

GENERAL NOTES

- 1.1 Fabrication shall be in accordance with A.S.C. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".
- 1.2 **MATERIALS**
- | ASTM DESIGNATION | MIN. YIELD STRENGTH |
|---------------------------------------|--------------------------------------|
| Hot Rolled Steel Shapes (W, & C) | A572
Fy = 50 KSI |
| Hot Rolled Steel Angles (L) | A36
Fy = 36 KSI |
| Steel Pipes | A500
Fy = 42 KSI |
| Structural Tubing | A500
Fy = 42 KSI |
| Structural Steel Web Plate | A572/A1011
Fy = 50 KSI |
| Structural Steel Flange Plates/Bars | A529/A572
Fy = 55 KSI |
| Cold Formed Light Gage | A653/A1011
Fy = 55 KSI |
| Roof and Wall Sheets | A792/A653
Fy = 50, 80 KSI |
| Cable Brace | A475 - TYPE 1
Extra High Strength |
| Rod Brace | A529
Fy = 50 KSI |
| MIN. TENSILE STRENGTH | |
| Machine Bolts & Nuts | A307
Fu = 60 KSI |
| High Strength Bolts (1" & less) | A325-TYPE 1
Fu = 120 KSI |
| High Strength Bolts (>1" to 1 1/2") | A325-TYPE 1
Fu = 105 KSI |
| Anchor Bolts (Not supplied by A.S.C.) | A36/A307/F1554
Fu = 60 KSI |
- 1.3 **PRIMER**
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is A.S.C. Gray Oxide color. This paint is not intended for long term exposure to the elements. A.S.C. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. A.S.C. shall not be responsible for any field applied paint and/or coatings. (AISC Code of Standard Practice, Latest Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.
- 1.4 **GALVANIZED OR SPECIAL COATINGS:**
See Contract Documents
- 1.5 **ALL BOLTS ARE 1/2" x 0'-1 1/4" A307 EXCEPT:**
a) Endwall rafter splice - 5/8" x 0'-1 3/4" A325-N
b) Endwall column to rafter connection - 1/2" x 0'-1 1/4" A325 MIN.(SEE WALL ELEVATION)
c) Main frame connections - SEE CROSS SECTION
d) Flange Brace connections - 1/2" x 0'-1 1/4" A325
NOTE: Washers are not supplied unless noted otherwise on drawing
- 1.6 **A325 BOLT TIGHTENING REQUIREMENTS**
All high strength bolts are A325-N unless specifically noted otherwise. Holes are not slotted and design is bearing connection. Structural bolts shall be tightened by the turn-of-the-nut method in accordance with the Latest Edition AISC "Specification For Structural Joints" using ASTM A325 or A490 Bolts, when specifically required. A325-N bolts are supplied without washer unless otherwise noted on the drawings.
All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.
- 1.7 **CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:**
INSIDE - Under roof panels & base of wall panels
OUTSIDE - Between roof panels & ridge cap
- Between wall panels & eave/gable trim
- 1.8 **ERECTION NOTE:**
All bracing, strapping, & bridging shown and provided by A.S.C. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.
- 1.9 **ERECTION AND UNLOADING NOT BY A.S.C.**
- 1.10 **SHORTAGES**
Any claims or shortages by buyer must be made to A.S.C. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.
- 1.11 **CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)**
Claims for correction of alleged misfits will be disallowed unless A.S.C. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of A.S.C.
- BUYER/END USE CUSTOMER RESPONSIBILITIES**
- 2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release A.S.C. to fabricate upon receiving such.
- 2.2 Armstrong Steel Corp (hereafter referred to as A.S.C.) standard specifications apply unless stipulated otherwise in the Contract Documents. A.S.C. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.
- 2.3 In case of discrepancies between A.S.C. structural steel plans and plans for other trades, A.S.C. plans shall govern. (Section 3 AISC Code of Standard Practices, Latest Edition)
- 2.4 Approval of A.S.C. drawings and calculations indicates that A.S.C. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the A.S.C. design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)
- 2.5 Once the BUYER/END USE CUSTOMER has signed A.S.C. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.
- 2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by A.S.C. and A.S.C. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or A.S.C. assumptions will govern (AISC Code of Standard Practice, Latest Edition)



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JOB NO. : 57318

CUSTOMER :
END USER :
END USE :
LOCATION :
PH. NO. :

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

Design Code / Wind Code	:IBC-06
Enclosure	:Closed
Dead Load (psf)	:2.00
Collateral Load (psf)	:1.00
Wind Load	
Basic Wind Speed, 3 sec gust (mph)	:95.00
Wind Importance Factor, Iw	:1.00
Wind Exposure	:C
Internal Pressure Coefficient, GCpi	:0.18 /-0.18
Wall Panel Design Wind Pressure (psf)	:19.70 /-21.40
Live Load	
Primary Framing (psf)	:20.00
Trib. Area Reduction	:No
Secondary Framing (psf)	:20.00
Snow Load	
Ground Snow Load, Pg (psf)	:25.00
Roof Snow Load, Pf (psf)	:25.00
Sloped Roof Snow Load, Ps (psf)	:25.00
Snow Exposure Factor, Ce	:1.0000
Snow Importance Factor, Is	:1.00
Thermal Factor, Ct	:1.20
Sloped Factor, Cs	:1.000
Seismic Load	
Seismic Importance Factor, Ie	:1.00
Seismic Use Group	:II - Normal
Site Class	:D
Mapped Spectral Response Acceleration	:Ss = 0.192 :S1 = 0.058
Spectral Response Coefficients	:Sds = 0.205 :Sd1 = 0.093
Seismic Design Category	:B
Basic Force Resisting Systems Used	:Steel System Not Specifically Detailed For Resistance
Total Design Base Shear, V (kips)	:Rigid Frames (OMF) :Braced Frames (OCBF/OMF) :Longitudinal = 0.97 :Transverse = 0.98
Response Modification Factors, R	:Rigid Frames = 3.00 Ω = 3.00 :SW X-Bracing = 3.00 Ω = 2.00
Seismic Response Coefficient, Cs	:Rigid Frames = 0.0683 :SW X-Bracing = 0.0683
Analysis Procedure Used	:Equivalent Lateral Force Procedure
Other Loads/Requirements	

BUILDING DESCRIPTION:

Width (ft)	:40
Length (ft)	:60
Eave Ht. at BSW (ft)	:10
Eave Ht. at FSW (ft)	:10
Roof Slope at BSW	:3.00:12
Roof Slope at FSW	:3.00:12
Bay Spacing (ft)	:3 at 20

COVERING AND TRIMS:

Roof Panels & Trims	
Panel Type	:26 Ga. R-Loc
Panel Color	:Hunter 40 yr
Trim Colors	
Gable/Eave Trim	:Hunter 40 yr
Wall Panel & Trims	
Panel Type	:26 Ga. R-Loc
Panel Color	:Rustic 40 yr
Trim Colors	
Corner Trims	:Hunter 40 yr
Opening Trims	:Hunter 40 yr
Base Trim	:Rustic 40 yr

Drawing Index

Drawing Name	Page(s)
Drawing Cover	COVER
3D Reference	3D REF
Anchor Bolt Plan	1
Anchor Bolt Details	2
Anchor Bolt Reactions	3
Rigid Frame	4
Front Sidewall	5
Back Sidewall	6
Left Endwall	7
Right Endwall	8
Roof Plan	9
Details	10-12

BUYER/END USE CUSTOMER RESPONSIBILITIES CONTINUED

- 2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that A.S.C. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that A.S.C. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by A.S.C.
- 2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with A.S.C. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (AISC Code of Standard Practice, Latest Edition.)
- 2.9 Armstrong Steel Corp is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete. Unless otherwise provided in the Order Documents, A.S.C. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site. It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Latest MBMA Low Rise Building Systems Manual)
- 2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to A.S.C. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (AISC Code of Standard Practice Latest Edition)
- 2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (AISC Code of Standard Practice Latest Edition)
- 2.12 **WARNING:** In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto Galvalume should be avoided.
- 2.13 **SAFETY COMMITMENT:** Armstrong Steel Corp has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of A.S.C. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.
- 2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.
- 2.15 It is recommended by Factory Mutual (Reference: B2.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

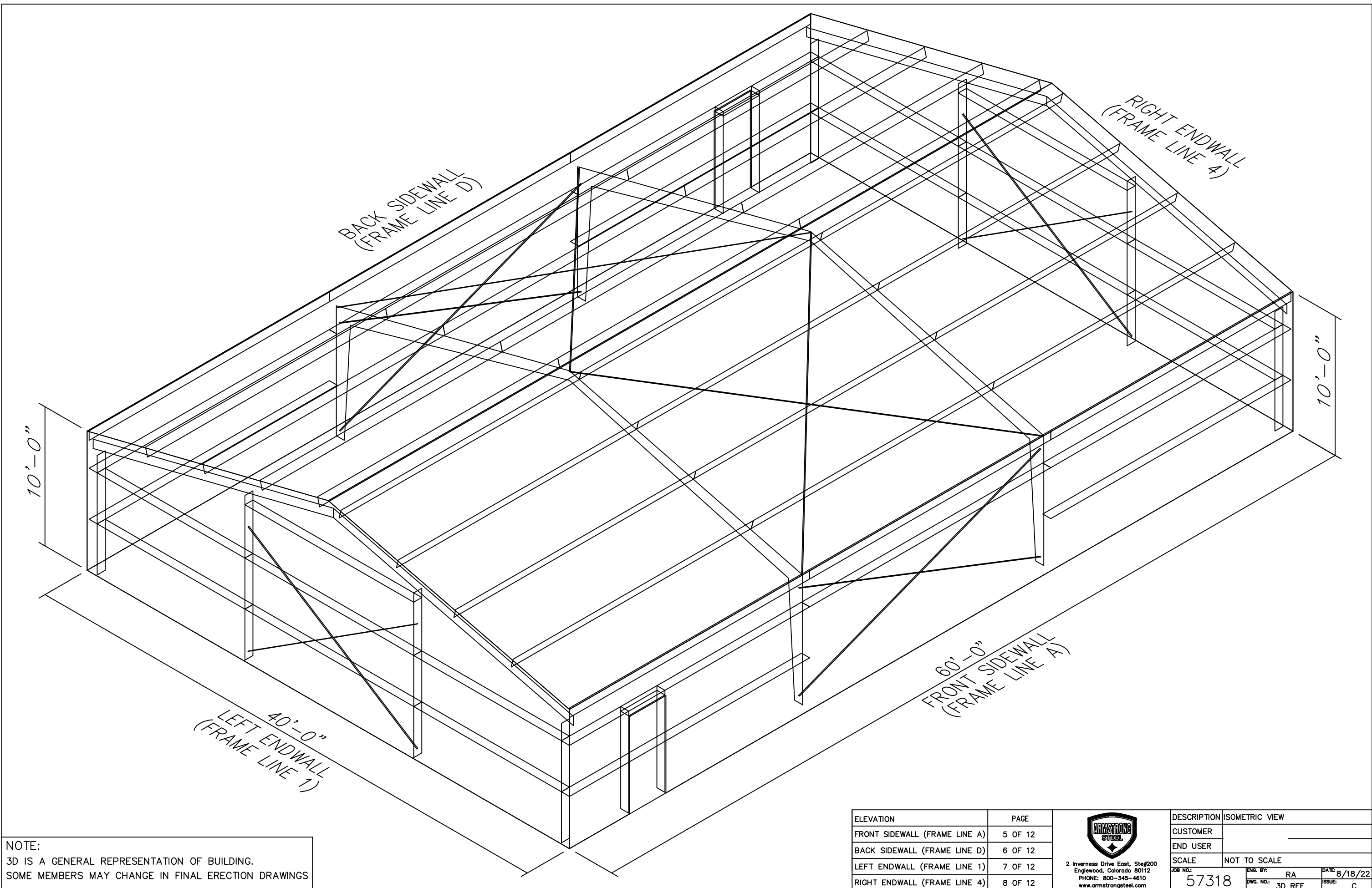
NOTE:
For Snow/Ice Removal Procedure, Refer to Metal Building System Manual 2002 Edition, Section A8.4, Page XI-A8-2.

Drawing Status

- APPROVAL:** **REVISED APPROVAL:**
These drawings, being for approval, are by definition not final, and are for conceptual representation only. their purpose is to confirm proper interpretation of the project documents. Only drawings issued "Construction" can be considered as complete.
- PERMIT:** **REVISED PERMIT:**
These drawings, being for permit, are by definition not final. Only drawings issued "Construction" can be considered as complete.
- CONSTRUCTION:**
Final drawings to be used in the erection of the building.

JOB NO : 57318

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT ARMSTRONG STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY A.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ARMSTRONG ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.



