

GENERAL NOTES

1.1 Fabrication shall be in accordance with A.S.C. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".

MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
Hot Rolled Steel Shapes (W, & C)	A572	Fy = 50 KSI
Hot Rolled Steel Angles (L)	A36	Fy = 36 KSI
Steel Pipes	A500	Fy = 42 KSI
Structural Tubing	A500	Fy = 42 KSI
Structural Steel Web Plate	A572/A1011	Fy = 50 KSI
Structural Steel Flange Plates/Bars	A529/A572	Fy = 55 KSI
Cold Formed Light Gage	A653/A1011	Fy = 55 KSI
Roof and Wall Sheets	A792/A653	Fy = 50, 80 KSI
Cable Brace	A475 - TYPE 1	Extra High Strength
Rod Brace	A529	Fy = 50 KSI
MIN. TENSILE STRENGTH		
Machine Bolts & Nuts	A307	Fu = 60 KSI
High Strength Bolts (1"Ø and less)	A325-TYPE 1	Fu = 120 KSI
High Strength Bolts (>1"Ø to 1 1/2"Ø)	A325-TYPE 1	Fu = 105 KSI
Anchor Bolts (Not supplied by A.S.C.)	A36/A307/F1554	Fu = 60 KSI

1.3 **PRIMER**
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is A.S.C. Gray Oxide color. This paint is not intended for long term exposure to the elements. A.S.C. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. A.S.C. shall not be responsible for any field applied paint and/or coatings. (AISC Code of Standard Practice, Latest Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.

1.4 **GALVANIZED OR SPECIAL COATINGS:**
See Contract Documents

1.5 **ALL BOLTS ARE 1/2"Ø x 0'-1 1/4" A307 EXCEPT :**
a) Endwall rafter splice - 5/8"Ø x 0'-1 3/4" A325-N
b) Endwall column to rafter connection - 1/2"Ø x 0'-1 1/4" A325 MIN.(SEE WALL ELEVATION)
c) Main frame connections - SEE CROSS SECTION
d) Flange Brace connections - 1/2"Ø x 0'-1 1/4" A325
NOTE: Washers are not supplied unless noted otherwise on drawing

1.6 **A325 BOLT TIGHTENING REQUIREMENTS**
All high strength bolts are A325-N unless specifically noted otherwise. Holes are not slotted and design is bearing connection. Structural bolts shall be tightened by the turn-of-the-nut method in accordance with the Latest Edition AISC "Specification For Structural Joints " using ASTM A325 or A490 Bolts, when specifically required. A325-N bolts are supplied without washer unless otherwise noted on the drawings.
All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.

1.7 **CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:**
INSIDE- Under roof panels & base of wall panels
OUTSIDE - Between roof panels & ridge cap
- Between wall panels & eave/gable trim

1.8 **ERECTION NOTE:**
All bracing, strapping, & bridging shown and provided by A.S.C. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.9 **ERECTION AND UNLOADING NOT BY A.S.C.**

1.10 **SHORTAGES**
Any claims or shortages by buyer must be made to A.S.C. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 **CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)**
Claims for correction of alleged misfits will be disallowed unless A.S.C. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of A.S.C.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- 2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release A.S.C. to fabricate upon receiving such.
- 2.2 Armstrong Steel Corp (hereafter referred to as A.S.C.) standard specifications apply unless stipulated otherwise in the Contract Documents. A.S.C. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.
- 2.3 In case of discrepancies between A.S.C. structural steel plans and plans for other trades, A.S.C. plans shall govern. (Section. 3 AISC Code of Standard Practices, Latest Edition)
- 2.4 Approval of A.S.C. drawings and calculations indicates that A.S.C. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the A.S.C. design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)
- 2.5 Once the BUYER/END USE CUSTOMER has signed A.S.C. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.
- 2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by A.S.C. and A.S.C. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or A.S.C. assumptions will govern (AISC Code of Standard Practice, Latest Edition)



PHONE: 800-345-4610
www.armstrongsteel.com

JOB NO. : 58143

CUSTOMER :
END USER :
END USE :
LOCATION :
PH. NO. :

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:	BUILDING DESCRIPTION:
Design Code / Wind Code	:IBC-21
Building Risk Category	:II - Normal
Enclosure	:Enclosed
Dead Load (psf)	:2.35
Collateral Load (psf)	:1.00
Wind Load	
Ultimate Wind Speed, (Vult) (mph)	:115.00
Wind Exposure	:C
Internal Pressure Coefficient, GCpi	:0.18 /-0.18
Wall Panel Design Wind Pressure (psf)	:24.00 /-26.10
Live Load	
Primary Framing (psf)	:20.00
Trib. Area Reduction	:Yes
Secondary Framing (psf)	:20.00
Snow Load	
Ground Snow Load, Pg (psf)	:30.00
Roof Snow Load, Pf (psf)	:30.00
Sloped Roof Snow Load, Ps (psf)	:30.00
Snow Exposure Factor, Ce	:1.00
Snow Importance Factor, Is	:1.00
Thermal Factor, Ct	:1.20
Seismic Load	
Seismic Importance Factor, Ie	:1.00
Site Class	:D
Mapped Spectral Response Acceleration	:Ss = 0.349 :S1 = 0.085
Spectral Response Coefficients	:Sds = 0.354 :Sd1 = 0.137
Seismic Design Category	:C
Basic Force Resisting Systems Used	:Steel System Not Specifically Detailed For Resistance
	:Rigid Frames (OMF)
	:Braced Frames (OCBF/OMF)
Total Design Base Shear, V (kips)	:Longitudinal = 1.50
	:Transverse = 1.68
Response Modification Factors, R	:Rigid Frames = 3.00 Ω = 3.00
	:SW X-Bracing = 3.00 Ω = 3.00
Seismic Response Coefficient, Cs	:Rigid Frames = 0.1181
	:SW X-Bracing = 0.1181

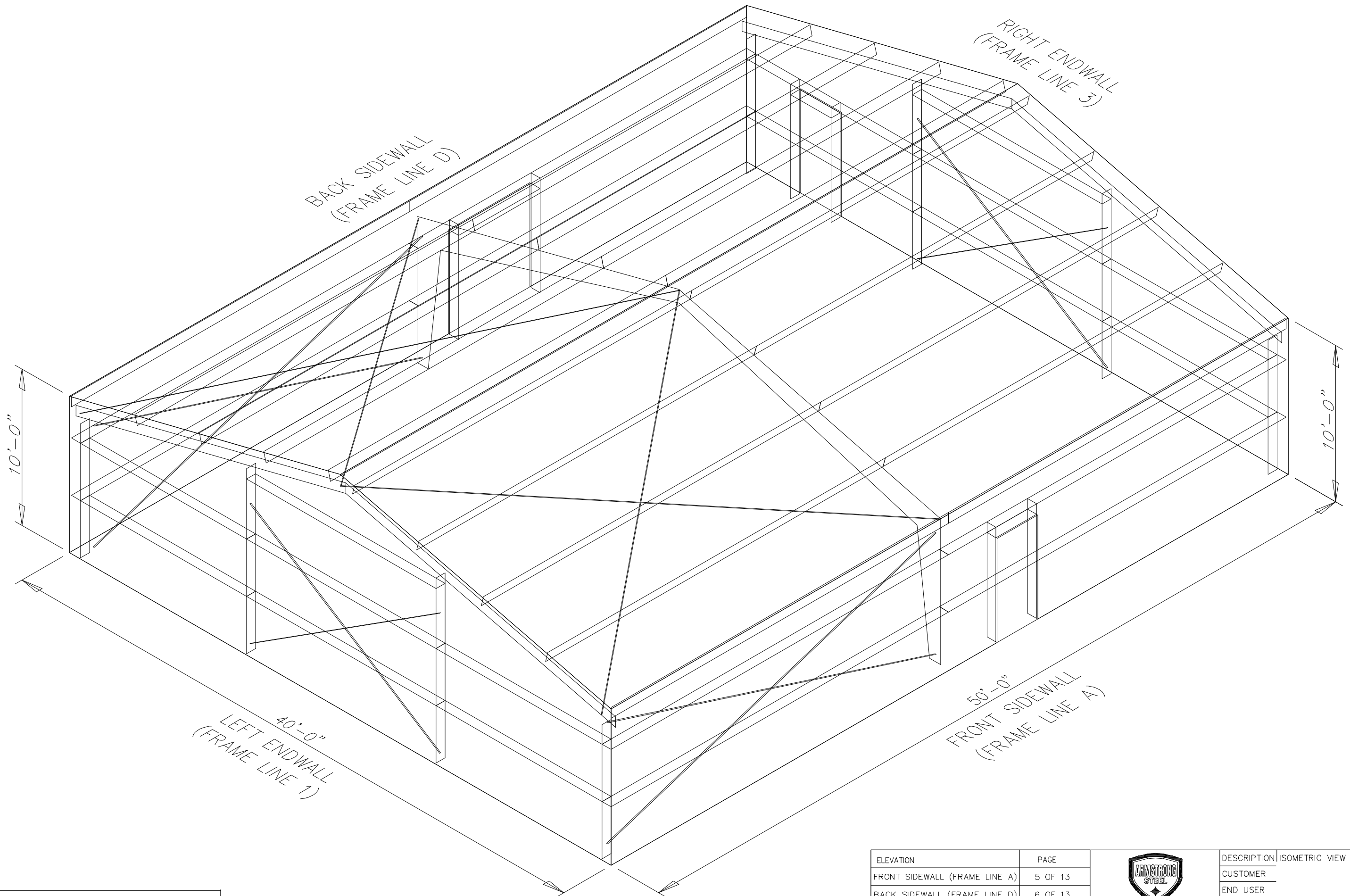
Drawing Index	
Drawing Name	Page(s)
Drawing Cover	COVER
3D Reference	3D REF
Anchor Bolt Plan	1
Anchor Bolt Details	2
Anchor Bolt Reactions	3
Rigid Frame	4
Front Sidewall	5
Back Sidewall	6
Left Endwall	7
Right Endwall	8
Roof Plan	9
Details	10-13

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT ARMSTRONG STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY A.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ARMSTRONG ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

