AR961L6	7 7/16" 6"	TRIM_60
7	1	Q773R6	6"	
8	1	AR961R6	7 7/16" 14'–2"	TRIM_60
9	2	AR3716	14'-2"	TRIM_50
10	2	JA6	12'-2"	TRIM_50
11	1	AR371610	10'-2"	TRIM_51
12	1	HE6	10'-3"	TRIM_51

MEMBER	TABLE		
FRAME L	INE D		
QUAN	MARK	PART	LENGTH
1	DJ-6	10X25C14	15'-3"
1	DJ-7	10X25C14	15'-3"
1	DH-2	10X25C16	9'–11"
2	E-3	08536DU4	24'-11"
1	E-5	08536DU4	24'-11"
1	E-6	08536DU4	24'-11"
3	G-16	10X25Z16	8'-8"
6	G-17	10X25Z16	29'-0"
3	G-19	10X25Z16	7'-7 1/2"
3	G-20	10X25Z14	26'-11'1/2"
2	CB-2	GS1724	29'-3"

SPECIAL BO	OLTS				
() ()	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A307	1/2"	1 1/4"	2

CONNECTION PLATES					
FRAM	IE LINE I	D			
	QUAN	MARK/PART			
1	2	BC-44			
2	2	BC-05			
3	2	BC-37E			
4	8	BC-01			



BACK SIDEWALL SHEETING & TRIM: FRAME LINE D

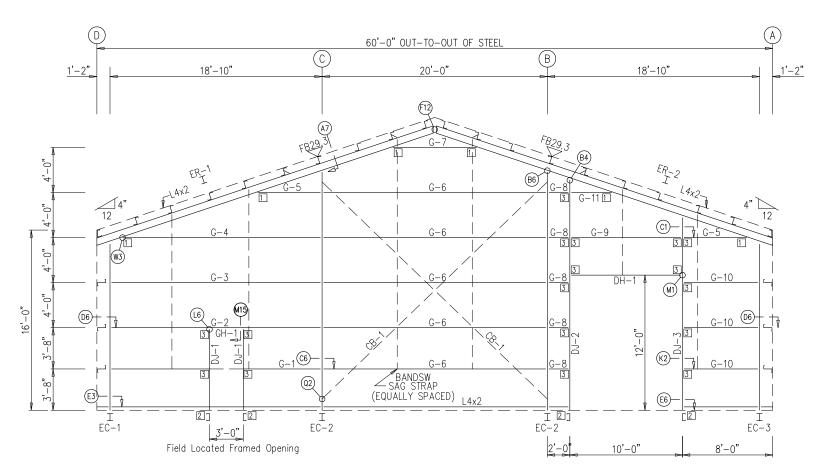
PANELS: 26 Ga. R-Loc - Galvalume Plus

MINOR FIELD WORK OF STRUCTURAL, SECONDARY
AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
A NORMAL PART OF METAL BUILDING ERECTION.
A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
FIELD WORK.

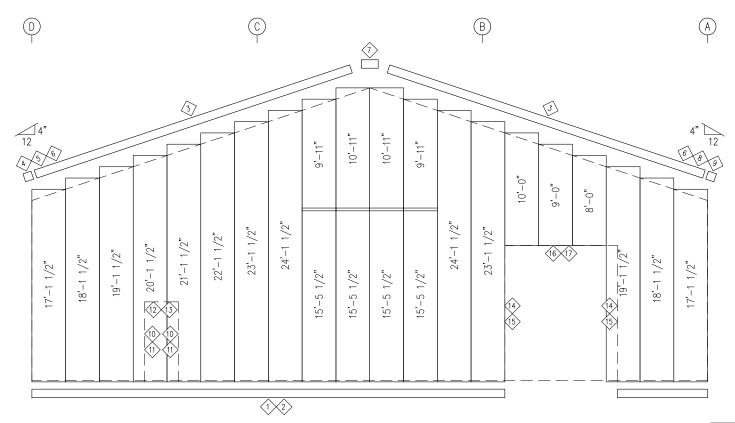
ISSUE DESCRIPTION		DATE	DRN.	CHK.	DES.
Р	PERMIT	09.23.21	AS	SW	RA
С	CONSRUCTION	12.03.21	RS	RS	RA