NOTE:
 3D IS A GENERAL REPRESENTATION OF BUILDING.
 SOME MEMBERS MAY CHANGE IN FINAL ERECTION DRAWINGS

ELEVATION	PAGE
FRONT SIDEWALL (FRAME LINE A)	5 OF 12
BACK SIDEWALL (FRAME LINE D)	6 OF 12
LEFT ENDWALL (FRAME LINE 1)	7 OF 12
RIGHT ENDWALL (FRAME LINE 4)	8 OF 12

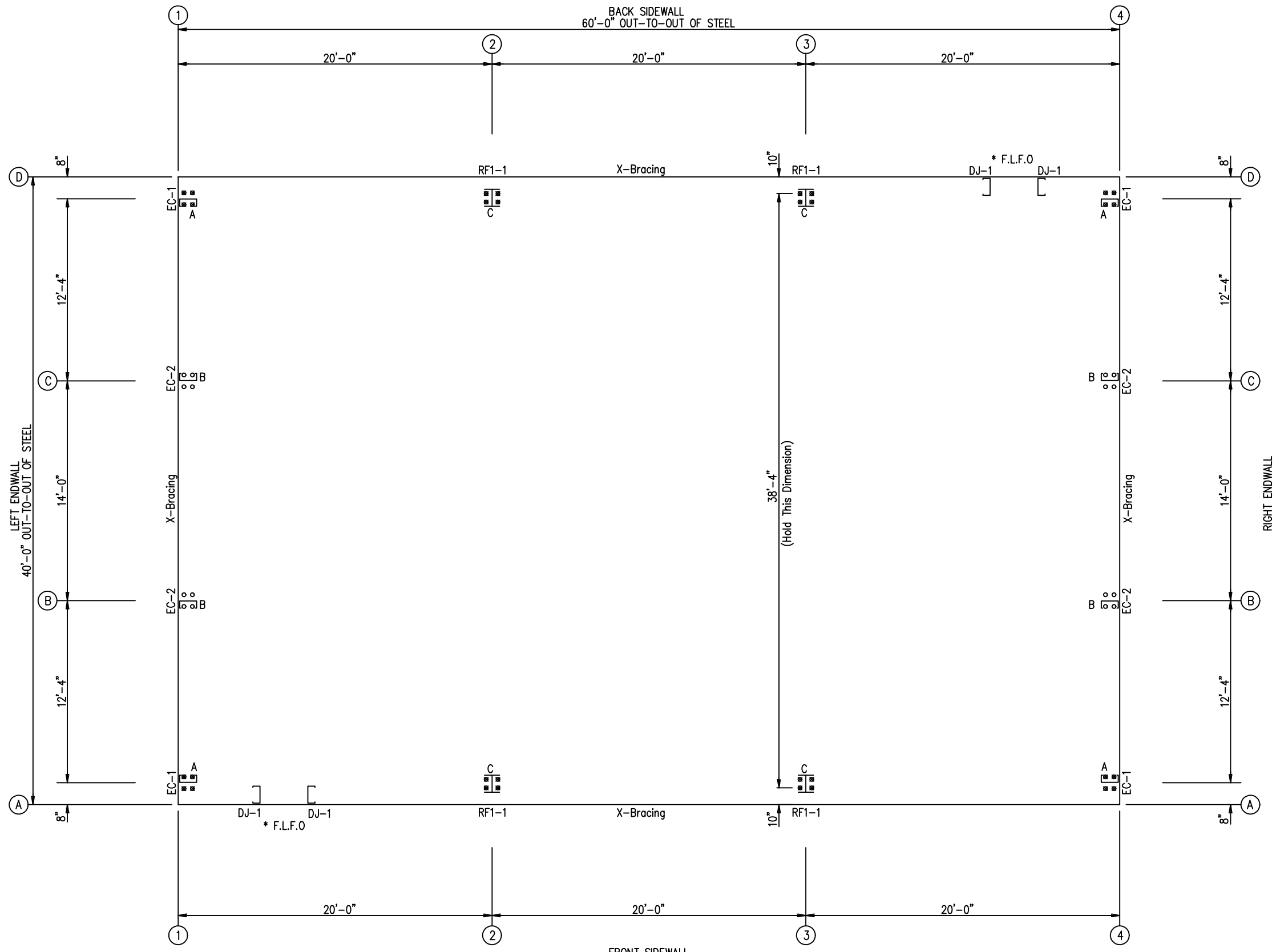


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DESCRIPTION	ISOMETRIC VIEW
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57318	ENG. BY: RA DATE: 8/18/22
	DWG. NO.: 3D REF ISSUE: C

ANCHOR BOLT SUMMARY

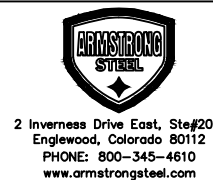
Qty	Locate	Dia (in)	Type
○ 16	Endwall	5/8"	A307
⊗ 16	Endwall	3/4"	A307
⊗ 16	Frame	3/4"	A307



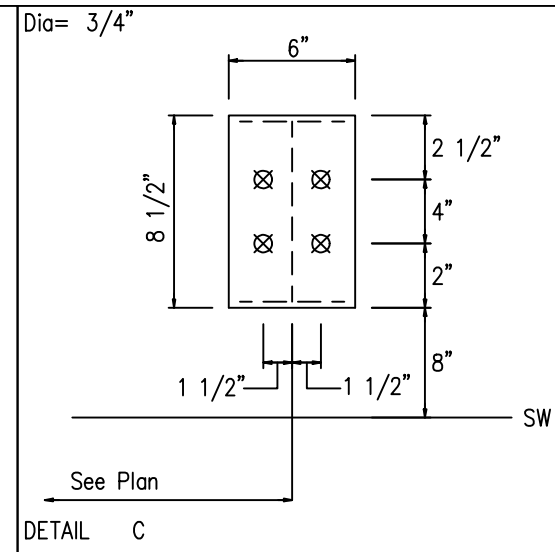
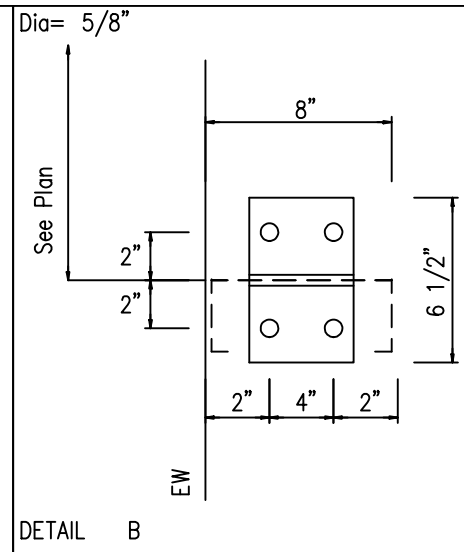
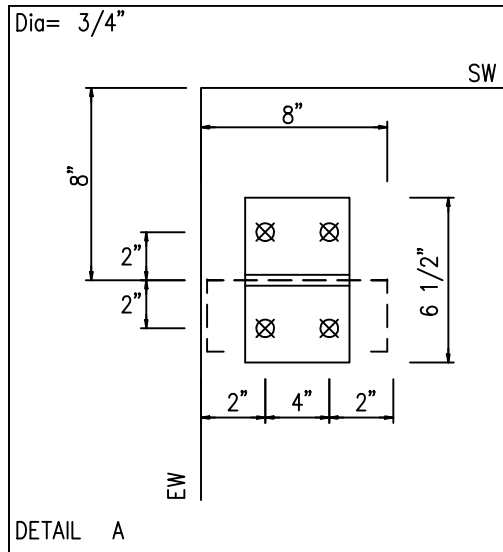
ANCHOR BOLT PLAN
 NOTE: All Base Plates @ 100'-0" (U.N.)
 * Field Located Framed Opening

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA



DESCRIPTION	ANCHOR BOLT PLAN		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO:	57318	ENG. BY:	RA
DATE:	8/18/22	DWG. NO.:	1 OF 12
ISSUE:	C		

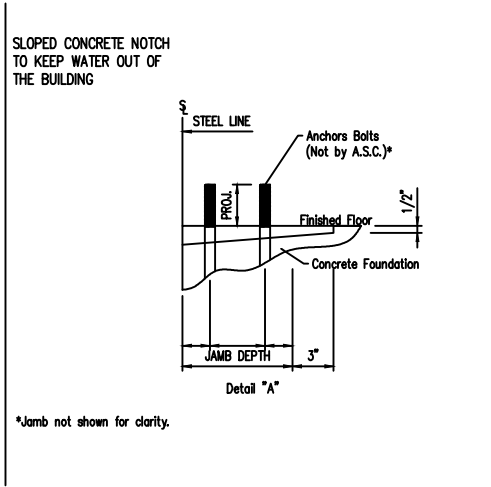
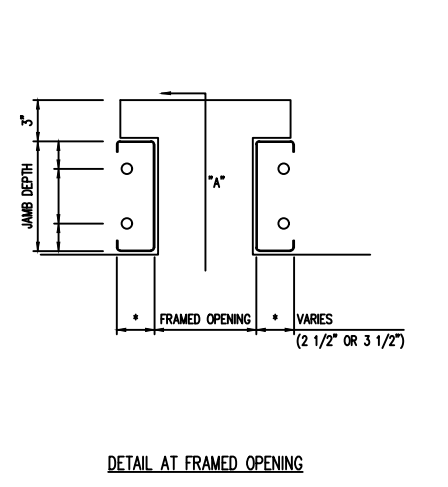
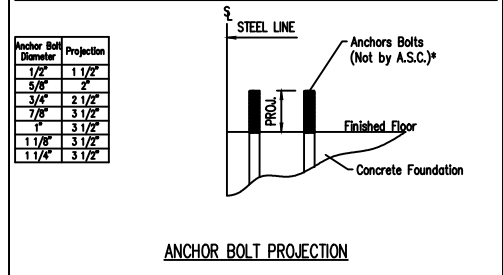


NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING MANUFACTURER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

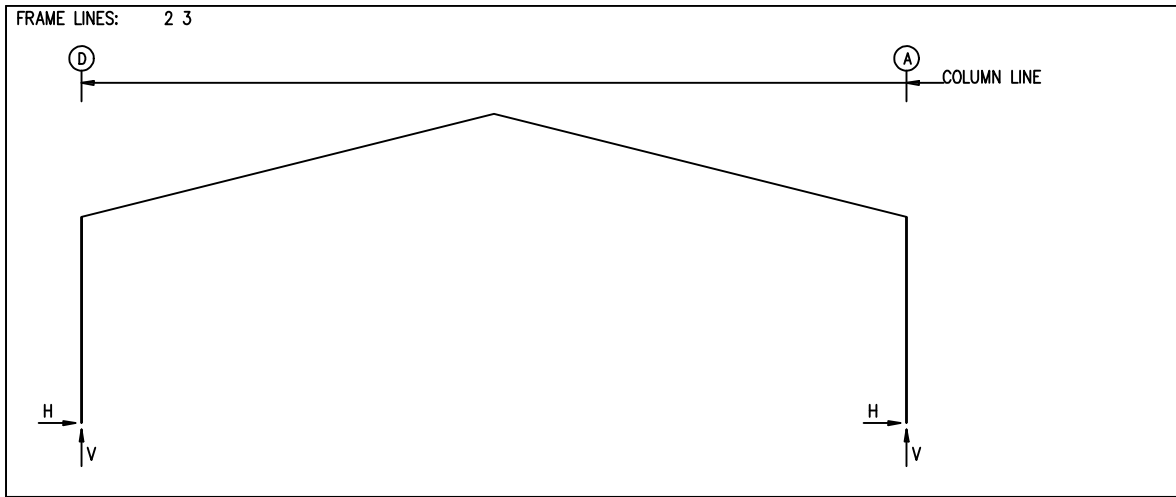
NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.



ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA

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DESCRIPTION	ANCHOR BOLT DETAILS	
CUSTOMER		
END USER		
SCALE	NOT TO SCALE	
JOB NO:	57318	ENG. BY: RA
		DATE: 8/18/22
		DWG. NO.: 2 OF 12
		ISSUE: C



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)					Vmin	Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H				Width	Length	Thick	
2*	D	1	6.5	11.6	2	-3.5	-5.0	4	0.750	6.000	8.500	0.500	0.0
2*	A	3	3.5	-5.0	1	-6.5	11.6	4	0.750	6.000	8.500	0.500	0.0
2*	Frame lines: 2 3												

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead	Collateral	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind_Suct	Wind_Long1	Wind_Long2	Seis_Left	Seis_Right
2*	D	0.6	1.2	0.2	0.4	4.5	8.0	-3.9	-5.7	-0.9	-4.2	0.1	0.1	0.1
2*	A	-0.6	1.2	-0.2	0.4	-4.5	8.0	-5.7	10.0	0.9	-4.2	0.1	0.1	-0.1

Frame Line	Column Line	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind_Long1	Wind_Long2	Seis_Left	Seis_Right
2*	D	-3.4	-3.3	-0.4	-1.8	-2.0	-6.5	-1.3	-4.1
2*	A	0.4	-1.8	3.4	-3.3	2.0	-6.5	1.3	-4.1

Frame Line	Column Line	Seis_Left	Seis_Right	FIUNB_SL_L	FIUNB_SL_R
2*	D	0.0	-0.2	4.6	9.6
2*	A	0.0	-0.2	-4.6	9.6

2* Frame lines: 2 3

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
16	Endwall	5/8"	A307
16	Endwall	3/4"	A307
16	Frame	3/4"	A307

BUILDING BRACING REACTIONS

Loc	Wall	Col Line	± Reactions(k)				Panel_Shear (lb/ft)	
			Wind	Seismic	Wind	Seis		
L_EW	1	C,B	1.0	0.9	0.2	0.2		
F_SW	A	2,3	1.7	0.7	0.5	0.2		
R_EW	4	B,C	1.0	0.9	0.2	0.2		
B_SW	D	3,2	1.7	0.7	0.5	0.2		

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead	Collat	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind Press
1	D	0.2	0.1	1.1	1.3	0.0	-0.9	0.0	-0.9	-0.5
1	C	0.5	0.2	3.0	3.7	-1.0	-3.6	0.0	-0.8	-1.2
1	B	0.5	0.2	3.0	3.7	0.0	-0.8	1.0	-3.6	-1.2
1	A	0.2	0.1	1.1	1.3	0.0	-0.9	0.0	-0.9	-0.5

Frm Line	Col Line	Wind Suct	Wind Long1	Wind Long2	Seis_Left	Seis_Right	Seis Long	E1UNB_SL_L	E1UNB_SL_R
1	D	0.5	-1.0	-0.6	0.0	0.0	0.0	0.0	0.3
1	C	1.4	-1.9	-1.2	-0.2	-0.2	0.0	0.0	1.6
1	B	1.4	-1.9	-1.2	0.0	0.2	0.2	0.0	4.5
1	A	0.5	-1.0	-0.6	0.0	0.0	0.0	0.0	1.3

Frm Line	Col Line	-LWIND1_L	-LWIND1_R	-LWIND2_L	-LWIND2_R
1	D	0.0	-0.4	0.0	0.0
1	C	0.0	-0.1	-0.1	-0.1
1	B	0.1	-0.1	0.0	-0.1
1	A	0.0	0.0	0.0	-0.4