BUYER/END USE CUSTOMER RESPONSIBILITIES CONTINUED

- 2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that A.S.C. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that A.S.C. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by A.S.C.
- 2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with A.S.C. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined, furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (AISC Code of Standard Practice, Latest Edition.)
- 2.9 Armstrong Steel Corp is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete. Unless otherwise provided in the Order Documents, A.S.C. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site. It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Latest MBMA Low Rise Building Systems Manual)
- 2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to A.S.C. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (AISC Code of Standard Practice Latest Edition)
- 2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (AISC Code of Standard Practice Latest Edition)
- 2.12 **WARNING:** In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto Galvalume should be avoided.
- 2.13 **SAFETY COMMITMENT:** Armstrong Steel Corp has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of A.S.C. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.
- 2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.
- 2.15 It is recommended by Factory Mutual (Reference: B2.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.
- | ROOF SNOW LOAD (IN PSF) | EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES) | RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES) |
|-------------------------|--|--|
| 20 | 16.60 | 8.30 |
| 25 | 17.25 | 8.62 |
| 30 | 17.90 | 8.95 |
| 35 | 18.55 | 9.28 |
| 40 | 19.20 | 9.60 |
| 45 | 19.85 | 9.92 |
| 50 | 20.50 | 10.25 |
| 55 | 21.15 | 10.58 |
| 60 | 21.80 | 10.90 |
| 65 | 22.45 | 11.22 |
| 70 | 23.10 | 11.55 |
| 75 | 23.75 | 11.88 |
| 80 | 24.40 | 12.20 |
- NOTE:
For Snow/Ice Removal Procedure, Refer to Metal Building System Manual 2002 Edition, Section A8.4, Page XI-A8-2.
- Drawing Status**
- APPROVAL:** **REVISED APPROVAL:**
These drawings, being for approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "Construction" can be considered as complete.
- PERMIT:** **REVISED PERMIT:**
These drawings, being for permit, are by definition not final. Only drawings issued "Construction" can be considered as complete.
- CONSTRUCTION:**
Final drawings to be used in the erection of the building.

JOB NO : 58143



NOTE:
 3D IS A GENERAL REPRESENTATION OF BUILDING.
 SOME MEMBERS MAY CHANGE IN FINAL ERECTION DRAWINGS

ELEVATION	PAGE
FRONT SIDEWALL (FRAME LINE A)	5 OF 13
BACK SIDEWALL (FRAME LINE D)	6 OF 13
LEFT ENDWALL (FRAME LINE 1)	7 OF 13
RIGHT ENDWALL (FRAME LINE 3)	8 OF 13

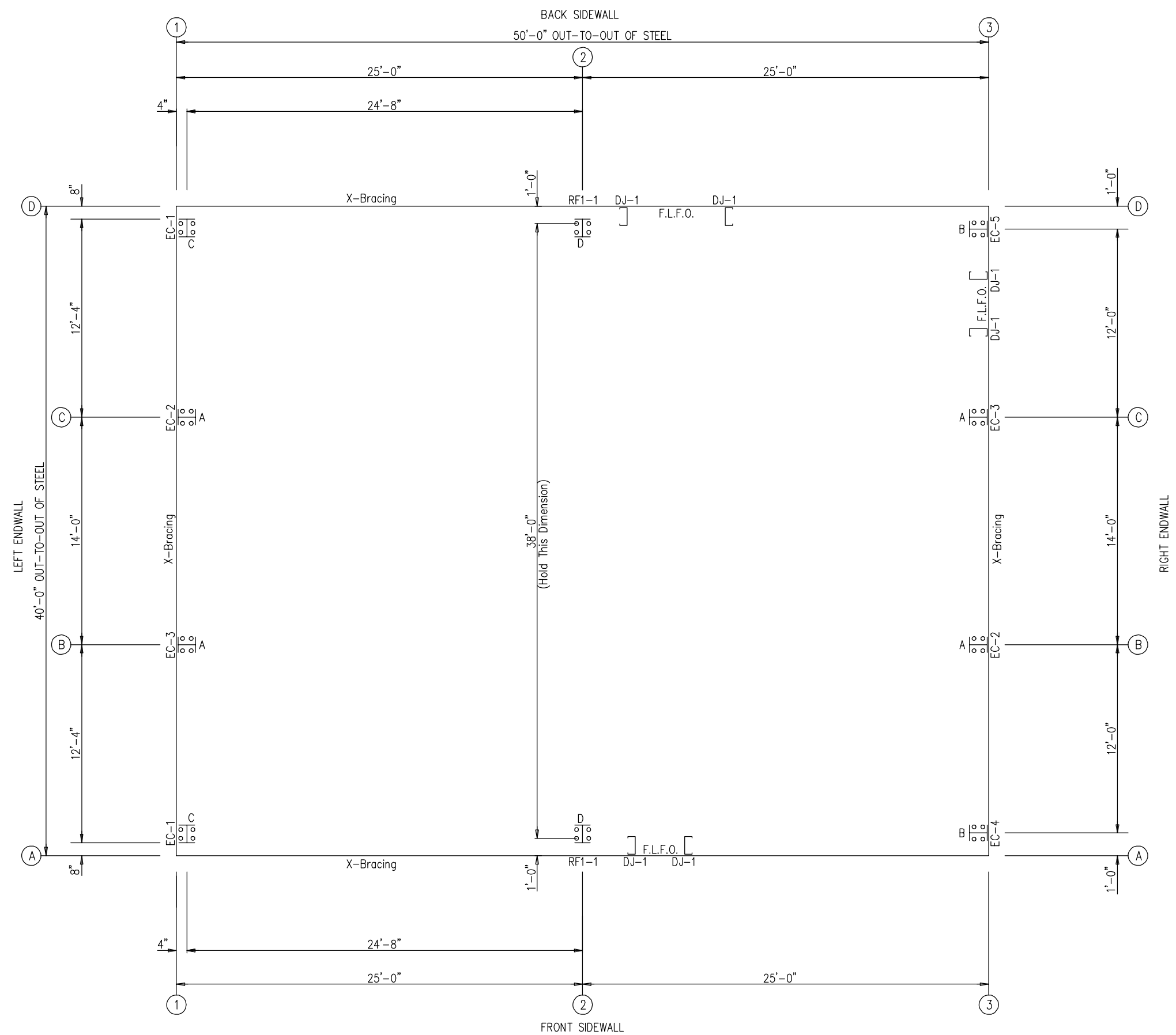


2 Inverness Drive East, Ste#200
 Englewood, Colorado 80112
 PHONE: 800-345-4610
 www.armstrongsteel.com

DESCRIPTION	ISOMETRIC VIEW
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58143	ENG. BY: MZ
DWG. NO.: 3D REFERENCE	DATE: 4/25/24
	ISSUE: C

ANCHOR BOLT SUMMARY

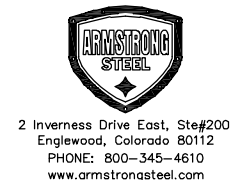
Qty	Locate	Dia (in)	Type
32	Endwall	3/4"	A307
8	Frame	3/4"	A307



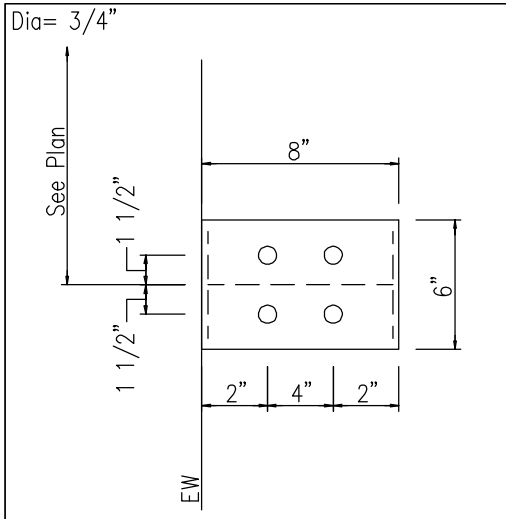
NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT PLAN
 NOTE: All Base Plates @ 100'-0" (U.N.)
 F.L.F.O. =Field Located Framed Opening

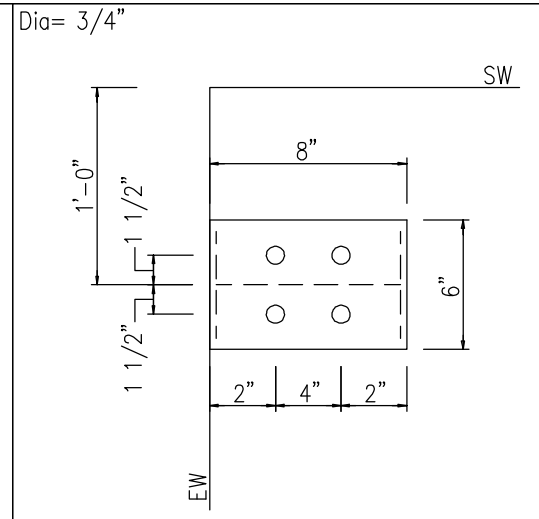
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ



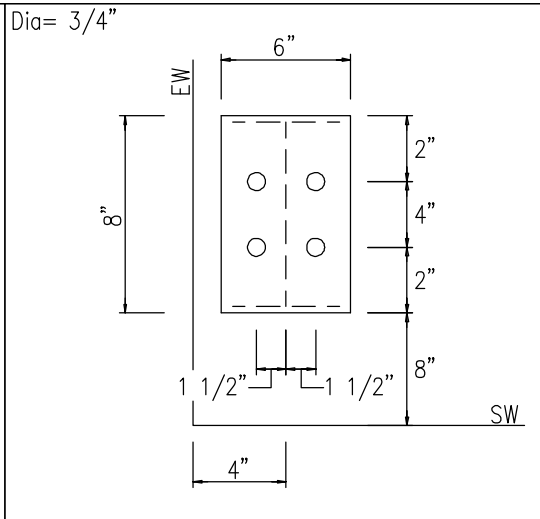
DESCRIPTION	ANCHOR BOLT PLAN		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	ENG. BY:	DATE:	
58143	MZ	4/25/24	
DWG. NO.:	1 OF 13	ISSUE:	C