DESCRIPTION	SIDEWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
лов но.: 5724	PATE: 9/15/21 DWG. NO.: 6 OF 12 ISSUE: C



LEFT ENDWALL FRAMING: FRAME LINE 1



LEFT	ENDWALL	SHEETING	&	TRIM:	FRAME	LINE	1

PANELS: 26 Ga. R-Loc - Galvalume Plus

	P PERMIT (DATE	DRN.	CHK.	DES.
			09.23.21	AS	SW	RA
			12.03.21	RS	RS	RA

ARMYRUB
2 Inverness Drive East, Ste
Englewood, Colorado 801

PHONE: 800-345-4610

DESCRIPTION	ENDWALL FRAMING & SHEETING	
CUSTOMER	(
END USER	(
SCALE	NOT TO SCALE	
лов no.: 5724	-9 ENG. BY: DATE: 9/15 -9 DWG. NO.: 7 OF 12 ISSUE: C	/21

TRIM_50 TRIM_50 TRIM_50 TRIM_51 TRIM_51 BOLT TABLE FRAME LINE 1 LOCATION ER-1/ER-2 A325 A325 A325 Columns/Raf Jamb

DETAIL TRIM_1

TRIM_1

TRIM_66

TRIM_100

TRIM_50 TRIM_50 TRIM_51 TRIM_51

TRIM TABLE
FRAME LINE 1
◇ID QUAN PART
1 2 BA610

14

15 16 17

BA6102

BA6

Q7646

Q765L6

Q7676 Q765R6 AR963R6

Q3706 JA6 AR3806 HE6

Q3706

JA6 Q3706102

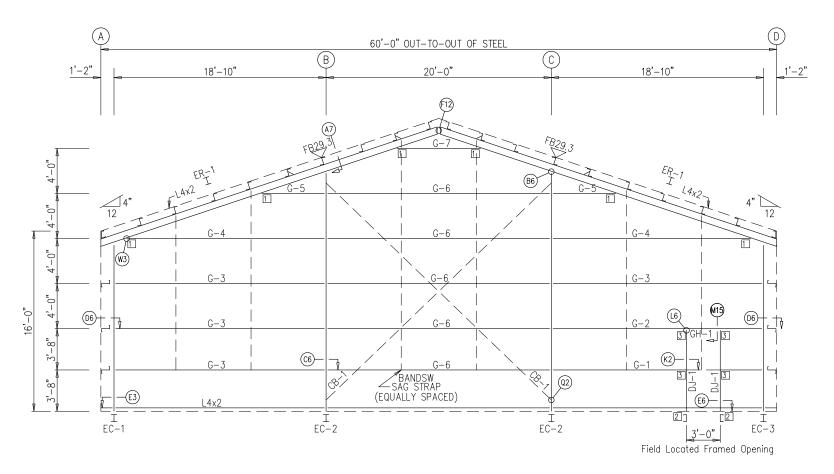
AR963L6 AR9626

CONNECTION PLATES FRAME LINE 1

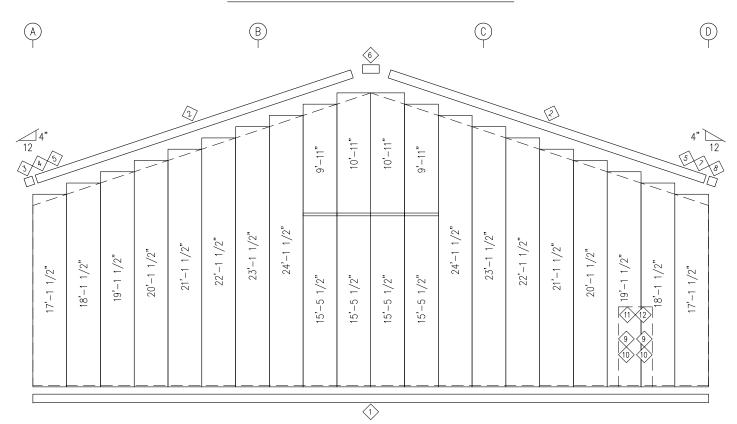
MEMBER			
L FRAME L	LINE 1		
QUAN	MARK	PART	LENGTH
1	EC-1	W10X12	14'-9 13/16"
2	EC-2	W10X12	21'-1 1/8"
l 1	EC-3	W10X12	14'-9 13/16"
li	ER-1	W08631	14'-9 13/16" 31'-6 15/16"
	ER-2	W08631	31'-6 15/16"
2	DJ-1	8X25C16	31'-6 15/16" 7'-0"
2	DJ-2	8X25C12	20'-5 1/8"
	DJ-3	8X25C12	17'-1 1/8"
	DH-1	8X25C16	9'-11"
			18'-2"
	G-1	8X25Z16	18'-2"
	G-2	8X25Z12	18'-2"