Frm Line	Col Line	Dead	Collat	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind Press
4	A	0.2	0.1	1.1	1.3	0.0	-0.9	0.0	-0.9	-0.5
4	B	0.5	0.2	3.0	3.7	-1.0	-3.6	0.0	-0.8	-1.2
4	C	0.5	0.2	3.0	3.7	0.0	-0.8	1.0	-3.6	-1.2
4	D	0.2	0.1	1.1	1.3	0.0	-0.9	0.0	-0.9	-0.5

Frm Line	Col Line	Wind Suct	Wind Long1	Wind Long2	Seis_Left	Seis_Right	Seis Long	E2UNB_SL_L	E2UNB_SL_R
4	A	0.5	-1.0	-0.6	0.0	0.0	0.0	0.0	0.3
4	B	1.4	-1.9	-1.2	-0.2	-0.2	0.0	0.0	1.6
4	C	1.4	-1.9	-1.2	0.0	0.2	0.2	0.0	4.5
4	D	0.5	-1.0	-0.6	0.0	0.0	0.0	0.0	1.3

Frm Line	Col Line	-LWIND1_L	-LWIND1_R	-LWIND2_L	-LWIND2_R
4	A	0.0	-0.4	0.0	0.0
4	B	0.0	-0.1	-0.1	-0.1
4	C	0.1	-0.1	0.0	-0.1
4	D	0.0	0.0	0.0	-0.4

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
1	D	6	0.5	-0.9	7	-0.5	-0.9	4	0.750	6.500	6.000	0.313	0.0
1	C	9	1.4	-3.3	7	-1.2	-1.6	4	0.625	6.500	6.000	0.313	0.0
1	B	11	1.4	-3.3	7	-1.2	-1.6	4	0.625	6.500	6.000	0.313	0.0
1	A	6	0.5	-0.9	7	-0.5	-0.9	4	0.750	6.500	6.000	0.313	0.0
4	A	6	0.5	-0.9	7	-0.5	-0.9	4	0.750	6.500	6.000	0.313	0.0
4	B	9	1.4	-3.3	7	-1.2	-1.6	4	0.625	6.500	6.000	0.313	0.0
4	C	11	1.4	-3.3	7	-1.2	-1.6	4	0.625	6.500	6.000	0.313	0.0
4	D	6	0.5	-0.9	7	-0.5	-0.9	4	0.750	6.500	6.000	0.313	0.0

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	= 40.00
Length (ft)	= 60.00
Eave Height (ft)	= 10.00/10.00
Roof Slope (rise/12)	= 3.00/3.00
Dead Load (psf)	= 2.00
Collateral Load (psf)	= 1.00
Live Load (psf)	= 20.00
Snow Load (psf)	= 25.00
Wind Speed (mph)	= 95.00
Wind Code	= IBC-06
Exposure	= C
Closed/Open	= C
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Design Category	= B
Seismic Coeff (Fa*Ss)	= 0.31

ID	Description
1	Dead+Collateral+Snow+Slide_Snow
2	0.6Dead+Wind_Left1
3	0.6Dead+Wind_Right1
4	0.6Dead+Wind_Long1L+LWIND1_L2E
5	0.6Dead+Wind_Long1L+LWIND1_R2E
6	0.6Dead+Wind_Suction+Wind_Long1L
7	0.6Dead+Wind_Pressure+Wind_Long1L
8	0.6Dead+Wind_Long1L+LWIND1_L
9	0.6Dead+Wind_Left1+Wind_Suction
10	Dead+Collateral+E1UNB_SL_L
11	0.6Dead+Wind_Right1+Wind_Suction
12	Dead+Collateral+E1UNB_SL_R
13	0.6Dead+Wind_Long1L+LWIND1_R
14	Dead+Collateral+E2UNB_SL_L
15	Dead+Collateral+E2UNB_SL_R

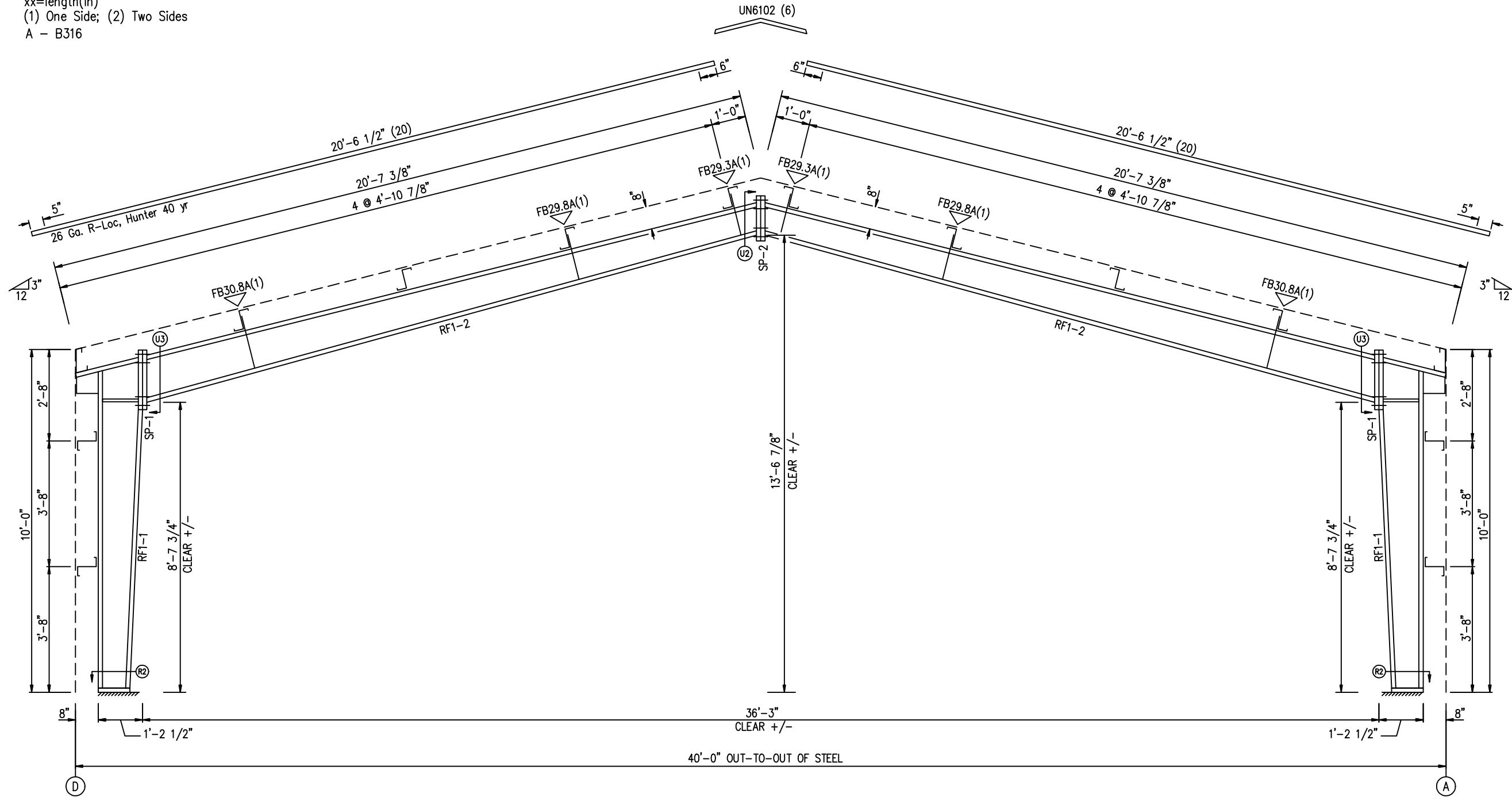
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA

		DESCRIPTION: ANCHOR BOLT REACTIONS CUSTOMER: END USER: SCALE: NOT TO SCALE
2 Inverness Drive East, Ste#200 Englewood, Colorado 80112 PHONE: 800-345-4610 www.armstrongsteel.com	JOB NO: 57318 ENG. BY: RA DATE: 8/18/22 DWG. NO: 3 OF 12 ISSUE: C	

SPLICE BOLT TABLE						
Mark	Qty Top	Qty Bot	Int	Type	Dia	Length
SP-1	4	4	0	A325	0.625	2.25
SP-2	4	4	0	A325	0.625	1.75

Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start/End	Thick	Length	Length	W x Thk x Length	W x Thk x Length
RF1-1	8.0/13.8	0.135	8'-0 3/4"		5 x 1/4" x 9'-5"	5 x 1/4" x 8'-3 15/16"
RF1-2	13.8/14.0	0.188	1'-7 13/16"		5 x 1/4" x 1'-10 7/8"	
RF1-2	13.0/ 8.0	0.135	18'-10"		5 x 1/4" x 18'-6 11/16"	5 x 1/4" x 18'-8"

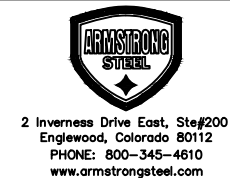
▽ FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 A - B316



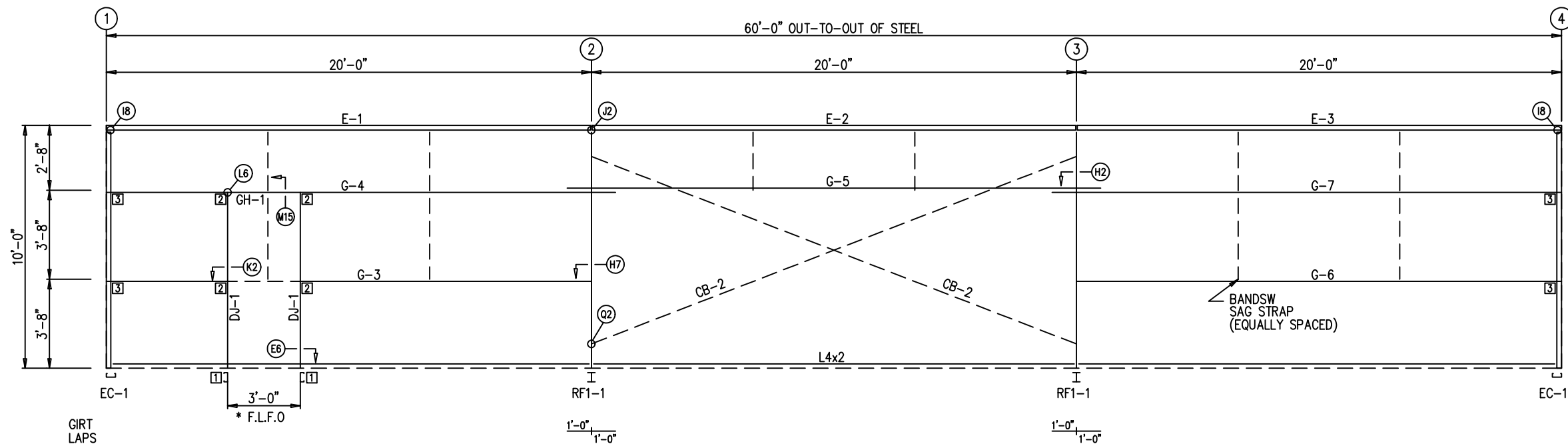
RIGID FRAME ELEVATION: FRAME LINE 2 3

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA



DESCRIPTION	RIGID FRAME ELEVATION		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO:	57318	ENG. BY:	RA
DATE:	8/18/22	DWG. NO.:	4 OF 12
ISSUE:	C		

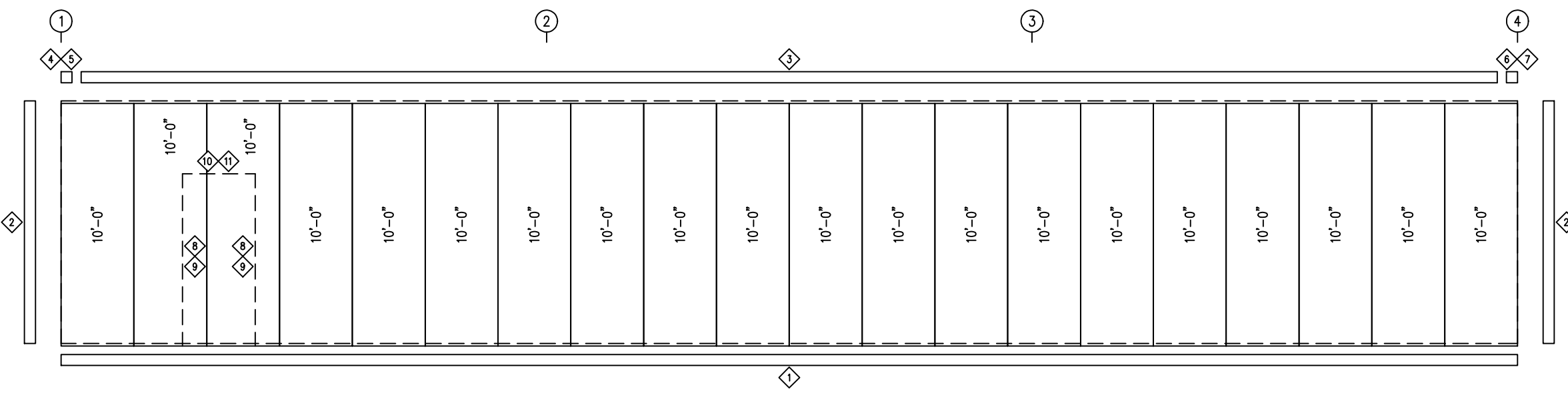


FRONT SIDEWALL FRAMING: FRAME LINE A
* Field Located Framed Opening

TRIM TABLE FRAME LINE A				
ID	QUAN	PART	LENGTH	DETAIL
1	3	BA6	20'-4"	TRIM_1
2	2	OU6102	10'-2"	TRIM_30
3	3	Q7726	20'-4"	TRIM_61
4	1	Q773L6	6"	
5	1	AR961L6	7 7/16"	TRIM_60
6	1	Q773R6	6"	
7	1	AR961R6	7 7/16"	TRIM_60
8	2	Q3706	7'-4"	TRIM_50
9	2	JA6	7'-2"	TRIM_50
10	1	AR3806	3'-7"	TRIM_51
11	1	HE6	3'-6"	TRIM_51