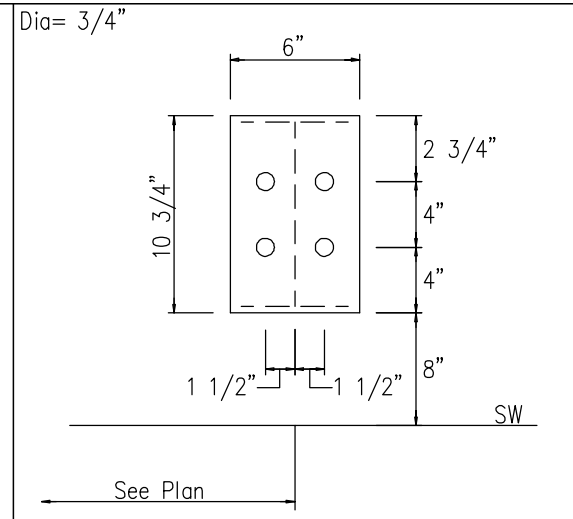
DETAIL A



DETAIL B



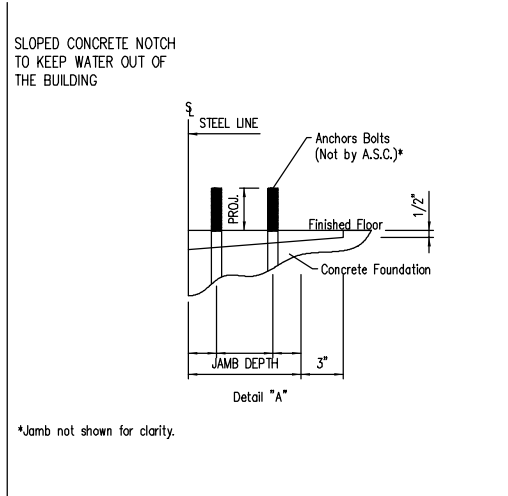
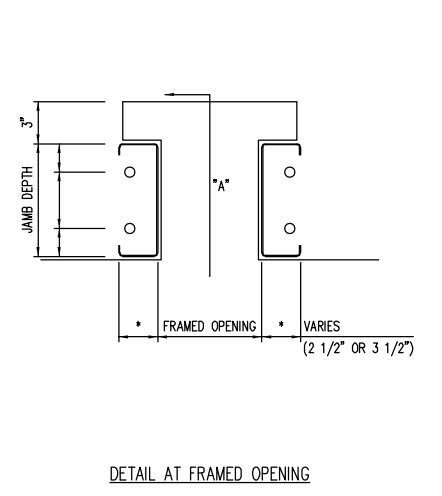
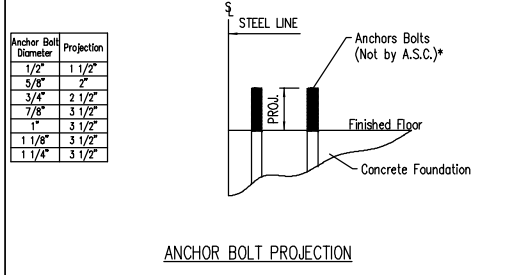
DETAIL C



DETAIL D

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING MANUFACTURER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.
 DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.
 NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.

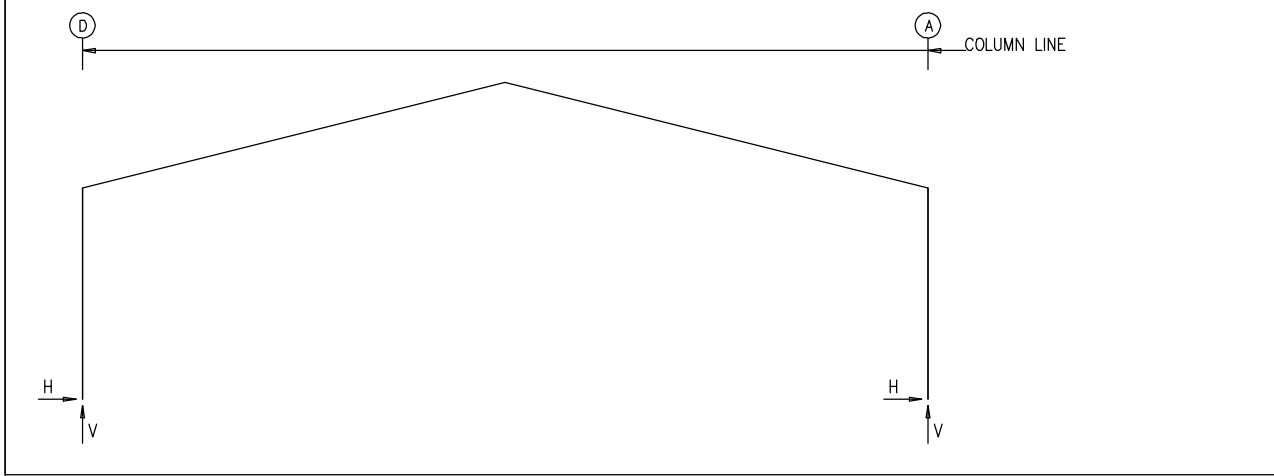


*Jamb not shown for clarity.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

2 Inverness Drive East, Ste#200
 Englewood, Colorado 80112
 PHONE: 800-345-4610
 www.armstrongsteel.com

DESCRIPTION	ANCHOR BOLT DETAILS	
CUSTOMER		
END USER		
SCALE	NOT TO SCALE	
JOB NO.:	58143	
ENG. BY:	MZ	DATE: 4/25/24
DWG. NO.:	2 OF 13	ISSUE: C



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)											
		Load Id	Hmax	V	Load Id	Hmin	V	Bolt(in)		Base_Plate(in)			Grout (in)
2	D	2	12.7	21.4	3	-4.1	-5.4	4	0.750	6.000	10.75	0.500	0.0
2	A	4	4.1	-5.4	2	-12.7	21.4	4	0.750	6.000	10.75	0.500	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	D	1.1	2.0	0.4	0.6	4.5	7.5	11.2	18.8	-7.9	-11.0	-1.6	-7.9
2	A	-1.1	2.0	-0.4	0.6	-4.5	7.5	-11.2	18.7	1.6	-7.9	7.9	-11.0

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	D	-7.0	-6.4	-0.7	-3.3	-2.3	-10.4	-3.4	-8.9	-0.4	-0.2	0.4	0.2
2	A	0.7	-3.3	7.0	-6.4	3.4	-8.9	2.3	-10.4	-0.4	0.2	0.4	-0.2

Frame Line	Column Line	Seismic_Long		F1UNB_SL_L		F1UNB_SL_R	
		Horz	Vert	Horz	Vert	Horz	Vert
2	D	0.0	-0.2	8.7	17.4	8.7	10.1
2	A	0.0	-0.2	-8.7	10.1	-8.7	17.4

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	± Reactions(k)				Panel_Shear (lb/ft)
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	
L_EW	1	C,B	1.5	1.3	0.4	0.3	
F_SW	A	1,2	2.0	0.6	0.7	0.2	
R_EW	3	B,C	1.5	1.3	0.4	0.3	
B_SW	D	2,1	2.0	0.6	0.7	0.2	

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
32	Endwall	3/4"	A307
8	Frame	3/4"	A307

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead		Collat		Live		Snow		Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2	
		Vert	Vert	Vert	Vert	Vert	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	D	0.3	0.1	1.3	1.3	2.0	0.0	-1.4	0.0	-1.4	0.0	-1.4	0.0	-0.7	0.0	-0.6	0.0
1	C	0.7	0.2	3.7	5.6	5.6	-1.5	-5.3	0.0	-1.1	-1.5	-4.2	0.0	0.0	0.0	0.0	0.0
1	B	0.7	0.2	3.7	5.6	5.6	0.0	-1.1	1.5	-5.3	0.0	0.0	1.5	-4.2	0.0	0.0	-4.2
1	A	0.3	0.1	1.3	2.0	0.0	0.0	-1.4	0.0	-1.4	0.0	-0.6	0.0	-0.6	0.0	-0.7	0.0

Frm Line	Col Line	Wind_Press		Wind_Suct		Wind_Long1		Wind_Long2		Seis_Left		Seis_Right		Seis_Long	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	D	-3.1	-0.6	1.2	0.6	0.0	-1.8	0.0	-1.2	0.0	0.0	0.0	0.0	-0.8	-0.2
1	C	-1.5	0.0	1.7	0.0	0.0	-3.1	-0.6	-2.7	-0.4	-0.4	0.0	0.4	0.0	0.0
1	B	-1.5	0.0	1.7	0.0	0.6	-2.7	0.0	-3.1	0.0	0.4	0.4	-0.4	0.0	0.0
1	A	-3.1	-0.6	1.2	0.6	0.0	-1.2	0.0	-1.8	0.0	0.0	0.0	0.0	-0.8	-0.2

Frm Line	Col Line	E1UNB_SL_L		E1UNB_SL_R	
		Horz	Vert	Horz	Vert
1	D	0.0	1.9	0.0	0.5
1	C	0.0	6.4	0.0	2.3
1	B	0.0	2.3	0.0	6.4
1	A	0.0	0.5	0.0	1.9

Frm Line	Col Line	Dead		Collat		Live		Snow		Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2		Wind Press
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	
3	A	0.3	0.1	1.3	2.0	0.0	-1.4	0.0	-1.4	0.0	-0.7	0.0	-0.6	0.0	-0.6	0.0	-0.6	
3	B	0.7	0.2	3.7	5.6	-1.5	-5.3	0.0	-1.1	-1.5	-4.2	0.0	0.0	-1.5	-4.2	0.0	-1.5	
3	C	0.7	0.2	3.7	5.6	0.0	-1.1	1.5	-5.3	0.0	0.0	1.5	-4.2	0.0	0.0	-1.5	-4.2	
3	D	0.3	0.1	1.3	2.0	0.0	-1.4	0.0	-1.4	0.0	-0.6	0.0	-0.6	0.0	-0.7	0.0	-0.6	

Frm Line	Col Line	Wind Suct		Wind_Long1		Wind_Long2		Seis_Left		Seis_Right		Seis Long		E2UNB_SL_L	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
3	A	0.7	0.0	-1.8	0.0	-1.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.9	
3	B	1.7	0.0	-3.1	-0.6	-2.7	-0.4	-0.4	0.0	0.4	0.0	0.0	0.0	6.4	
3	C	1.7	0.6	-2.7	0.0	-3.1	0.0	0.4	0.4	-0.4	0.0	0.0	0.0	2.3	
3	D	0.7	0.0	-1.2	0.0	-1.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.5	

Frm Line	Col Line	E2UNB_SL_R	
		Horz	Vert
3	A	0.0	0.5
3	B	0.0	2.3
3	C	0.0	6.4
3	D	0.0	1.9