	G-3	8X25Z14	10 -2
	G-4	8X25Z14	17'-1 3/8"
2 5 1	G-5	8X25Z16	5'-1 3/8" 19'-4"
5	G-6	8X25Z14	19'-4"
	G-7	8X25Z16	6'-10 13/16"
5	G-8	8X25Z16	1'-4"
1	G-9	8X25Z16	9'-11"
3	G-10	8X25Z16	6'-2"
1	G-11	8X25Z16	3'-4 7/8" 28'-1 1/2"
2	CB-1	GS1716	28'-1 1/2"
1	GH-1	HW816Z	3'-0"

FLANGE BRACE TABLE FRAME LINE 1 VID QUAN MARK FB29.

MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.



RIGHT ENDWALL FRAMING: FRAME LINE 5



MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

RIGHT ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. R-Loc - Galvalume Plus

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
Р	PERMIT	09.23.21	AS	SW	RA
С	CONSTRUCTION	12.03.21	RS	RS	RA

AMSTOLIC
2 Inverness Drive East, Ste#2
Englewood, Colorado 80112
DHONE: 800-345-4610

DESCRIPTION	ENDWALL FRAMING & SHEETING	
CUSTOMER		_
END USER		
SCALE	NOT TO SCALE	
лов no.: 5724	Participants Partic	_

TRIM	TABLE						
	IE LINE	5					
♦ID	QUAN	PART		NGTH		DETAI	L
1	3	BA6	20)'-4"		TRIM_	_1
2 3	4	Q7646	16	s'-2"		TRIM_	_66
3	1	Q765L6	6	,			
4	1	AR963L6	9	1/8"			
5	2	AR9626	- 18	1/16			
6	1	Q7676	1'	-4 "		TRIM_	_100
7	1	Q765R6	6'	,			
8	1	AR963R6	9	1/8"			
9	2	Q3706	7'	-4"		TRIM_	_50
10	2	JA6	7'	-2"		TRIM_	
11	1	AR3806	3'	−7 "		TRIM_	
12_	1	HE6	3	-6"		TRIM_	_51
Г	BOLT T	A DI C					
	FRAME						
	LOCATION			QUAN	TYPE	DIA	LENGTH
-	ER-1/E			8	A325	5/8"	1 1/2"
	0-1	- /D - f		4	A 70E	1/2"	1 1/4"