MEMBER TABLE FRAME LINE A				
QUAN	MARK	PART	LENGTH	
2	DJ-1	8X25C16	7'-0"	
1	E-1	08536DU3	19'-11"	
1	E-2	08536DU3	19'-11"	
1	E-3	08536DU3	19'-11"	
1	G-3	8X25Z16	20'-2 1/4"	
1	G-4	8X25Z16	20'-11 1/2"	
1	G-5	8X25Z16	22'-0"	
1	G-6	8X25Z16	20'-2 1/4"	
1	G-7	8X25Z16	20'-11 1/2"	
1	GH-1	HW816Z	3'-0"	
2	CB-2	GS1716	22'-1"	

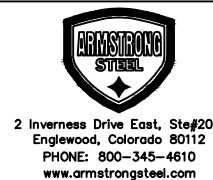
CONNECTION PLATES FRAME LINE A			
ID	QUAN	MARK/PART	
1	2	BC-05	
2	4	BC-01	
3	4	BC-07	



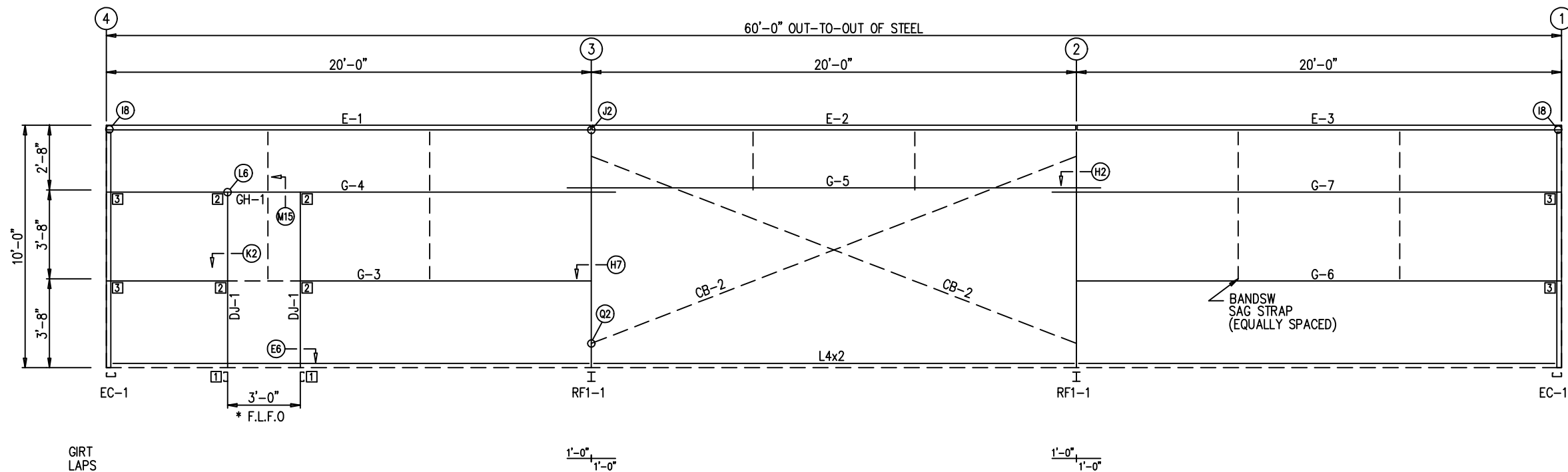
FRONT SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. R-Loc - Rustic 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/10/23	RB	DP	RA



DESCRIPTION	SIDEWALL FRAMING & SHEETING		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO:	57318	ENG. BY:	RA
		DATE:	8/18/22
		DWG. NO.:	5 OF 12
		ISSUE:	C

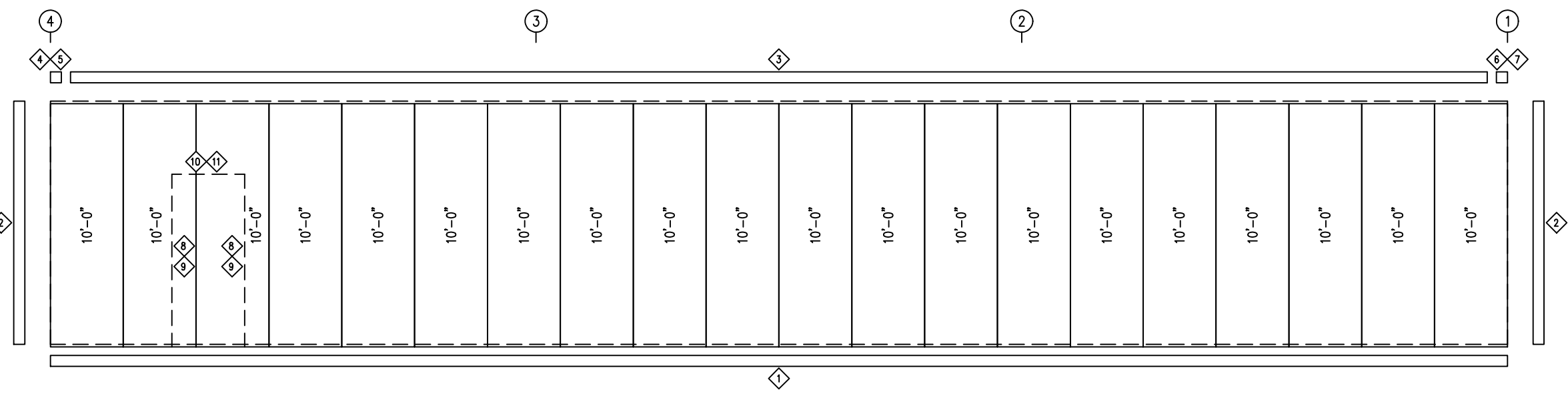


BACK SIDEWALL FRAMING: FRAME LINE D
* Field Located Framed Opening

TRIM TABLE FRAME LINE D				
ID	QUAN	PART	LENGTH	DETAIL
1	3	BA6	20'-4"	TRIM_1
2	2	OU6102	10'-2"	TRIM_30
3	3	Q7726	20'-4"	TRIM_61
4	1	Q773L6	6"	
5	1	AR961L6	7 7/16"	TRIM_60
6	1	Q773R6	6"	
7	1	AR961R6	7 7/16"	TRIM_60
8	2	Q3706	7'-4"	TRIM_50
9	2	JA6	7'-2"	TRIM_50
10	1	AR3806	3'-7"	TRIM_51
11	1	HE6	3'-6"	TRIM_51

MEMBER TABLE FRAME LINE D				
QUAN	MARK	PART	LENGTH	
2	DJ-1	8X25C16	7'-0"	
1	E-1	08536DU3	19'-11"	
1	E-2	08536DU3	19'-11"	
1	E-3	08536DU3	19'-11"	
1	G-3	8X25Z16	20'-2 1/4"	
1	G-4	8X25Z16	20'-11 1/2"	
1	G-5	8X25Z16	22'-0"	
1	G-6	8X25Z16	20'-2 1/4"	
1	G-7	8X25Z16	20'-11 1/2"	
1	GH-1	HW816Z	3'-0"	
2	CB-2	GS1716	22'-1"	

CONNECTION PLATES FRAME LINE D			
ID	QUAN	MARK/PART	
1	2	BC-05	
2	4	BC-01	
3	4	BC-07	



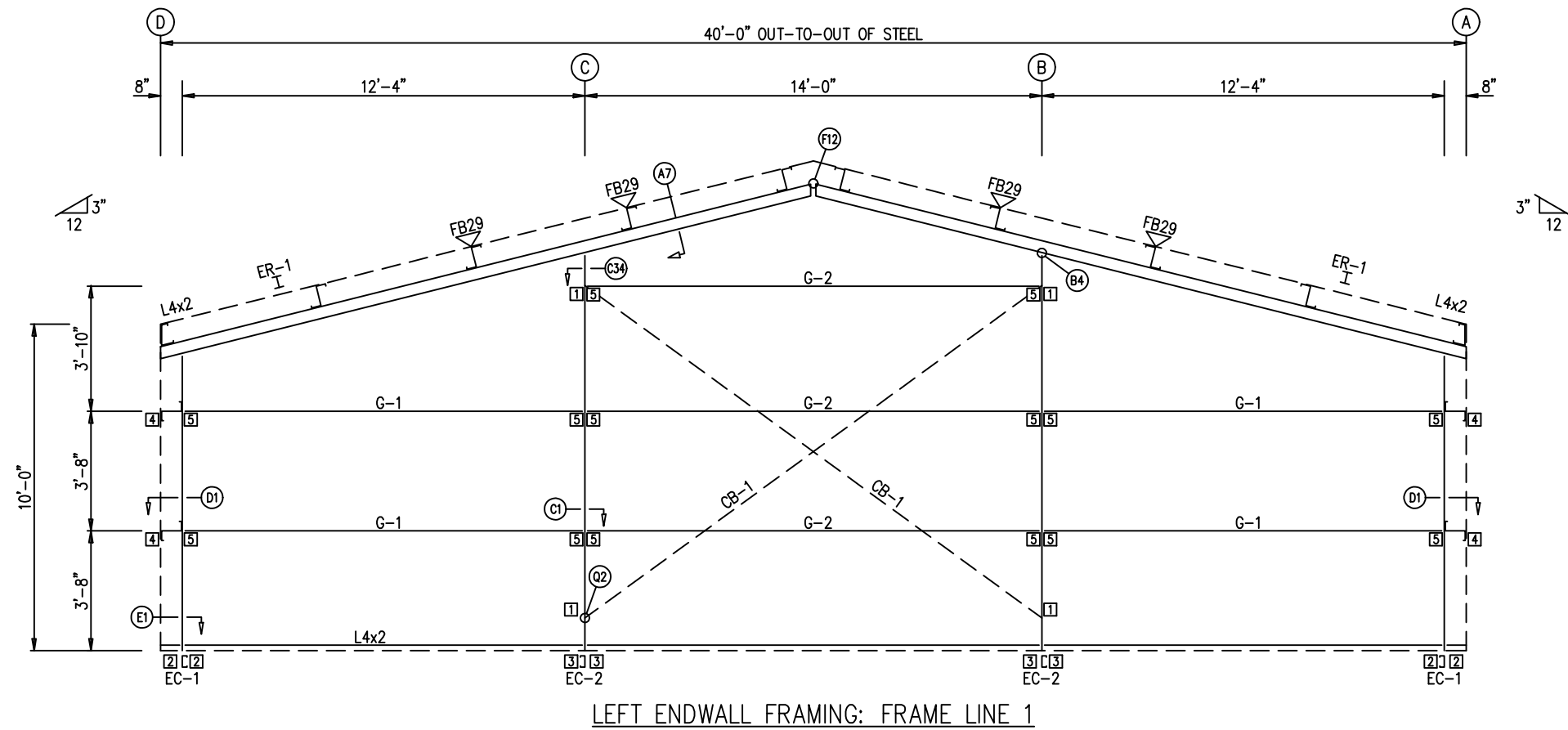
BACK SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 Ga. R-Loc - Rustic 40 yr

NOTE:
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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA



DESCRIPTION	SIDEWALL FRAMING & SHEETING		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO:	57318	ENG. BY:	RA
		DWG. NO.:	6 OF 12
		DATE:	8/18/22
		ISSUE:	C



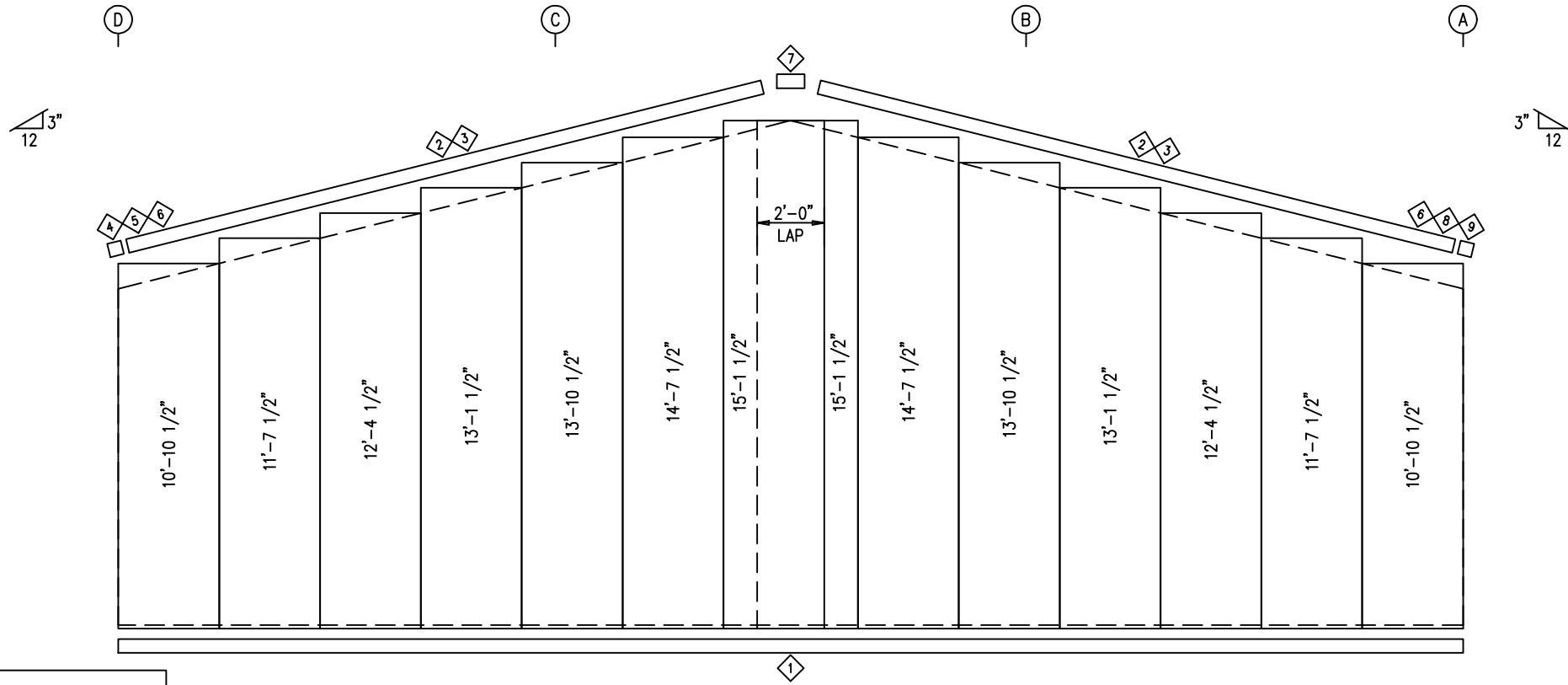
TRIM TABLE FRAME LINE 1				
ID	QUAN	PART	LENGTH	DETAIL
1	2	BA6	20'-4"	TRIM_1
2	2	Q7646102	10'-2"	TRIM_66
3	2	Q7646	12'-2"	TRIM_66
4	1	Q765L6	6"	
5	1	AR963L6	9 1/8"	
6	2	AR9626	8 1/16"	
7	1	Q7676	1'-4"	TRIM_100
8	1	Q765R6	6"	
9	1	AR963R6	9 1/8"	