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)											
		Load Id	Hmax	V	Load Id	Hmin	V	Bolt(in)		Base_Plate(in)			Grout (in)
1	D	5	0.7	-0.5	6	-1.8	-1.3	4	0.750	6.000	8.000	0.375	0.0
		2	0.0	2.4	7	1.0	-2.8						
1	C	7	1.0	-2.8	6	-0.9	-1.4	4	0.750	6.000	8.000	0.375	0.0
		8	0.0	7.3	7	1.0	-2.8						
1	B	9	1.0	-2.8	10	-0.9	-1.4	4	0.750	6.000	8.000	0.375	0.0
		11	0.0	7.3	9	1.0	-2.8						
1	A	12	0.7	-0.5	10	-1.8	-1.3	4	0.750	6.000	8.000	0.375	0.0
		2	0.0	2.4	10	-1.8	-1.3						
3	A	5	0.4	-0.9	6	-0.3	-0.9	4	0.750	6.000	8.000	0.375	0.0
		2	0.0	2.4	5	0.4	-0.9						
3	B	7	1.0	-2.8	6	-0.9	-1.4	4	0.750	6.000	8.000	0.375	0.0
		13	0.0	7.3	7	1.0	-2.8						
3	C	9	1.0	-2.8	10	-0.9	-1.4	4	0.750	6.000	8.000	0.375	0.0
		14	0.0	7.3	9	1.0	-2.8						
3	D	12	0.4	-0.9	10	-0.3	-0.9	4	0.750	6.000	8.000	0.375	0.0
		2	0.0	2.4	12	0.4	-0.9						

NOTES FOR REACTIONS

Building reactions are based on the following building data:

- Width (ft) = 40.0
- Length (ft) = 50.0
- Eave Height (ft) = 10.0/10.0
- Roof Slope (rise/12) = 3.00/3.00
- Roof Dead Load (psf) = 2.4
- Wall Dead Load = 2.0
- Left Endwall (psf) = 2.0
- Right Endwall (psf) = 2.0
- Front Sidewall (psf) = 2.0
- Back Sidewall (psf) = 2.0
- Roof Live Load (psf) = 20.0
- Frame Live Load (psf) = 12.0
- Collateral Load (psf) = 1.0
- Snow Load (psf) = 30.0
- Wind Speed (mph) = 115.0
- Wind Code = IBC-21
- Exposure = C
- Closure = Enclosed
- Internal Wind Coeff = -0.18, +0.18
- Risk Category = II - Normal
- Importance - Wind = 1.00
- Importance - Seismic = 1.00
- Seismic Design Category = C
- Seismic Coeff (Sms) = 0.53

ID	Description
1	Dead+Collateral+Snow
2	Dead+Collateral+Snow+Slide_Snow
3	0.6Dead+0.6Wind_Left1
4	0.6Dead+0.6Wind_Right1
5	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
6	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
7	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
8	Dead+Collateral+E1UNB_SL_L
9	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
10	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
11	Dead+Collateral+E1UNB_SL_R
12	0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
13	Dead+Collateral+E2UNB_SL_L
14	Dead+Collateral+E2UNB_SL_R

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

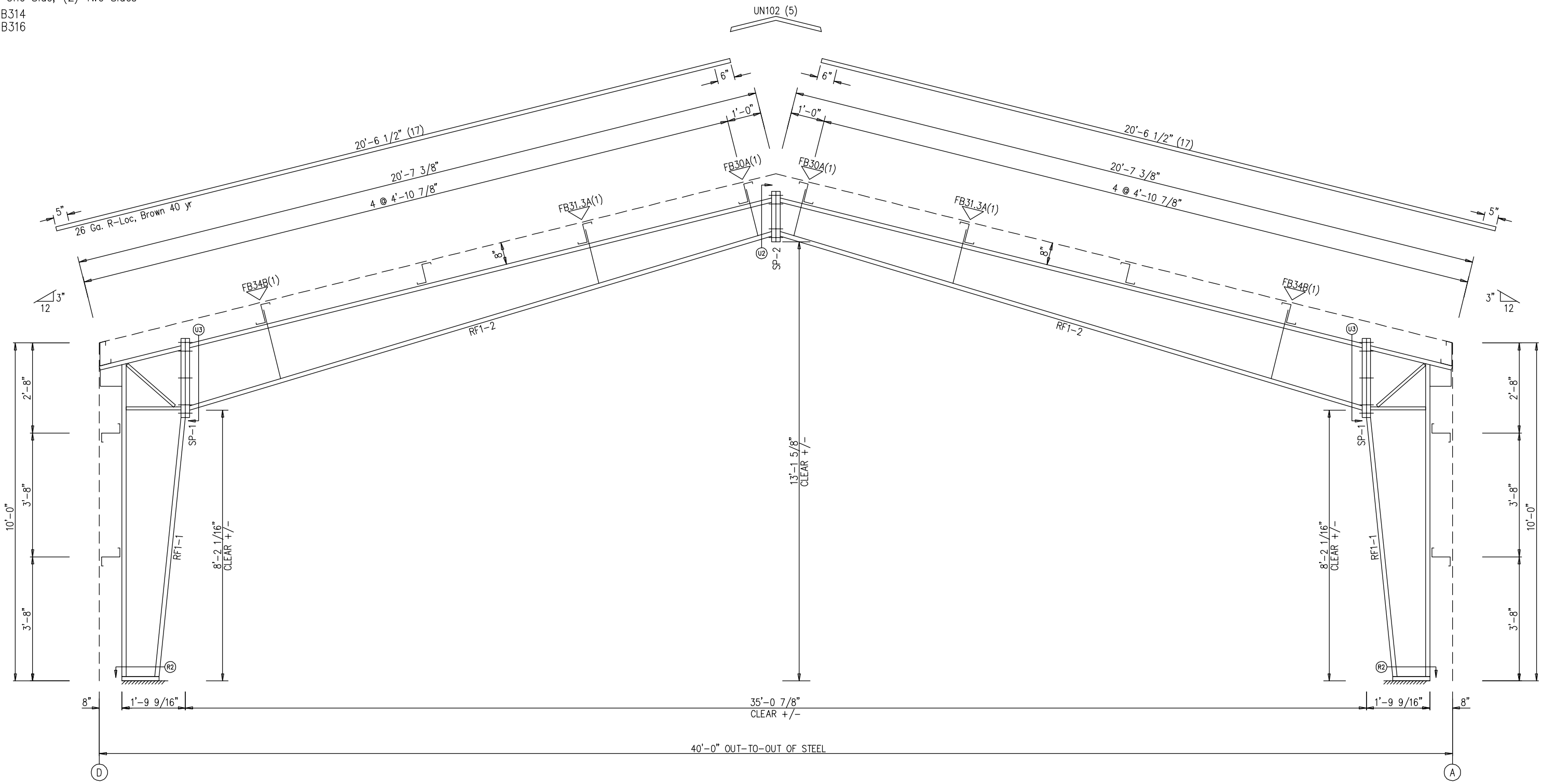
2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ANCHOR BOLT REACTIONS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58143	ENG. BY: MZ DATE: 4/25/24
	DWG. NO.: 3 OF 13 ISSUE: C

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	2	A325	0.625	2.25
SP-2	4	4	0	A325	0.625	1.75

MEMBER TABLE								
Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length	W x Thk x Length	
RF1-1	10.0	21.0	0.135	9'-10 3/8"	5 x 1/4" x 9'-4 7/8"	5 x 3/8" x 2'-6 1/16"	5 x 5/16" x 7'-10 3/4"	
RF1-2	20.0	10.0	0.135	18'-4 9/16"	5 x 1/4" x 17'-11 1/2"	5 x 1/4" x 17'-11 1/2"	5 x 5/16" x 8'-11 11/16"	

▽ FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 B - B314
 A - B316



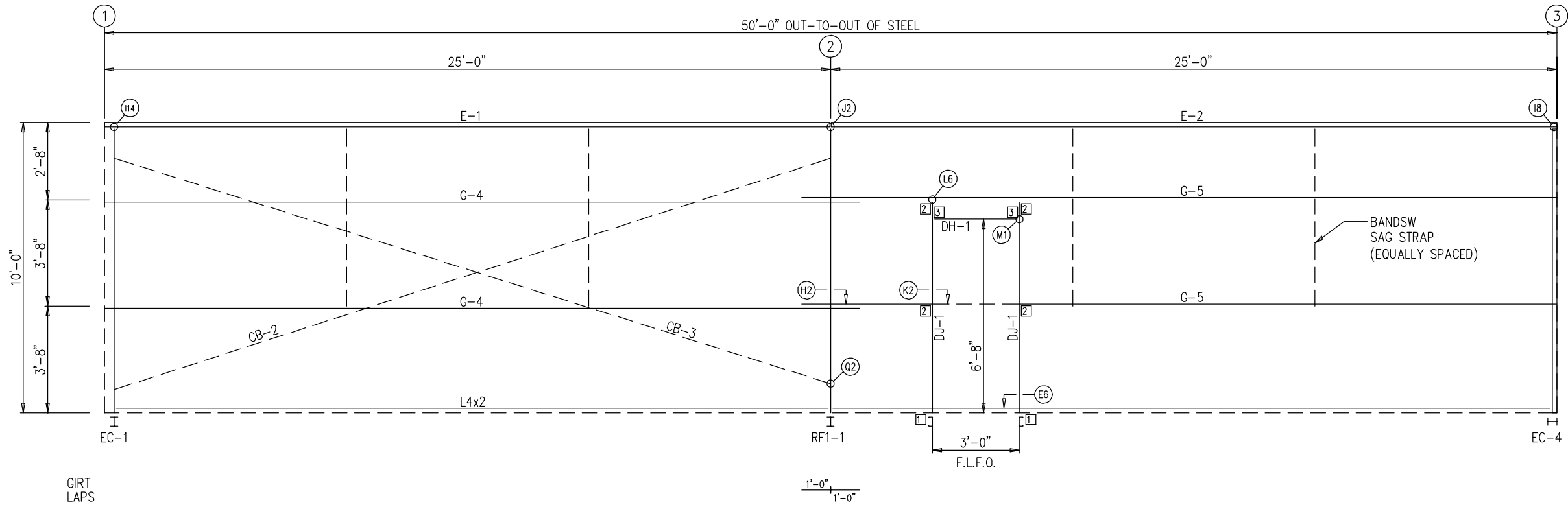
RIGID FRAME ELEVATION: FRAME LINE 2

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

ARMSTRONG STEEL
 2 Inverness Drive East, Ste#200
 Englewood, Colorado 80112
 PHONE: 800-345-4610
 www.armstrongsteel.com

DESCRIPTION	RIGID FRAME ELEVATION		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	58143	ENG. BY:	MZ
DATE:	4/25/24	ISSUE:	C
DWG. NO.:	4 OF 13		