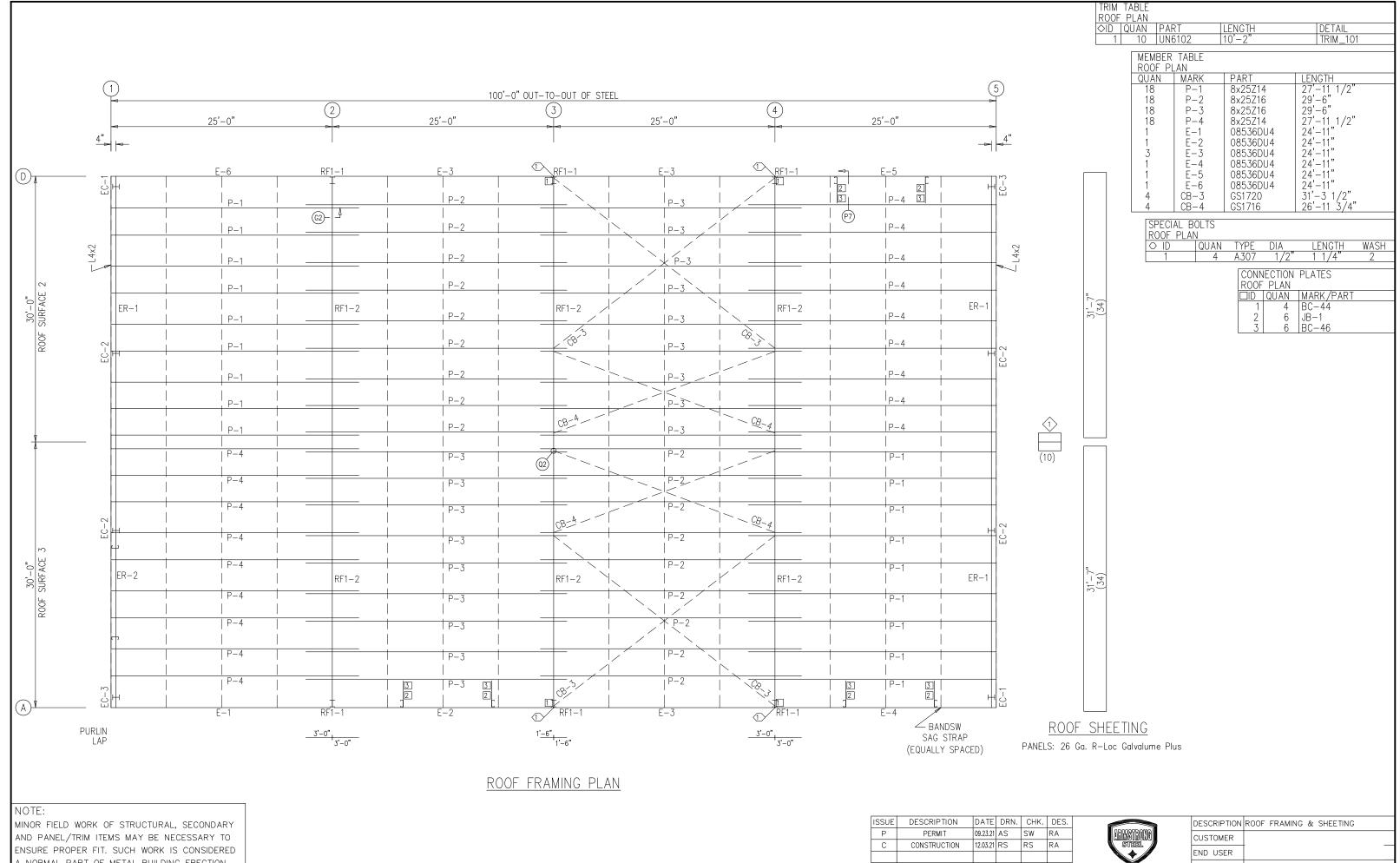
Columns/Raf

MEMBER	TABLE		
FRAME L	INE 5		
QUAN	MARK	PART	LENGTH
1	EC-1	W10X12	14'-9 13/16"
2	EC-2	W10X12	21'-1 1/8"
1	EC-3	W10X12	14'-9 13/16"
2	ER-1	W08631	31'-6 15 ['] /16"
2	DJ-1	8X25C16	7'-0"
1	G-1	8X25Z16	18'-2"
1	G-2	8X25Z12	18'-2"
4	G-3	8X25Z14	18'-2"
2	G-4	8X25Z14	17'-1 3/8"
2 2 5	G-5	8X25Z16	5'-1'3/8"
5	G-6	8X25Z14	19'-4"
1	G-7	8X25Z16	6'-10 13/16"
2	CB-1	GS1716	28'-1 1/2"
1	GH-1	HW816Z	3'-0"