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 1/2"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE FRAME LINE 1			
QUAN	MARK	PART	LENGTH
2	EC-1	8X25C16	8'-7 5/8"
2	EC-2	8X35C16	11'-8 5/8"
2	ER-1	W8X10	20'-6 7/8"
4	G-1	8X25Z16	11'-8"
3	G-2	8X25Z16	13'-11"
2	CB-1	GS1716	17'-6"

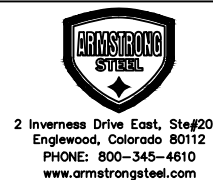
FLANGE BRACE TABLE FRAME LINE 1			
ID	QUAN	MARK	LENGTH
1	4	FB29	2'-5"

CONNECTION PLATES FRAME LINE 1		
ID	QUAN	MARK/PART
1	4	BC-50A
2	4	BC-500
3	4	BC-04
4	4	BC-42
5	14	BC-01

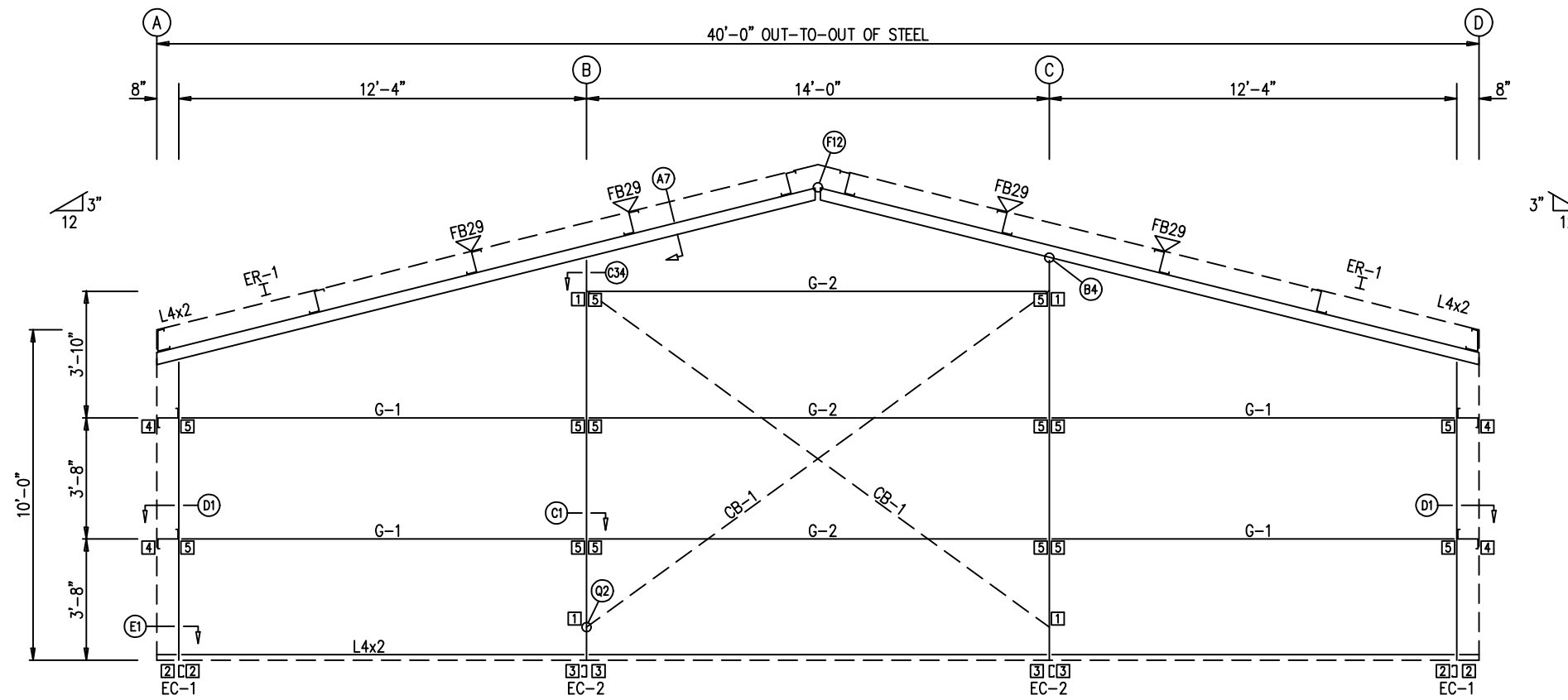


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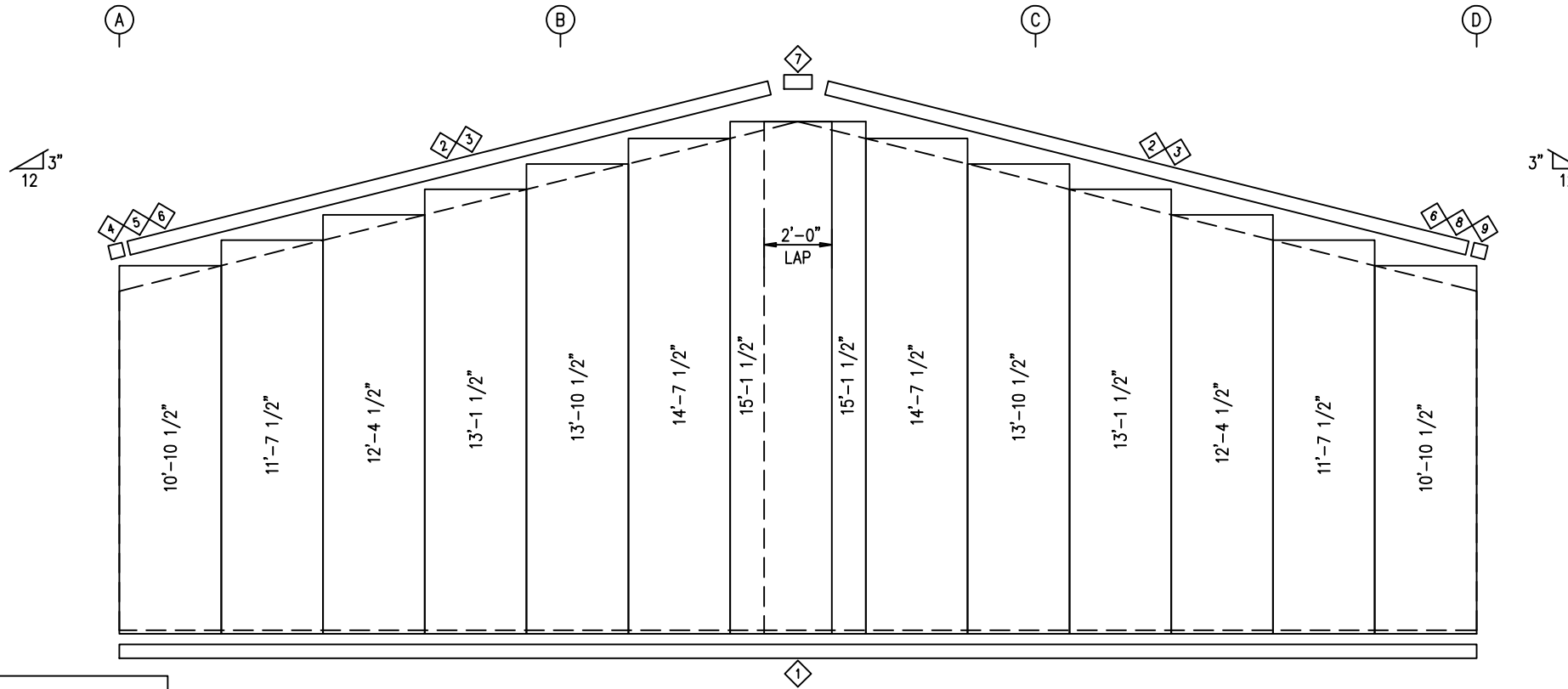
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA



DESCRIPTION	ENDWALL FRAMING & SHEETING		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO:	57318	ENG. BY:	RA
		DWG. NO.:	7 OF 12
		DATE:	8/18/22
		ISSUE:	C



RIGHT ENDWALL FRAMING: FRAME LINE 4



RIGHT ENDWALL SHEETING & TRIM: FRAME LINE 4
PANELS: 26 Ga. R-Loc - Rustic 40 yr

TRIM TABLE FRAME LINE 4				
ID	QUAN	PART	LENGTH	DETAIL
1	2	BA6	20'-4"	TRIM_1
2	2	Q7646102	10'-2"	TRIM_66
3	2	Q7646	12'-2"	TRIM_66
4	1	Q765L6	6"	
5	1	AR963L6	9 1/8"	
6	2	AR9626	8 1/16"	
7	1	Q7676	1'-4"	TRIM_100
8	1	Q765R6	6"	
9	1	AR963R6	9 1/8"	

BOLT TABLE FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 1/2"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE FRAME LINE 4			
QUAN	MARK	PART	LENGTH
2	EC-1	8X25C16	8'-7 5/8"
2	EC-2	8X35C16	11'-8 5/8"
2	ER-1	W8X10	20'-6 7/8"
4	G-1	8X25Z16	11'-8"
3	G-2	8X25Z16	13'-11"
2	CB-1	GS1716	17'-6"

FLANGE BRACE TABLE FRAME LINE 4			
ID	QUAN	MARK	LENGTH
1	4	FB29	2'-5"

CONNECTION PLATES FRAME LINE 4		
ID	QUAN	MARK/PART
1	4	BC-50A
2	4	BC-500
3	4	BC-04
4	4	BC-42
5	14	BC-01

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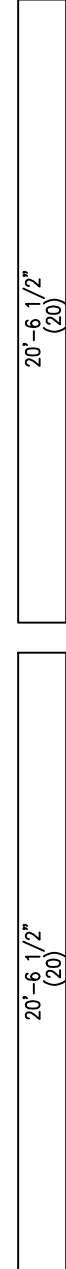
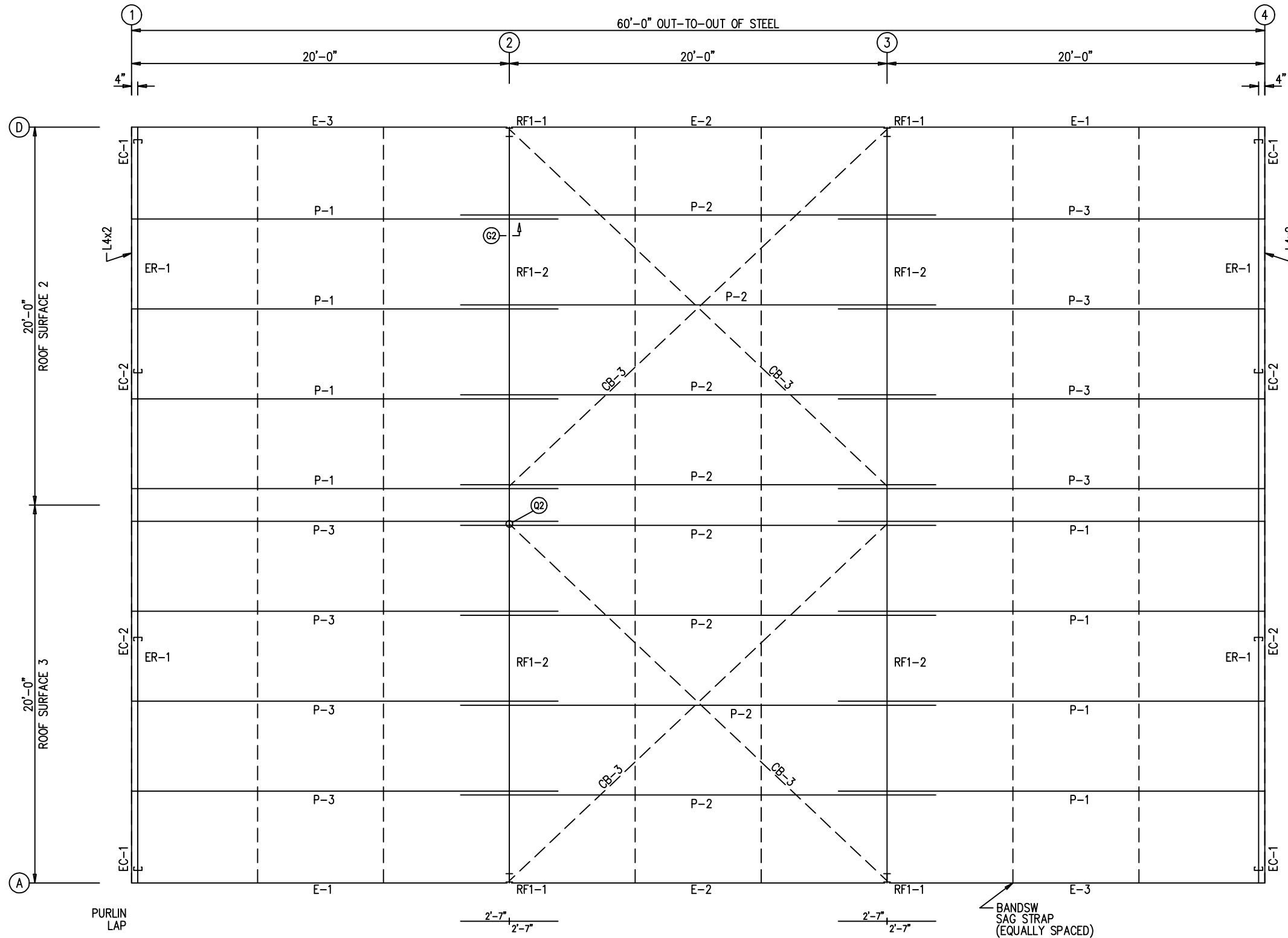
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA