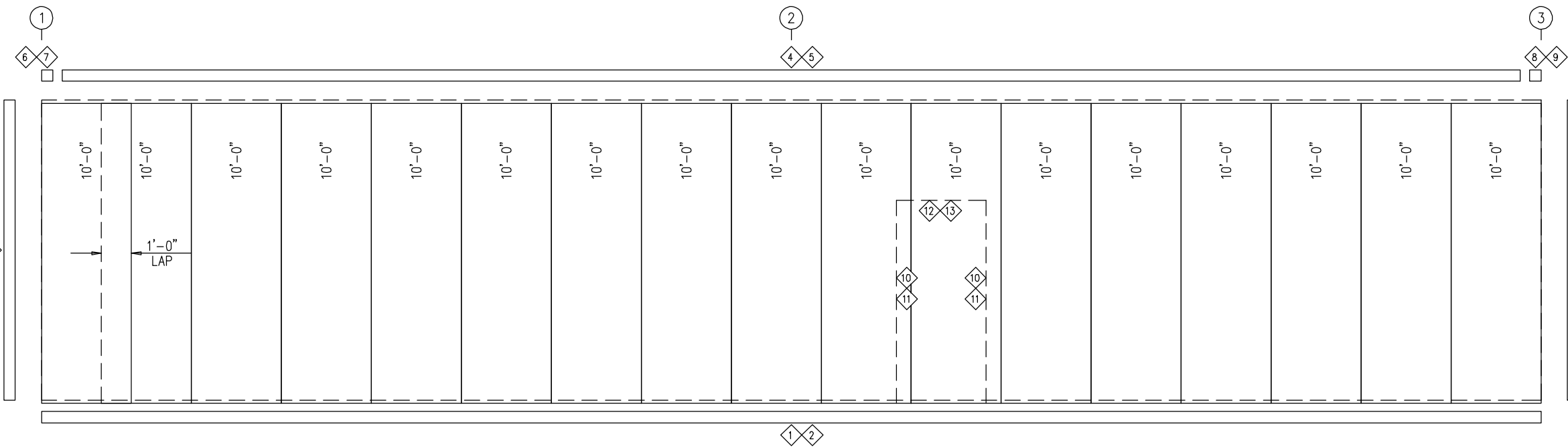
FRONT SIDEWALL FRAMING: FRAME LINE A
F.L.F.O. =Field Located Framed Opening

TRIM TABLE FRAME LINE A				
ID	QUAN	PART	LENGTH	DETAIL
1	1	BA6102	10'-2"	TRIM_1
2	2	BA6	20'-4"	TRIM_1
3	2	OU6102	10'-2"	TRIM_30
4	1	Q7726	16'-2"	TRIM_61
5	2	Q7726	18'-2"	TRIM_61
6	1	Q773L6	6"	
7	1	AR961L6	7 7/16"	TRIM_60
8	1	Q773R6	6"	
9	1	AR961R6	7 7/16"	TRIM_60
10	2	Q3706	7'-4"	TRIM_50
11	2	JA6	7'-2"	TRIM_50
12	1	Q3706	3'-7"	TRIM_51
13	1	HE6	3'-6"	TRIM_51

MEMBER TABLE FRAME LINE A				
QUAN	MARK	PART	LENGTH	
2	DJ-1	8X25C16	7'-0"	
1	DH-1	8X25C16	2'-11"	
1	E-1	08536DU3	24'-11"	
1	E-2	08536DU3	24'-11"	
2	G-4	8X25Z16	25'-11 1/2"	
2	G-5	8X25Z16	25'-11 1/2"	
1	CB-2	GS1716	26'-5 1/4"	
1	CB-3	GS1716	26'-4 3/4"	

CONNECTION PLATES FRAME LINE A			
ID	QUAN	MARK/PART	
1	2	BC-05	
2	4	BC-01	
3	2	BC-500	

NOTE:
BUILDING IS DESIGNED AS SHOWN,
ANY WINDOWS OR OTHER PENETRATIONS MUST
BE PLACE BETWEEN GIRTS. CUTTING GIRTS IS
NOT CONSIDERED PART OF THE FIELD WORK.



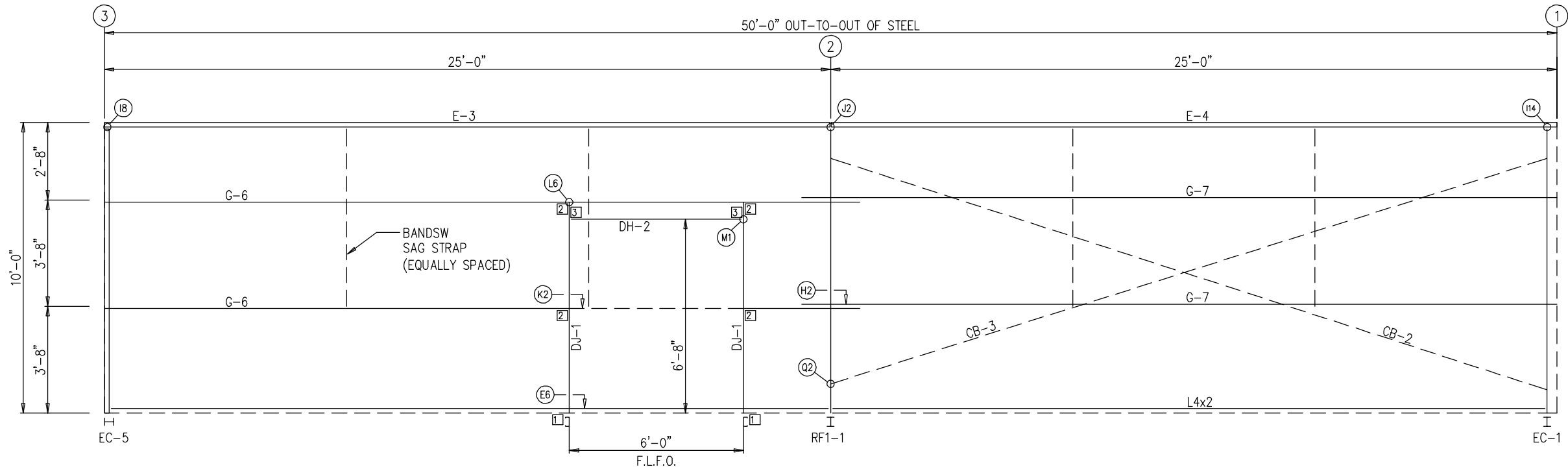
FRONT SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. R-Loc - Lt. Stone 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY
AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
A NORMAL PART OF METAL BUILDING ERECTION.
A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	SIDEWALL FRAMING & SHEETING		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	58143	ENG. BY:	MZ
		DWG. NO.:	5 OF 13
		DATE:	4/25/24
		ISSUE:	C



TRIM TABLE FRAME LINE D				
ID	QUAN	PART	LENGTH	DETAIL
1	1	BA6102	10'-2"	TRIM_1
2	2	BA6	20'-4"	TRIM_1
3	2	OU6102	10'-2"	TRIM_30
4	1	Q7726	16'-2"	TRIM_61
5	2	Q7726	18'-2"	TRIM_61
6	1	Q773L6	6"	
7	1	AR961L6	7 7/16"	TRIM_60
8	1	Q773R6	6"	
9	1	AR961R6	7 7/16"	TRIM_60
10	2	Q3706	7'-4"	TRIM_50
11	2	JA6	7'-2"	TRIM_50
12	1	Q3706	7'-4"	TRIM_51
13	1	HE6	6'-3"	TRIM_51

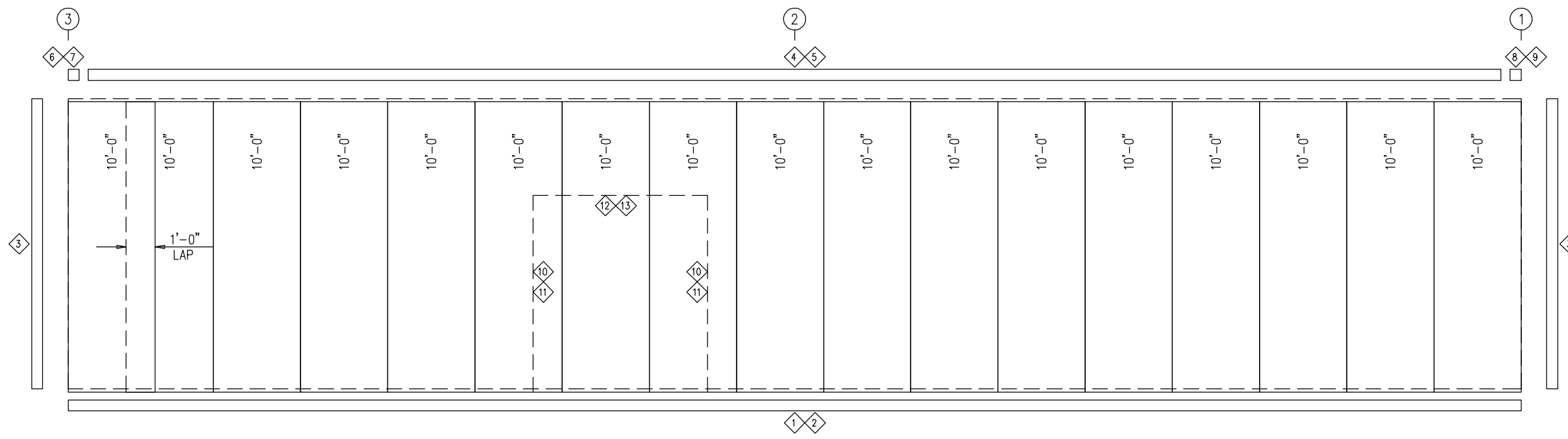
MEMBER TABLE FRAME LINE D				
QUAN	MARK	PART	LENGTH	
2	DJ-1	8X25C16	7'-0"	
1	DH-2	8X25C16	5'-11"	
1	E-3	08536DU3	24'-11"	
1	E-4	08536DU3	24'-11"	
2	G-6	8X25Z16	25'-11 1/2"	
2	G-7	8X25Z16	25'-11 1/2"	
1	CB-2	GS1716	26'-5 1/4"	
1	CB-3	GS1716	26'-4 3/4"	

CONNECTION PLATES FRAME LINE D			
ID	QUAN	MARK/PART	
1	2	BC-05	
2	4	BC-01	
3	2	BC-500	

GIRT LAPS

BACK SIDEWALL FRAMING: FRAME LINE D
F.L.F.O. = Field Located Framed Opening

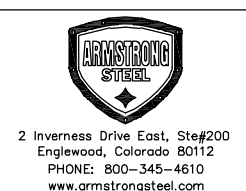
NOTE:
BUILDING IS DESIGNED AS SHOWN,
ANY WINDOWS OR OTHER PENETRATIONS MUST
BE PLACE BETWEEN GIRTS. CUTTING GIRTS IS
NOT CONSIDERED PART OF THE FIELD WORK.