A325 A325

	NGE BRAC		
FRA	ME LINE :	5	
$\triangle ID$	QUAN	MARK	LENGTH
1	2	FB29.3	2'-5 1/4"
		NEOTION DI ATI	

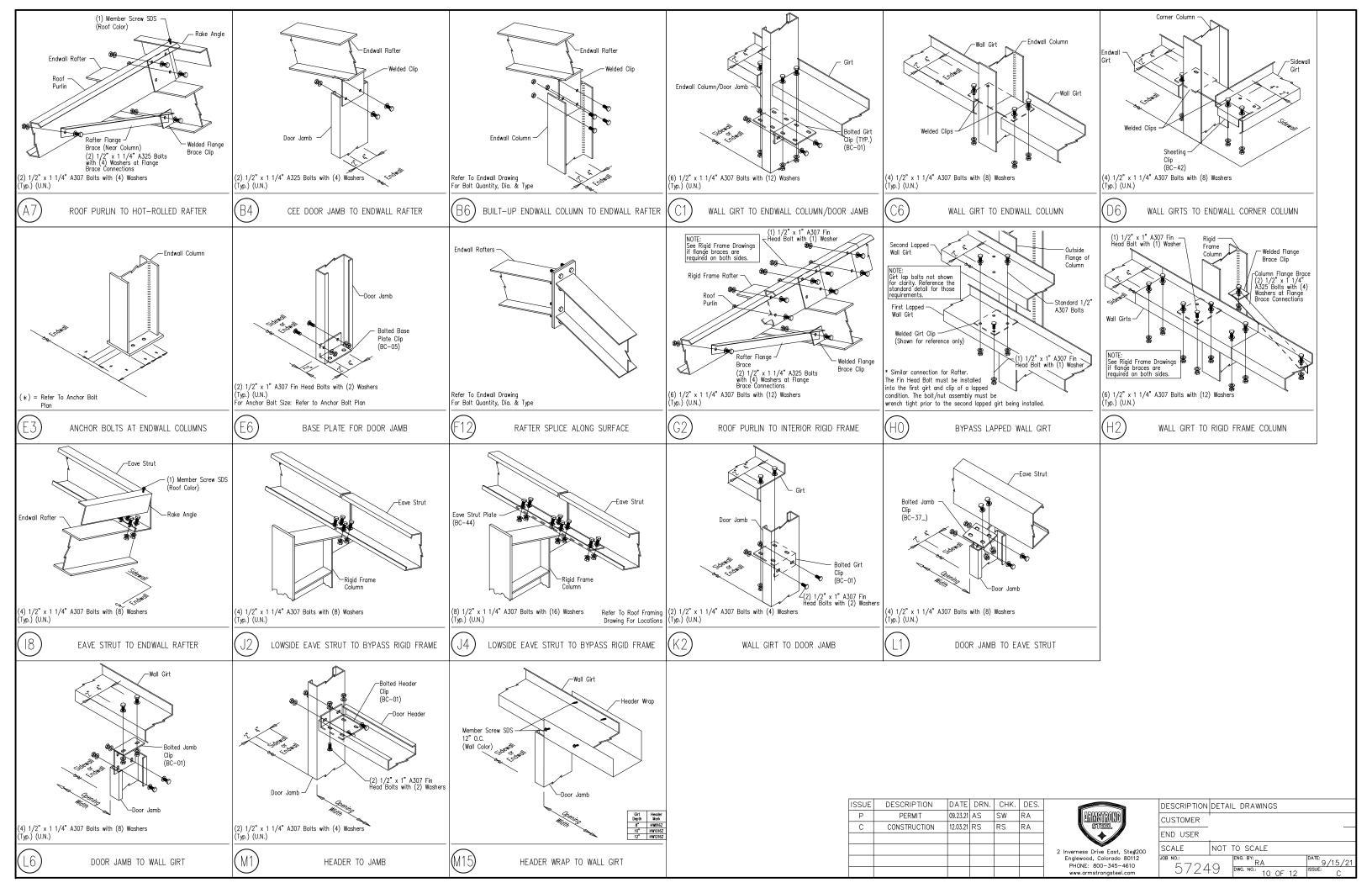
l CONN	IECTION	PLATES
FRAM	IE LINE :	5
	QUAN	MARK/PART
1	6	BC-15E
2	2	BC-05
3	4	BC-01