DESCRIPTION: ENDWALL FRAMING & SHEETING			
CUSTOMER	END USER	SCALE	NOT TO SCALE
JOB NO: 57318	ENG. BY: RA	DATE: 8/18/22	ISSUE: C
	DWG. NO: 8 OF 12		

TRIM TABLE				
ROOF PLAN				
ID	QUAN	PART	LENGTH	DETAIL
1	6	UN6102	10'-2"	TRIM_101

MEMBER TABLE				
ROOF PLAN				
QUAN	MARK	PART	LENGTH	
8	P-1	8x25Z14	22'-6 1/2"	
8	P-2	8x25Z16	25'-2"	
8	P-3	8x25Z14	22'-6 1/2"	
2	E-1	08536DU3	19'-11"	
2	E-2	08536DU3	19'-11"	
2	E-3	08536DU3	19'-11"	
4	CB-3	GS1716	26'-8 3/4"	

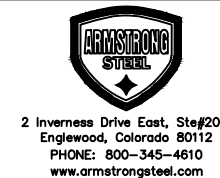


ROOF FRAMING PLAN

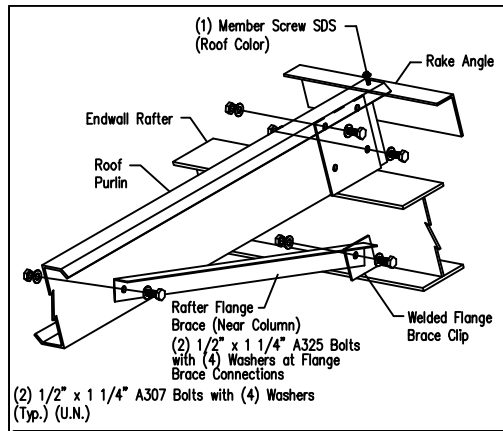
ROOF SHEETING
 PANELS: 26 Ga. R-Loc
 Hunter 40 yr

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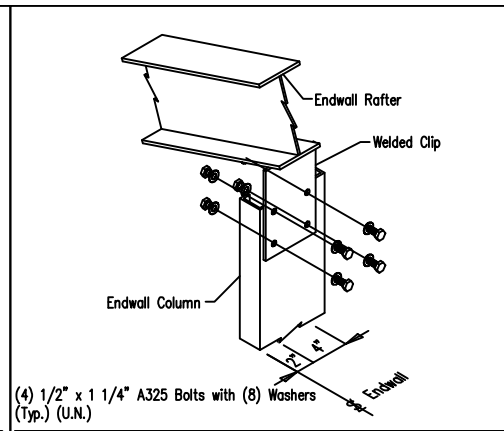
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA



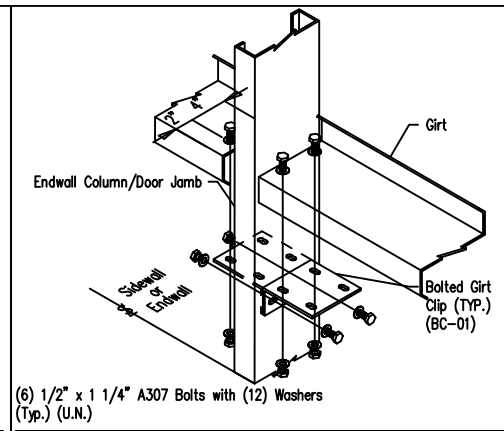
DESCRIPTION			
ROOF FRAMING & SHEETING			
CUSTOMER			
END USER			
SCALE		NOT TO SCALE	
JOB NO.:	57318	ENG. BY:	RA
		DATE:	8/18/22
DWG. NO.:	9 OF 12	ISSUE:	C



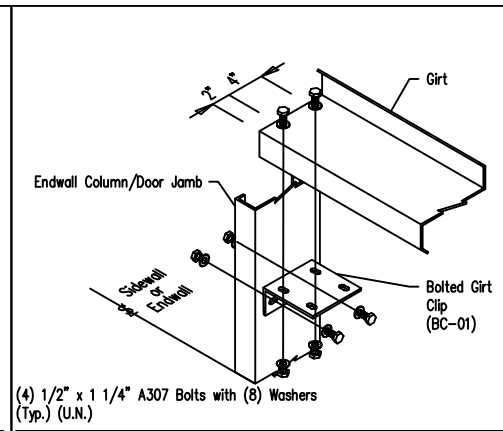
(A7) ROOF PURLIN TO HOT-ROLLED RAFTER
 (2) 1/2" x 1 1/4" A307 Bolts with (4) Washers (Typ.) (U.N.)



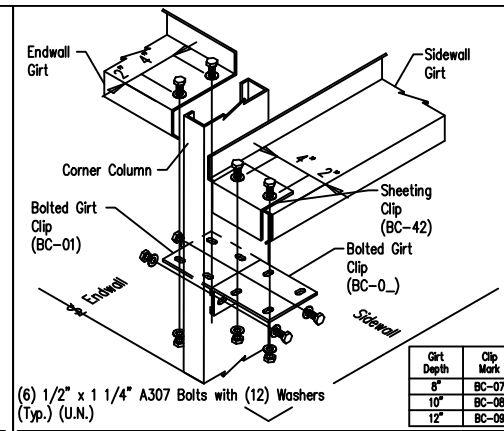
(B4) CEE ENDWALL COLUMN TO ENDWALL RAFTER
 (4) 1/2" x 1 1/4" A325 Bolts with (8) Washers (Typ.) (U.N.)



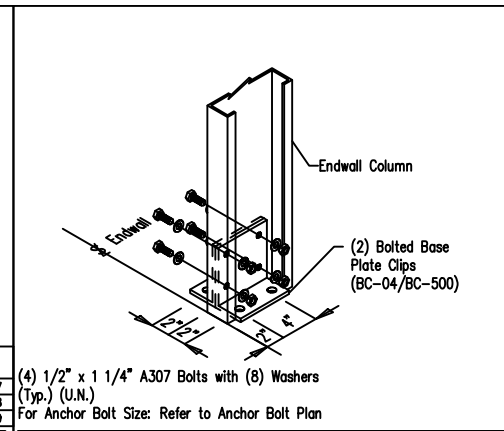
(C1) WALL GIRTS TO ENDWALL COLUMN/DOOR JAMB
 (6) 1/2" x 1 1/4" A307 Bolts with (12) Washers (Typ.) (U.N.)



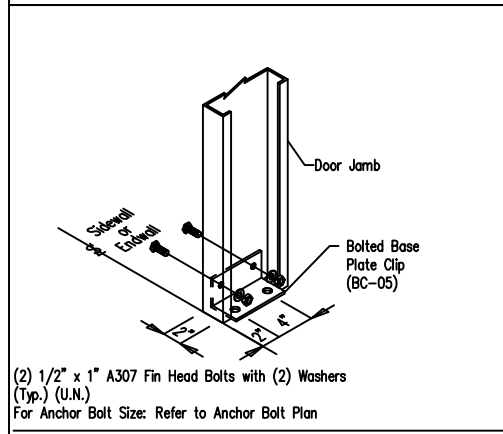
(C34) WALL GIRTS TO ENDWALL COLUMN/DOOR JAMB
 (4) 1/2" x 1 1/4" A307 Bolts with (8) Washers (Typ.) (U.N.)



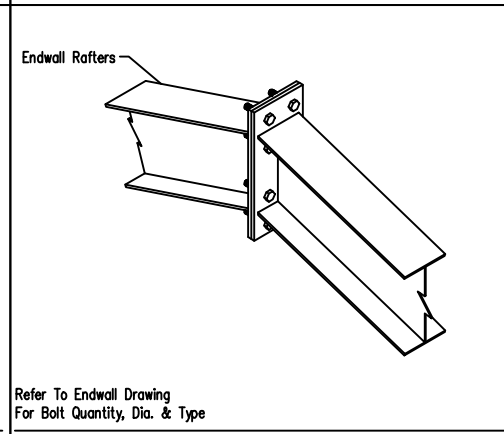
(D1) WALL GIRTS TO ENDWALL CORNER COLUMN
 (6) 1/2" x 1 1/4" A307 Bolts with (12) Washers (Typ.) (U.N.)



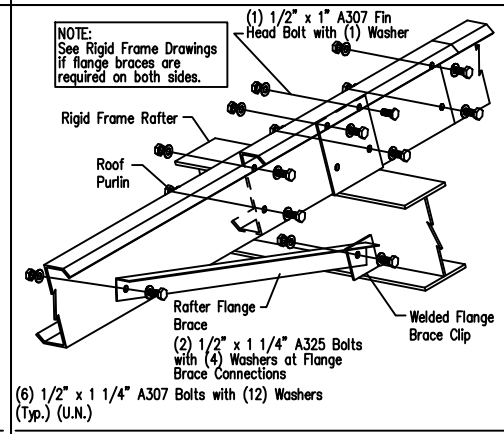
(E1) BASE PLATE FOR ENDWALL COLUMN
 (4) 1/2" x 1 1/4" A307 Bolts with (8) Washers (Typ.) (U.N.)
 For Anchor Bolt Size: Refer to Anchor Bolt Plan



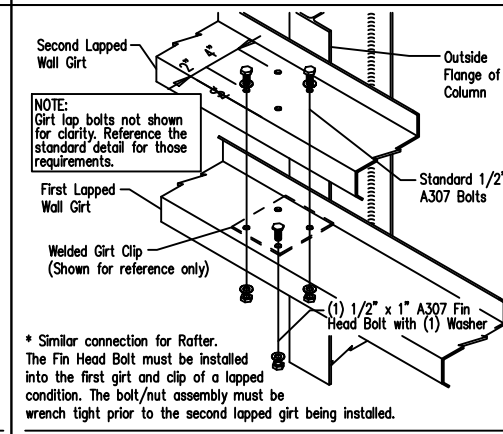
(E6) BASE PLATE FOR DOOR JAMB
 (2) 1/2" x 1" A307 Fin Head Bolts with (2) Washers (Typ.) (U.N.)
 For Anchor Bolt Size: Refer to Anchor Bolt Plan



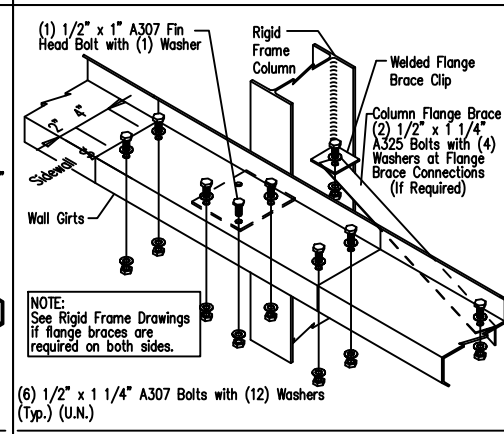
(F12) RAFTER SPLICE ALONG SURFACE
 Refer To Endwall Drawing For Bolt Quantity, Dia. & Type



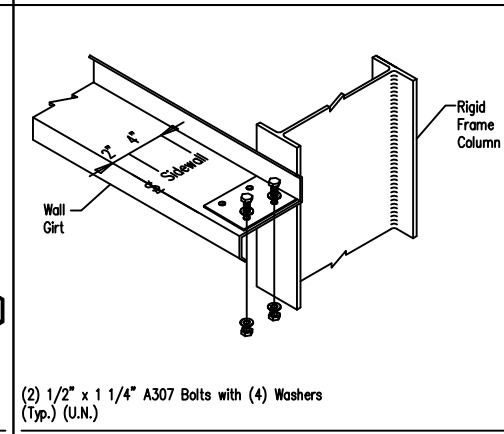
(G2) ROOF PURLIN TO INTERIOR RIGID FRAME
 (6) 1/2" x 1 1/4" A307 Bolts with (12) Washers (Typ.) (U.N.)



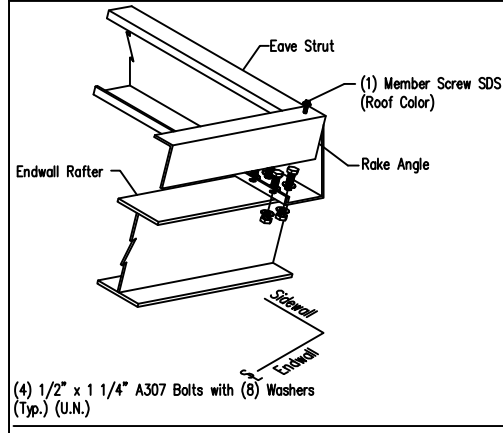
(H0) BYPASS LAPPED WALL GIRTS
 * Similar connection for Rafter. The Fin Head Bolt must be installed into the first girt and clip of a lapped condition. The bolt/nut assembly must be wrench tight prior to the second lapped girt being installed.



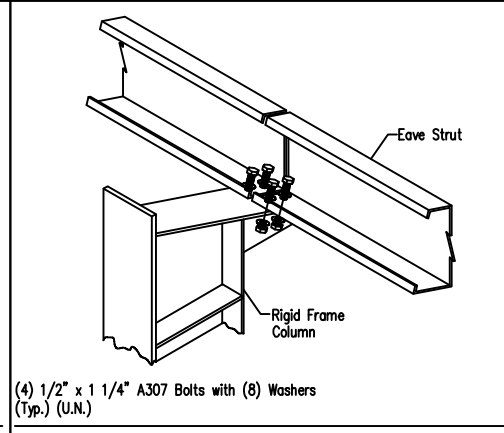
(H2) WALL GIRTS TO RIGID FRAME COLUMN
 (6) 1/2" x 1 1/4" A307 Bolts with (12) Washers (Typ.) (U.N.)