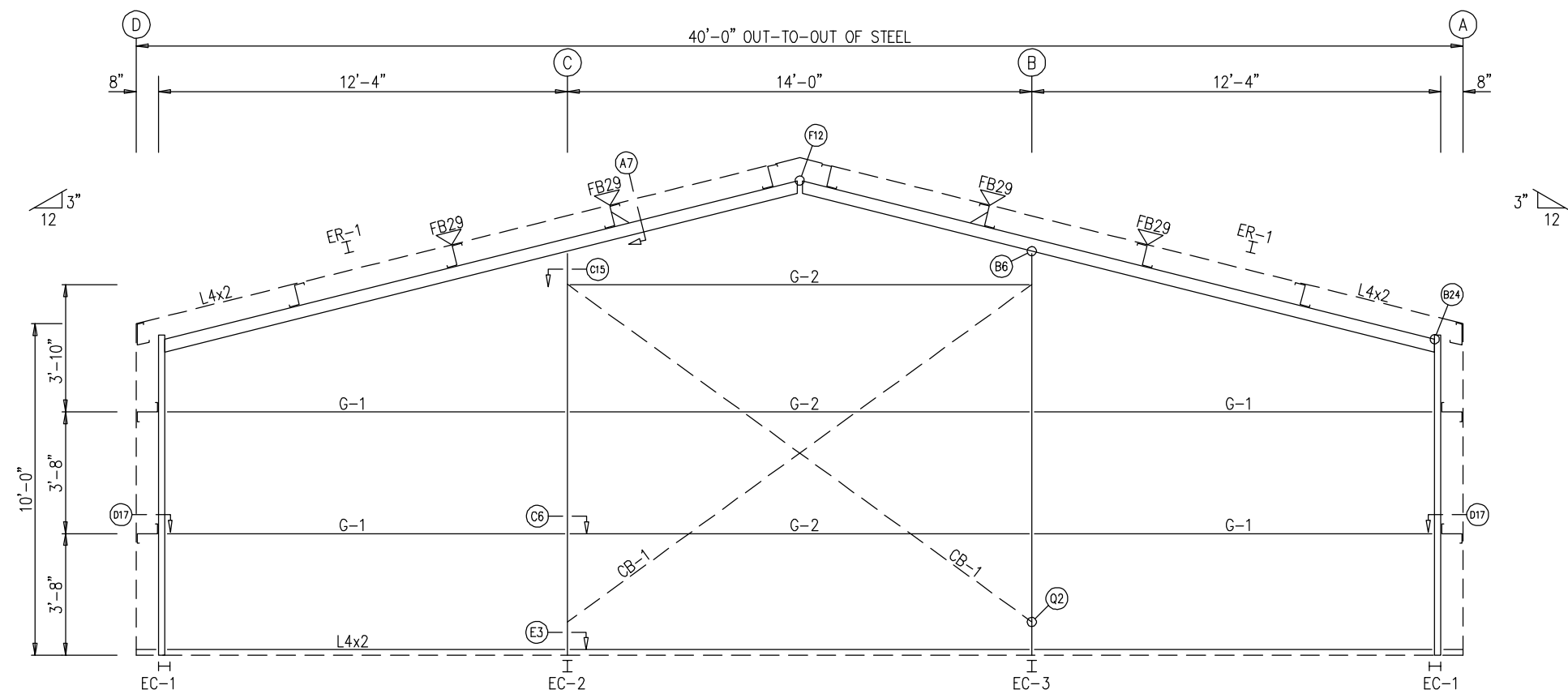
BACK SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 Ga. R-Loc - Lt. Stone 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY
AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
A NORMAL PART OF METAL BUILDING ERECTION.
A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ



DESCRIPTION	SIDEWALL FRAMING & SHEETING		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	58143	ENG. BY:	MZ
DWG. NO.:	6 OF 13	DATE:	4/25/24
ISSUE:	C		



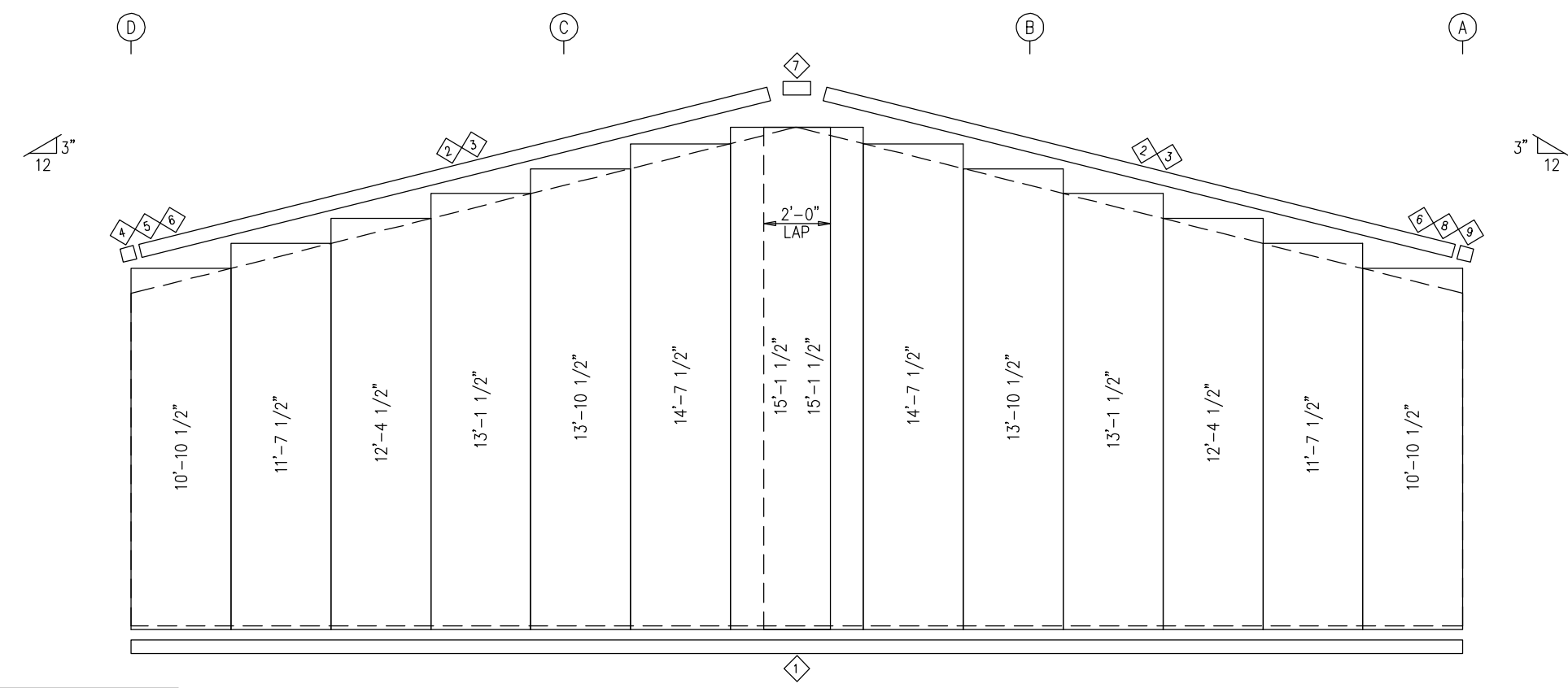
LEFT ENDWALL FRAMING: FRAME LINE 1

TRIM TABLE FRAME LINE 1				
ID	QUAN	PART	LENGTH	DETAIL
1	2	BA6	20'-4"	TRIM_1
2	2	Q7646102	10'-2"	TRIM_66
3	2	Q7646	12'-2"	TRIM_66
4	1	Q765L6	6"	
5	1	AR963L6	9 1/8"	
6	2	AR9626	8 1/16"	
7	1	Q7676	1'-4"	TRIM_100
8	1	Q765R6	6"	
9	1	AR963R6	9 1/8"	

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Cor_Column/Raf	4	A325	5/8"	1 1/2"
ER-1/ER-1	8	A325	5/8"	1 1/2"
Int_Column/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE FRAME LINE 1				
QUAN	MARK	PART	LENGTH	
2	EC-1	W8X10	9'-7 3/4"	
1	EC-2	W8X10	11'-8 5/8"	
1	EC-3	W8X10	11'-8 5/8"	
2	ER-1	W8X10	19'-5"	
4	G-1	8X25Z16	11'-3 5/8"	
3	G-2	8X25Z16	13'-4"	
2	CB-1	GS1716	17'-5 1/2"	

FLANGE BRACE TABLE FRAME LINE 1			
ID	QUAN	MARK	LENGTH
1	4	FB29	2'-5"



LEFT ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. R-Loc - Lt. Stone 40 yr