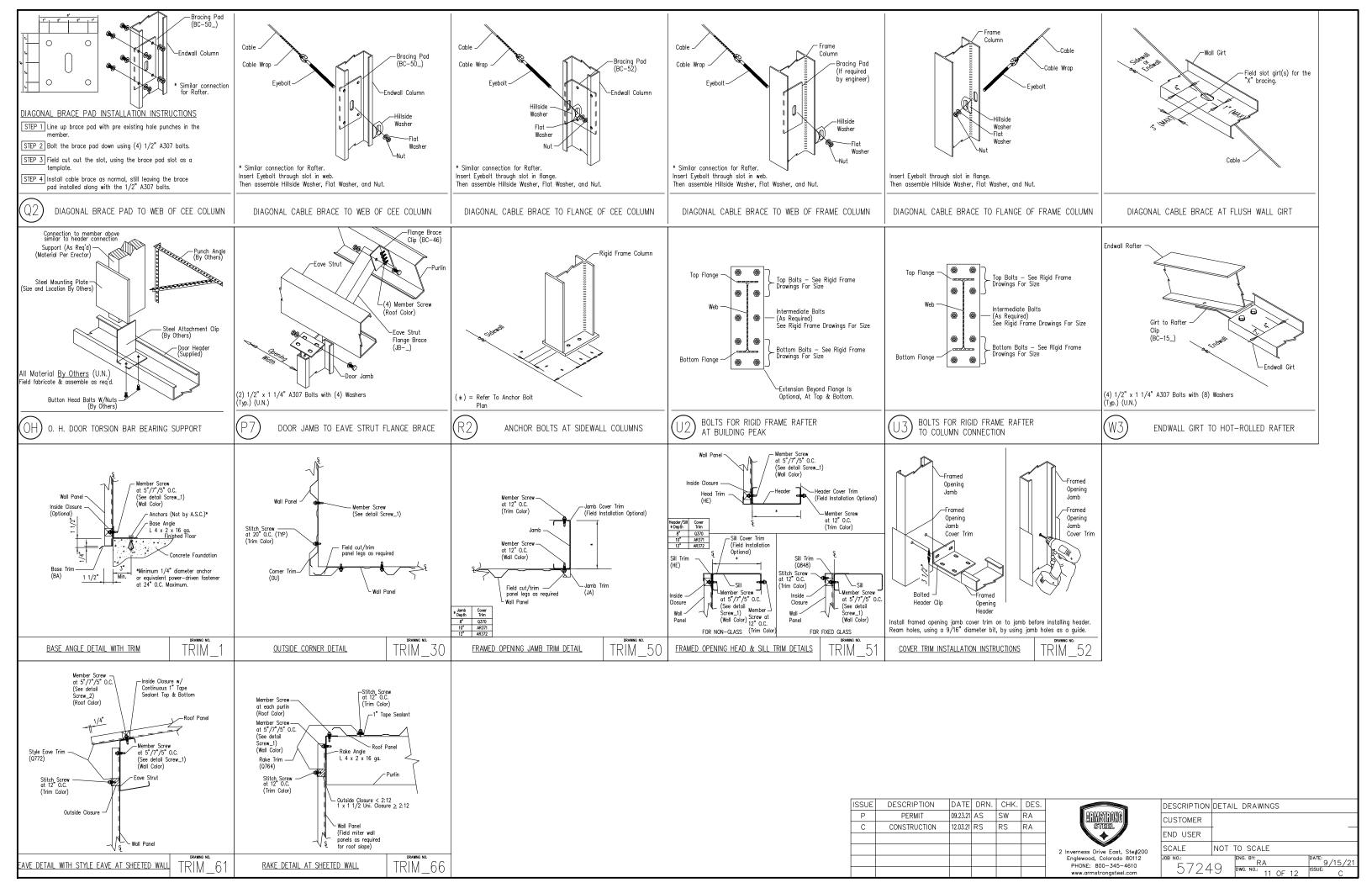


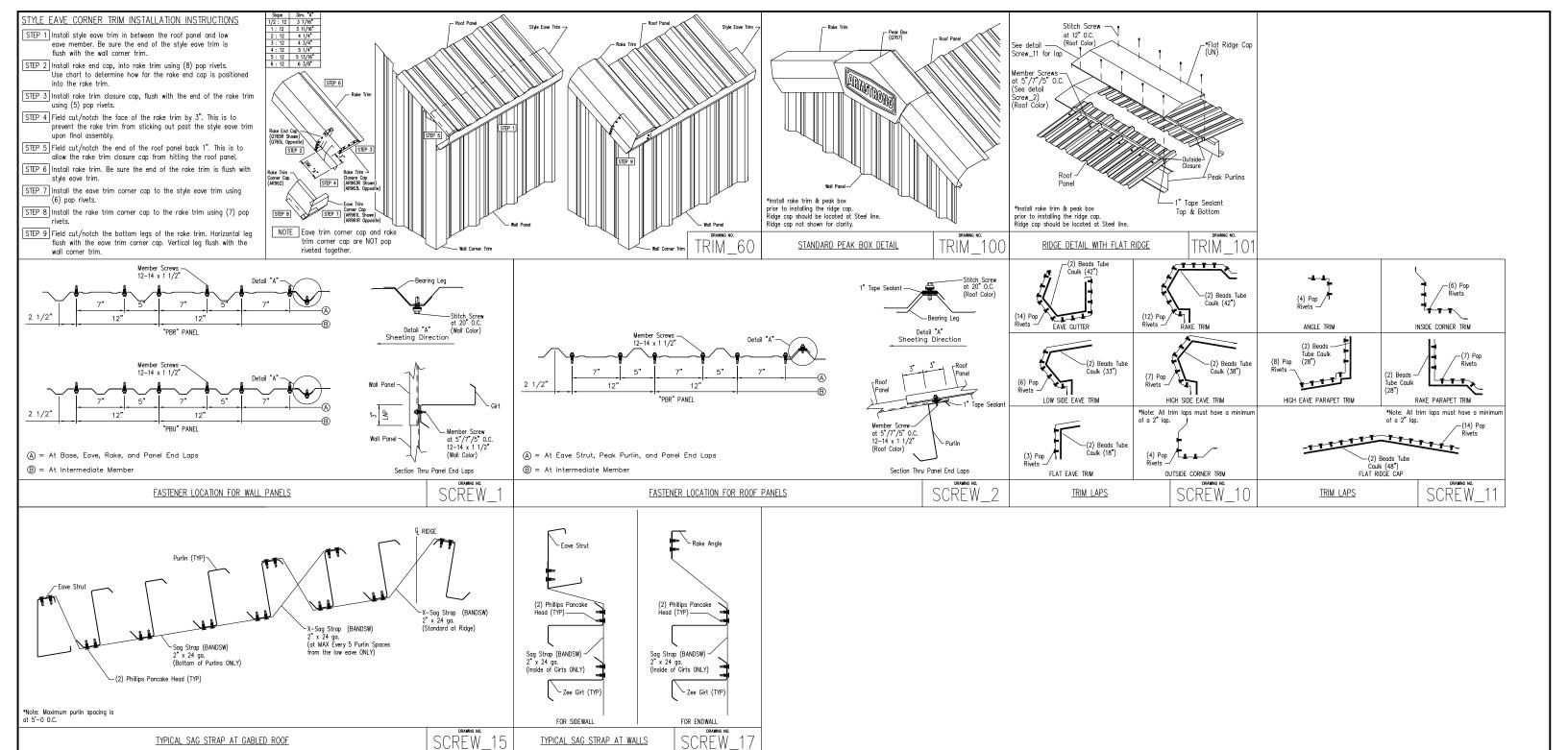
A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

2 Inverness Drive East, Ste#200 Englewood, Colorado 80112 PHONE: 800-345-4610

SCALE NOT TO SCALE ENG. BY: RA DWG. NO.: 9 OF 12 DATE: 9/15/21 ISSUE: C_ JOB NO.: 57249







TYPICAL SAG STRAP AT WALLS

TYPICAL SAG STRAP AT GABLED ROOF

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	
Р	PERMIT	09.23.21	AS	SW	RA	
С	CONSTRUCTION	12.03.21	RS	RS	RA	
						2

CONTROL ON O	DESCRIPTION	DETAIL DRAWINGS	
	CUSTOMER		_
9115155	END USER		
rness Drive East, Ste#200	SCALE	NOT TO SCALE	
lewood, Colorado 80112 HONE: 800-345-4610 ww.armstrongsteel.com	^{ЈОВ NO.:} 5724	RA	9/15/2 SSUE: C