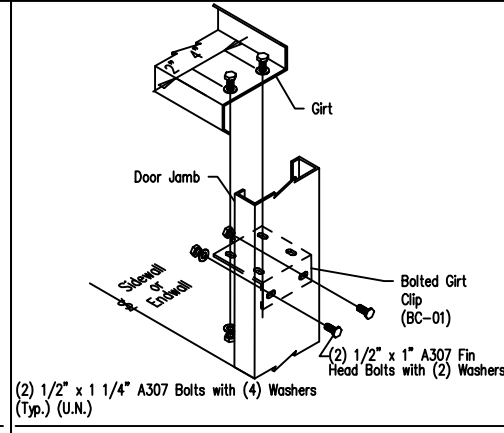
(H7) WALL GIRTS TO RIGID FRAME COLUMN
 (2) 1/2" x 1 1/4" A307 Bolts with (4) Washers (Typ.) (U.N.)



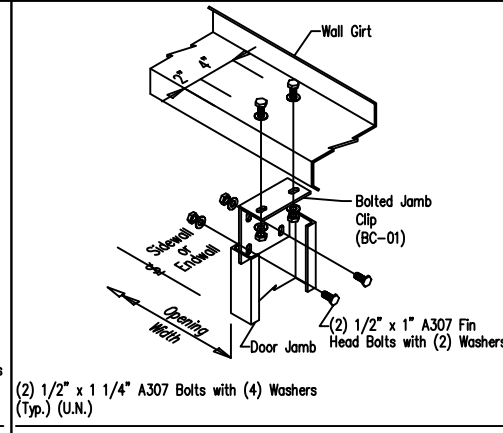
(I8) EAVE STRUT TO ENDWALL RAFTER
 (4) 1/2" x 1 1/4" A307 Bolts with (8) Washers (Typ.) (U.N.)



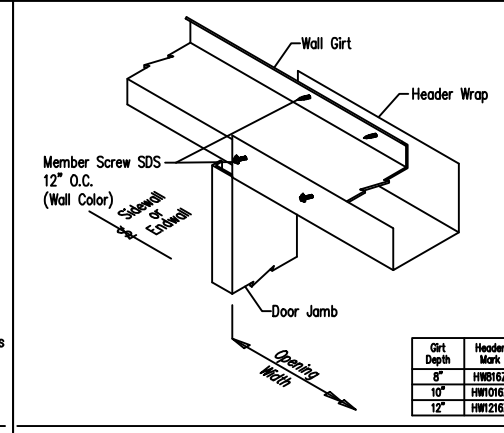
(J2) LOWSIDE EAVE STRUT TO BYPASS RIGID FRAME
 (4) 1/2" x 1 1/4" A307 Bolts with (8) Washers (Typ.) (U.N.)



(K2) WALL GIRTS TO DOOR JAMB
 (2) 1/2" x 1 1/4" A307 Bolts with (4) Washers (Typ.) (U.N.)




(L6) DOOR JAMB TO WALL GIRTS
 (2) 1/2" x 1 1/4" A307 Bolts with (4) Washers (Typ.) (U.N.)



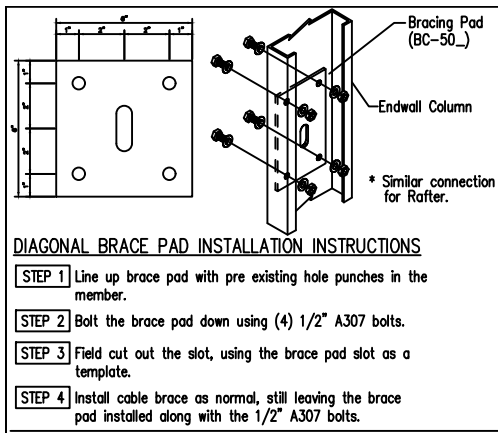
(M15) HEADER WRAP TO WALL GIRTS

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/20/23	RB	DP	RA

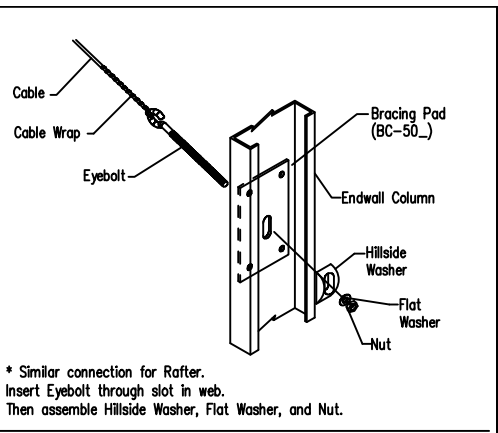


2 Inverness Drive East, Ste#200
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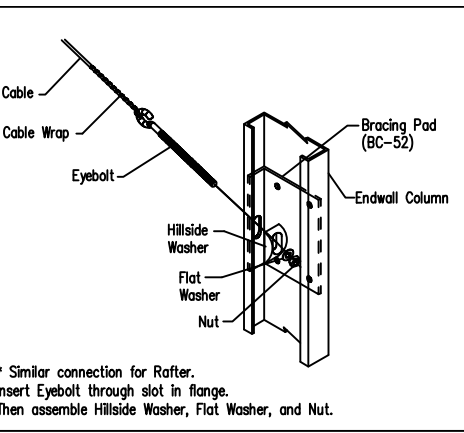
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57318	ENG. BY: RA DATE: 8/18/22
	DWG. NO.: 10 OF 12 ISSUE: C



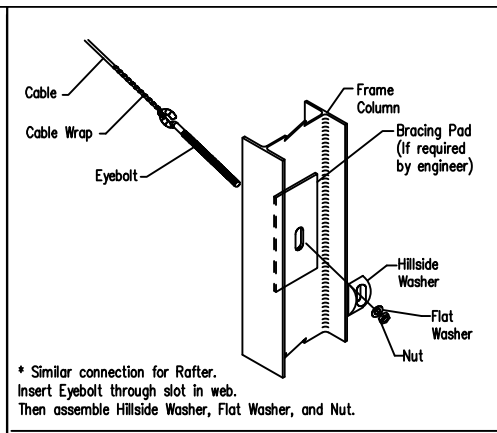
Q2 DIAGONAL BRACE PAD TO WEB OF CEE COLUMN



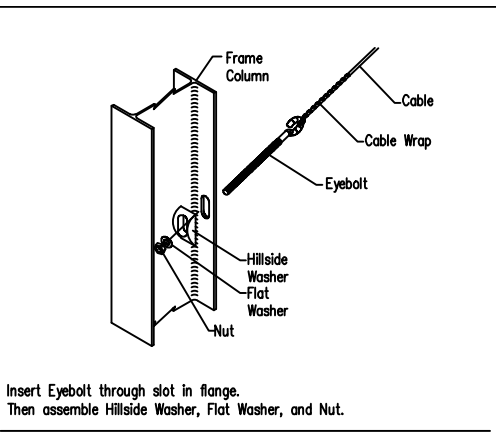
DIAGONAL CABLE BRACE TO WEB OF CEE COLUMN



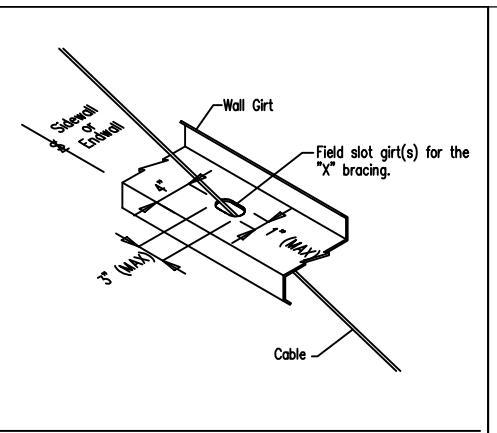
DIAGONAL CABLE BRACE TO FLANGE OF CEE COLUMN



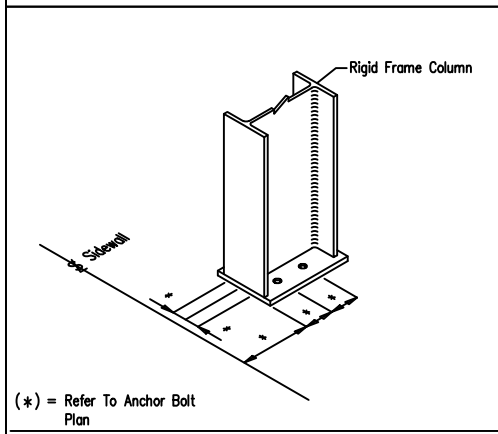
DIAGONAL CABLE BRACE TO WEB OF FRAME COLUMN



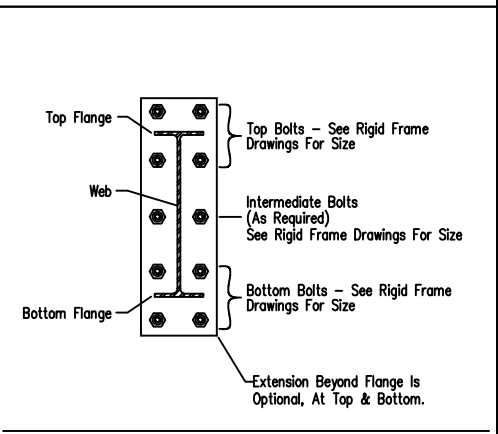
DIAGONAL CABLE BRACE TO FLANGE OF FRAME COLUMN



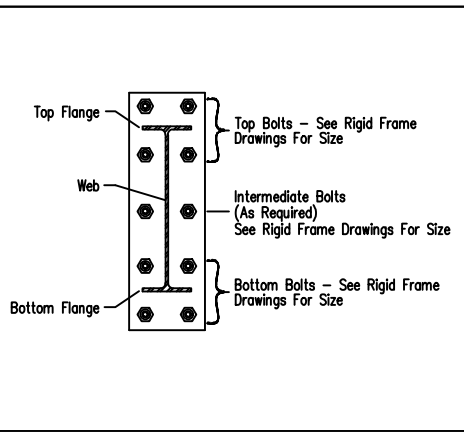
DIAGONAL CABLE BRACE AT FLUSH WALL GIRTS