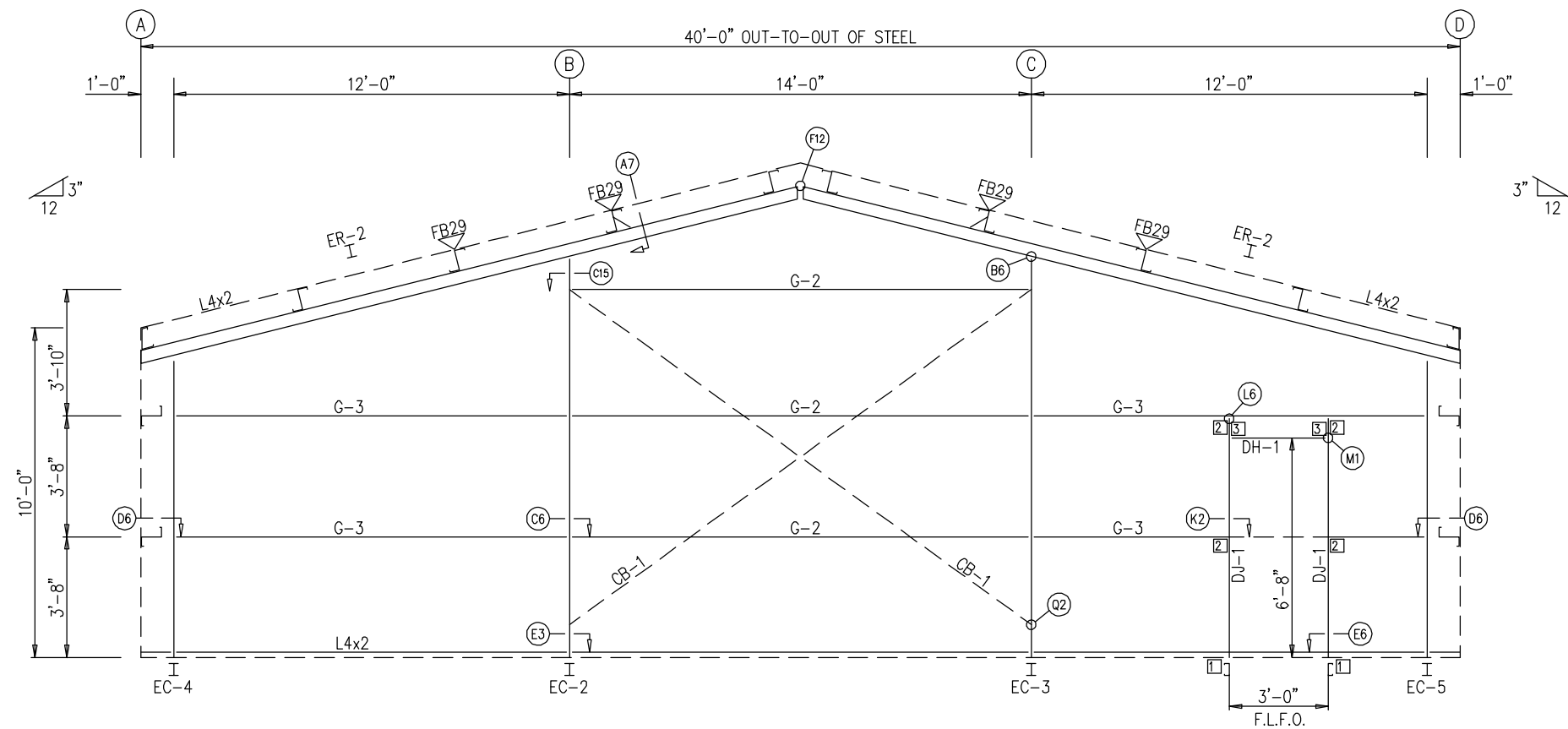
NOTE:
BUILDING IS DESIGNED AS SHOWN,
ANY WINDOWS OR OTHER PENETRATIONS MUST
BE PLACE BETWEEN GIRTS. CUTTING GIRTS IS
NOT CONSIDERED PART OF THE FIELD WORK.

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY
AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
A NORMAL PART OF METAL BUILDING ERECTION.
A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
FIELD WORK.

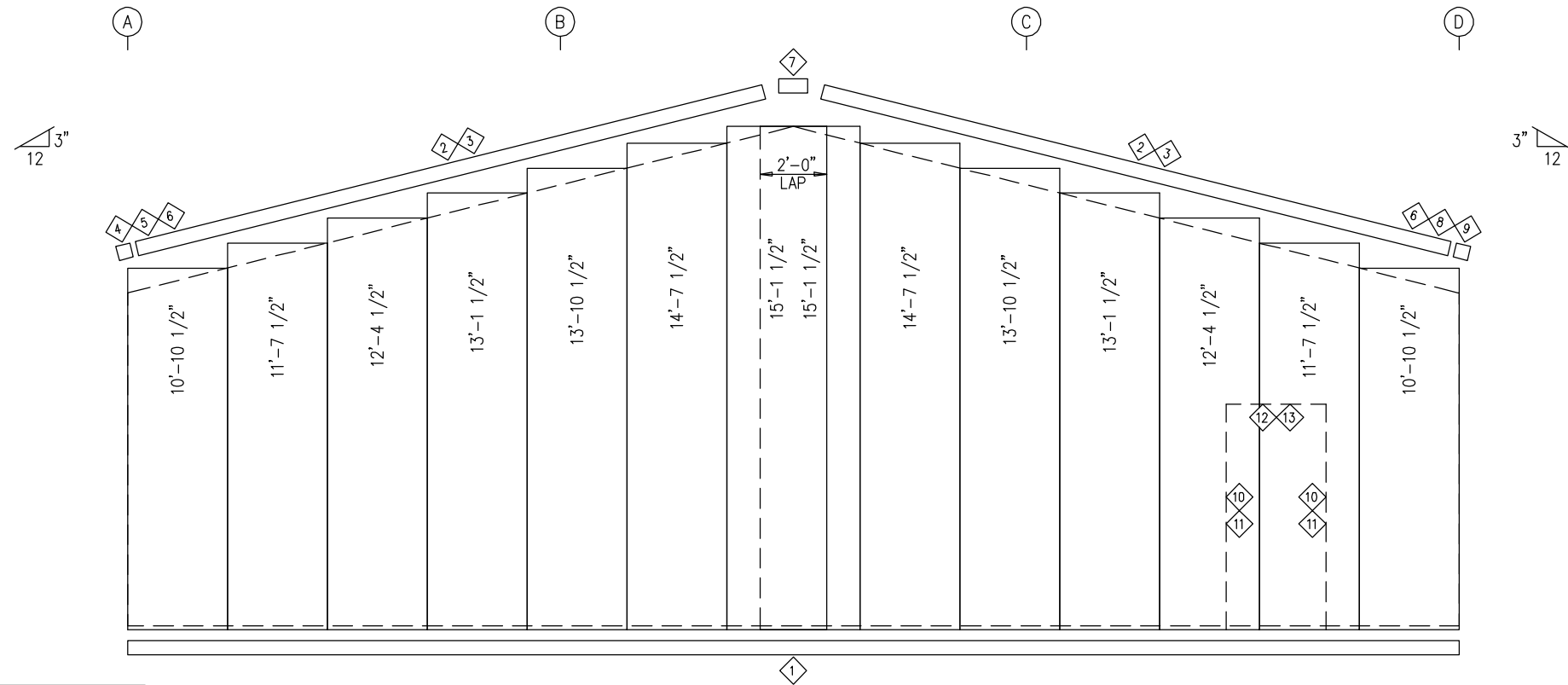
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ENDWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.:	58143
ENG. BY:	MZ
DATE:	4/25/24
DWG. NO.:	7 OF 13
ISSUE:	C



RIGHT ENDWALL FRAMING: FRAME LINE 3
F.L.F.O. =Field Located Framed Opening



RIGHT ENDWALL SHEETING & TRIM: FRAME LINE 3
PANELS: 26 Ga. R-Loc - Lt. Stone 40 yr

TRIM TABLE FRAME LINE 3				
ID	QUAN	PART	LENGTH	DETAIL
1	2	BA6	20'-4"	TRIM_1
2	2	Q7646102	10'-2"	TRIM_66
3	2	Q7646	12'-2"	TRIM_66
4	1	Q765L6	6"	
5	1	AR963L6	9 1/8"	
6	2	AR9626	8 1/16"	
7	1	Q7676	1'-4"	TRIM_100
8	1	Q765R6	6"	
9	1	AR963R6	9 1/8"	
10	2	Q3706	7'-4"	TRIM_50
11	2	JA6	7'-2"	TRIM_50
12	1	Q3706	3'-7"	TRIM_51
13	1	HE6	3'-6"	TRIM_51

BOLT TABLE FRAME LINE 3				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-2/ER-2	8	A325	5/8"	1 1/2"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE FRAME LINE 3			
QUAN	MARK	PART	LENGTH
1	EC-2	W8X10	11'-8 5/8"
1	EC-3	W8X10	11'-8 5/8"
1	EC-4	W8X10	8'-8 5/8"
1	EC-5	W8X10	8'-8 5/8"
2	ER-2	W8X10	20'-6 7/8"
2	DJ-1	8X25C16	7'-0"
1	DH-1	8X25C16	2'-11"
3	G-2	8X25Z16	13'-4"
4	G-3	8X25Z16	11'-4"
2	CB-1	GS1716	17'-5 1/2"

FLANGE BRACE TABLE FRAME LINE 3			
ID	QUAN	MARK	LENGTH
1	4	FB29	2'-5"

CONNECTION PLATES FRAME LINE 3		
ID	QUAN	MARK/PART
1	2	BC-05
2	4	BC-01
3	2	BC-500

NOTE:
BUILDING IS DESIGNED AS SHOWN,
ANY WINDOWS OR OTHER PENETRATIONS MUST
BE PLACE BETWEEN GIRTS. CUTTING GIRTS IS
NOT CONSIDERED PART OF THE FIELD WORK.

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY
AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
A NORMAL PART OF METAL BUILDING ERECTION.
A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
FIELD WORK.

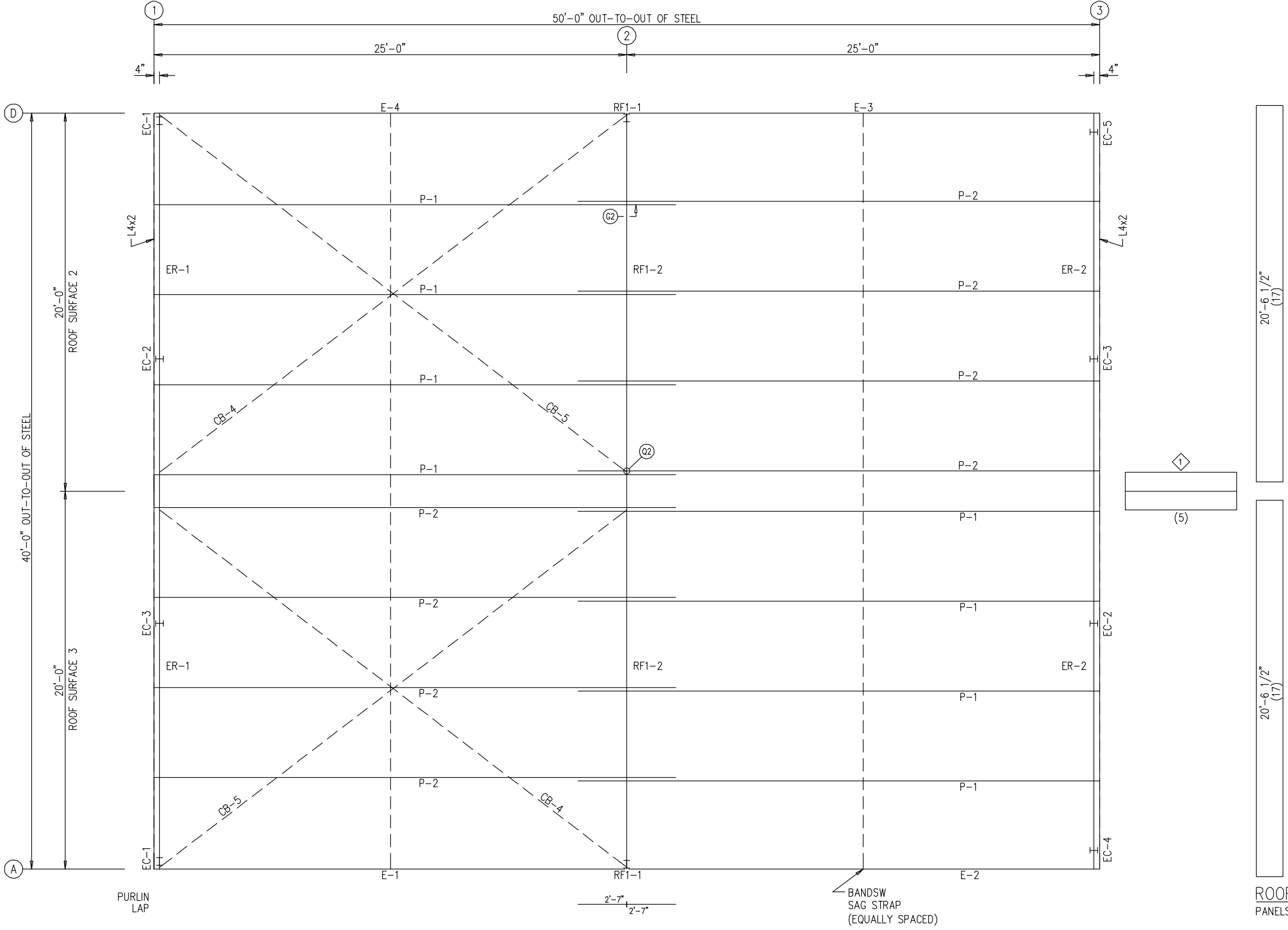
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ENDWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.:	58143
ENG. BY:	MZ
DATE:	4/25/24
DWG. NO.:	8 OF 13
ISSUE:	C