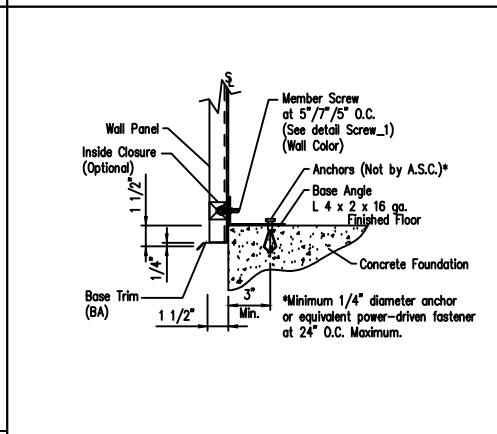
R2 ANCHOR BOLTS AT SIDEWALL COLUMNS



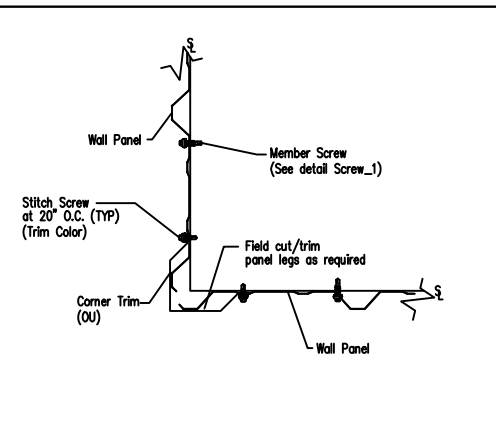
U2 BOLTS FOR RIGID FRAME RAFTER AT BUILDING PEAK



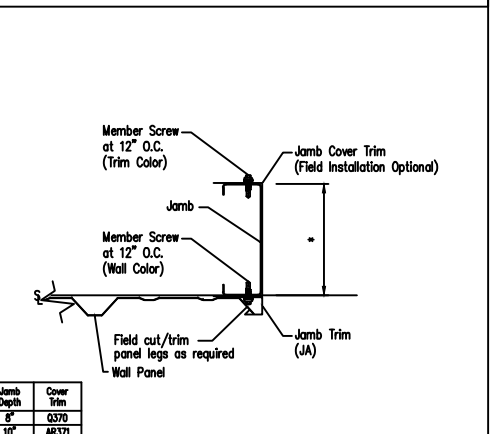
U3 BOLTS FOR RIGID FRAME RAFTER TO COLUMN CONNECTION



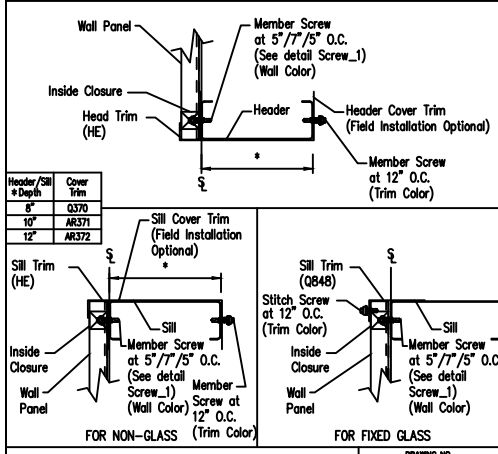
BASE ANGLE DETAIL WITH TRIM TRIM_1



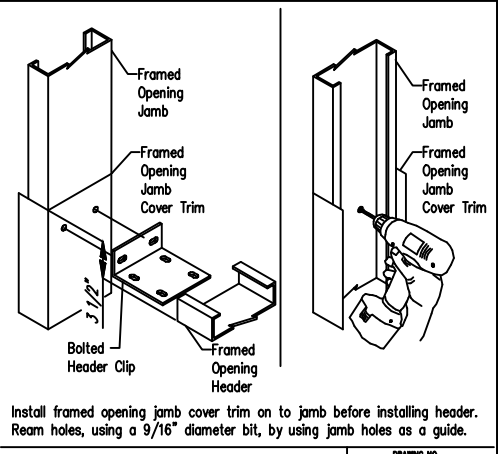
OUTSIDE CORNER DETAIL TRIM_30



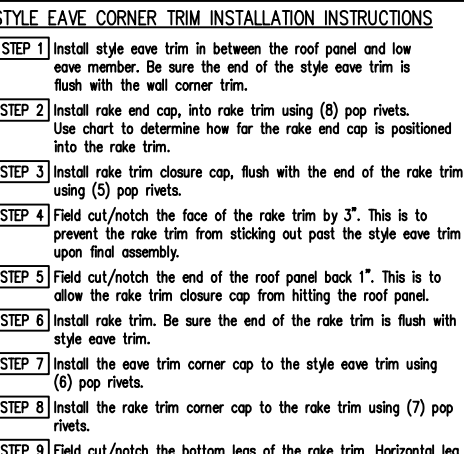
FRAMED OPENING JAMB TRIM DETAIL TRIM_50



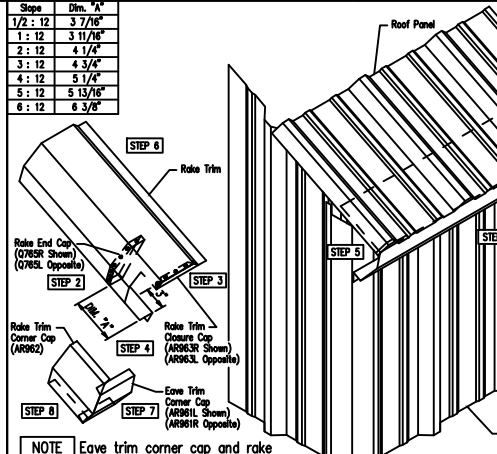
FRAMED OPENING HEAD & SILL TRIM DETAILS TRIM_51



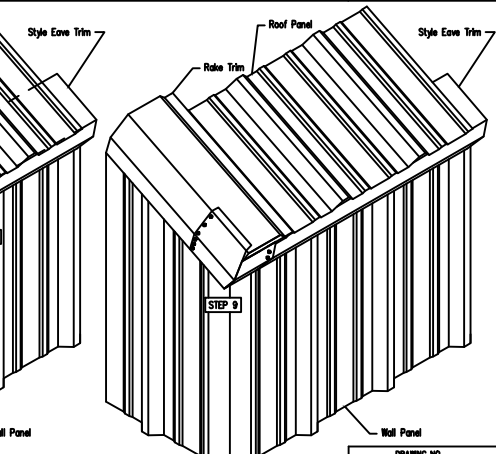
COVER TRIM INSTALLATION INSTRUCTIONS TRIM_52



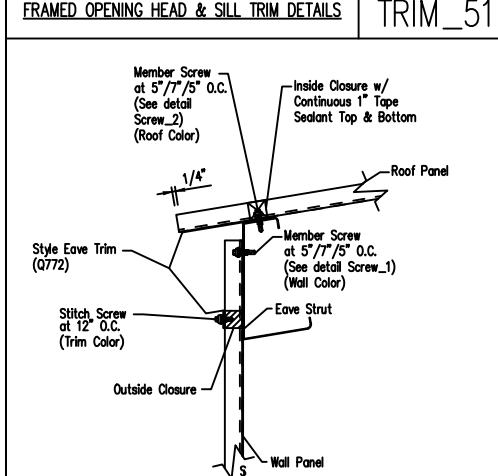
STYLE EAVE CORNER TRIM INSTALLATION INSTRUCTIONS



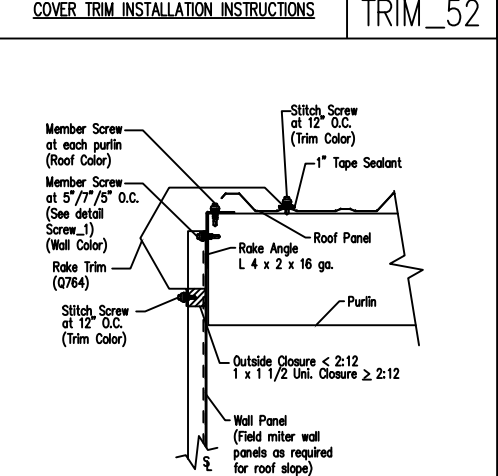
TRIM_60



RAKE DETAIL AT SHEETED WALL TRIM_66



EAVE DETAIL WITH STYLE EAVE AT SHEETED WALL TRIM_61

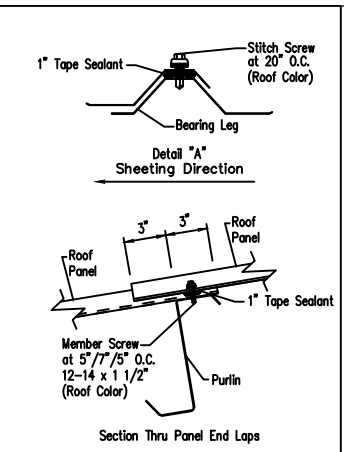
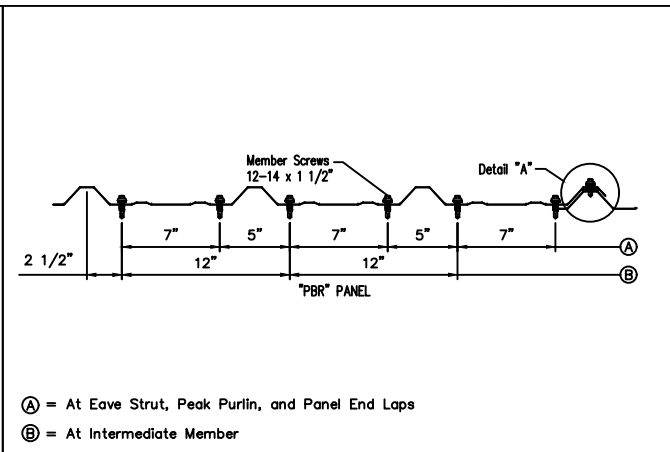
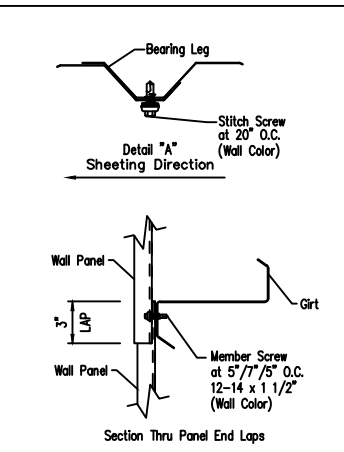
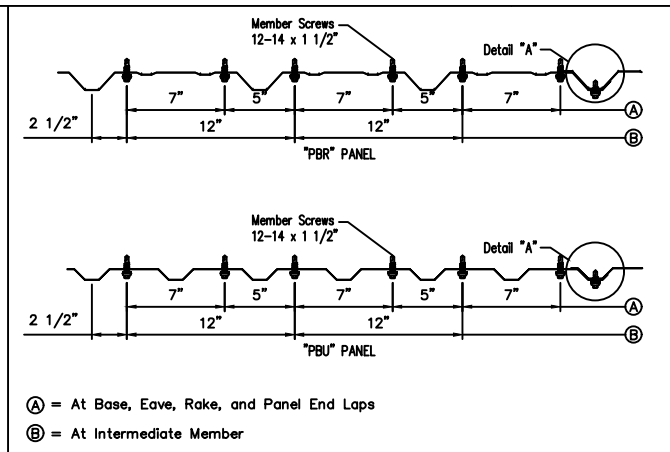
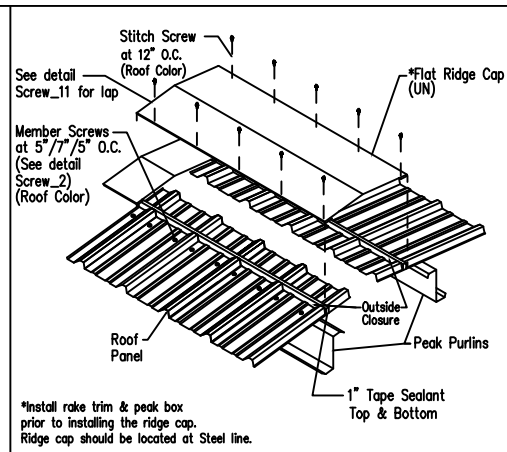
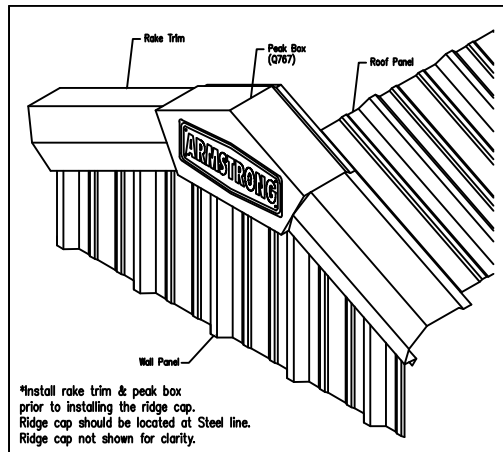


RAKE DETAIL AT SHEETED WALL TRIM_66

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/10/23	RB	DP	RA

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DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.:	57318
ENG. BY:	RA
DATE:	8/18/22
DWG. NO.:	11 OF 12
ISSUE:	C

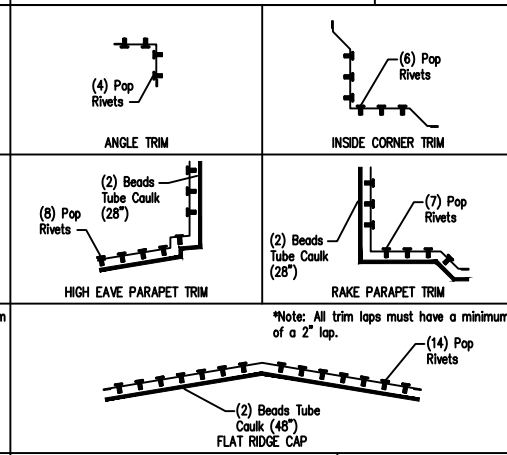
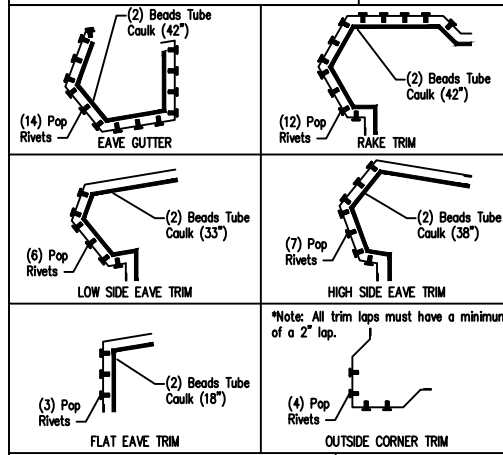


STANDARD PEAK BOX DETAIL TRIM_100

RIDGE DETAIL WITH FLAT RIDGE TRIM_101

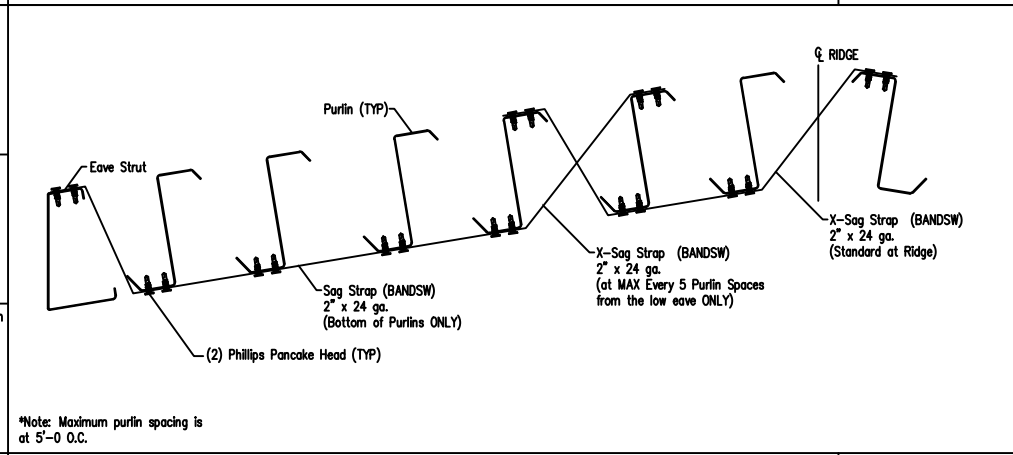
FASTENER LOCATION FOR WALL PANELS SCREW_1

FASTENER LOCATION FOR ROOF PANELS SCREW_2

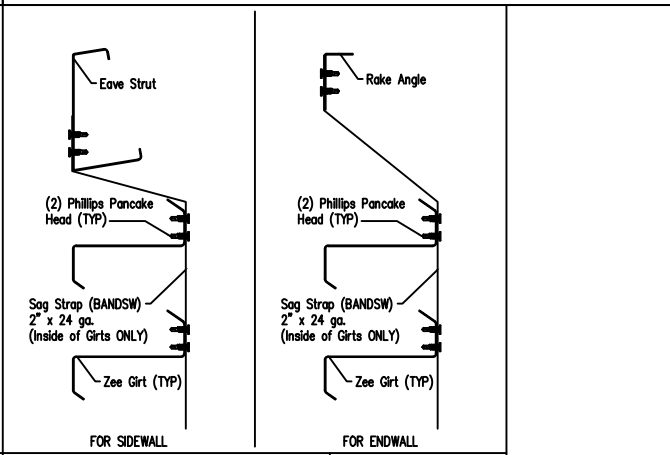


TRIM LAPS SCREW_10

TRIM LAPS SCREW_11

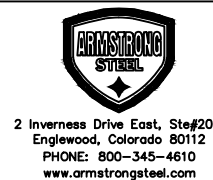


TYPICAL SAG STRAP AT GABLED ROOF SCREW_15



TYPICAL SAG STRAP AT WALLS SCREW_17

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	8/18/22	SN	DP	RA
C	CONSTRUCTION	2/10/23	RB	DP	RA



DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 57318	ENG. BY: RA DATE: 8/18/22
	DWG. NO.: 12 OF 12 ISSUE: C