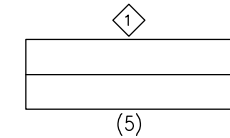
TRIM TABLE				
ROOF PLAN				
ID	QUAN	PART	LENGTH	DETAIL
1	5	UN6102	10'-2"	TRIM_101

MEMBER TABLE				
ROOF PLAN				
QUAN	MARK	PART	LENGTH	
8	P-1	8x35Z12	27'-6 1/2"	
8	P-2	8x35Z12	27'-6 1/2"	
1	E-1	08536DU3	24'-11"	
1	E-2	08536DU3	24'-11"	
1	E-3	08536DU3	24'-11"	
1	E-4	08536DU3	24'-11"	
2	CB-4	GS1716	30'-11 3/4"	
2	CB-5	GS1716	30'-3 3/4"	



20'-6 1/2"
(17)

20'-6 1/2"
(17)



ROOF SHEETING
PANELS: 26 Ga. R-Loc Brown 40 yr

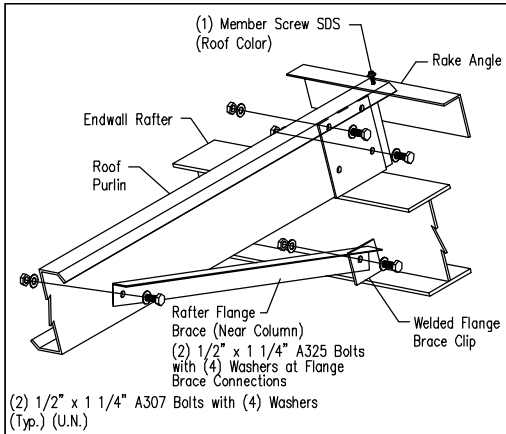
ROOF FRAMING PLAN

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

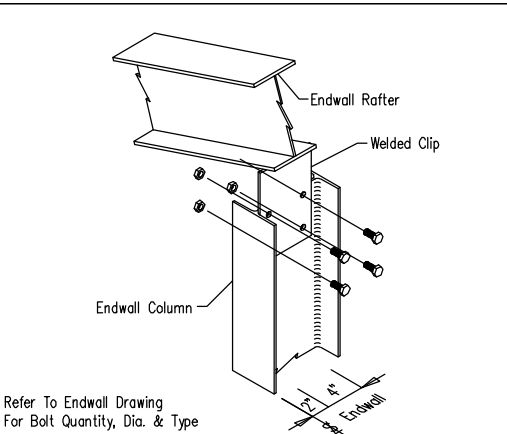
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

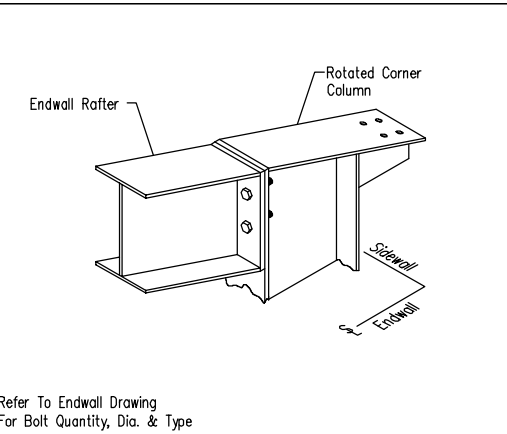
DESCRIPTION	ROOF FRAMING & SHEETING		
CUSTOMER			
END USER			
SCALE	NOT TO SCALE		
JOB NO.:	58143	ENG. BY:	MZ
DATE:	4/25/24	DWG. NO.:	9 OF 13
ISSUE:	C		



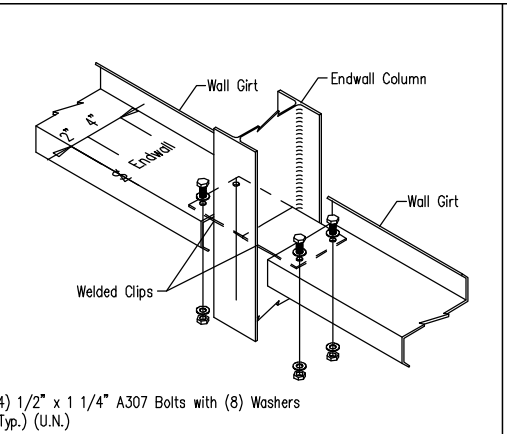
A7 ROOF PURLIN TO HOT-ROLLED RAFTER



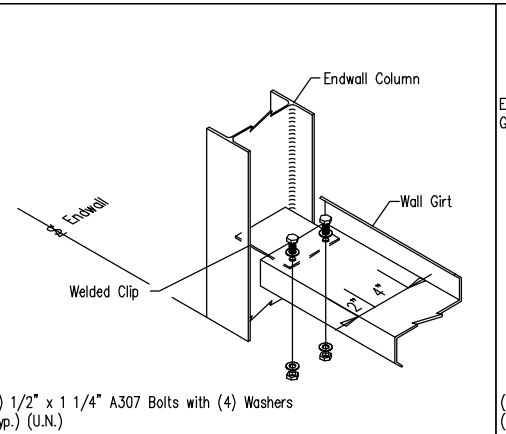
B6 BUILT-UP ENDWALL COLUMN TO ENDWALL RAFTER



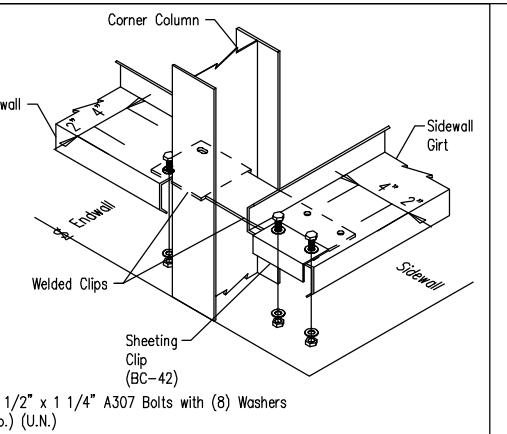
B24 ROTATED CORNER COLUMN TO ENDWALL RAFTER



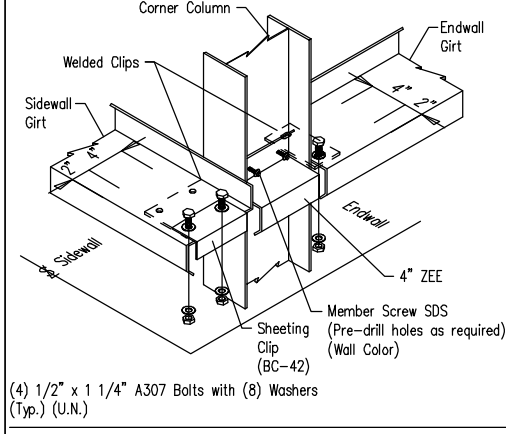
C6 WALL GIRTS TO ENDWALL COLUMN



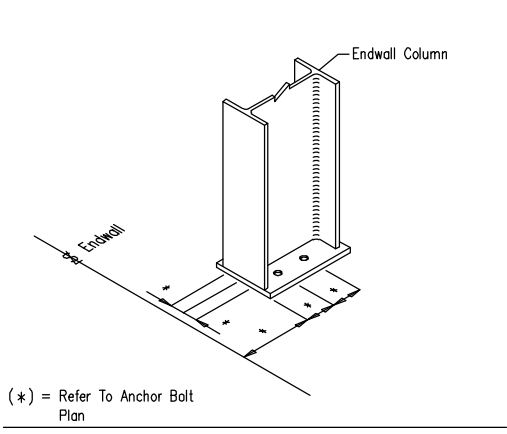
C15 WALL GIRTS TO ENDWALL COLUMN



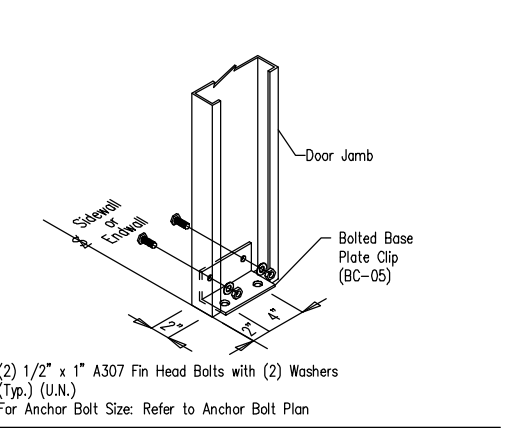
D6 WALL GIRTS TO ENDWALL CORNER COLUMN



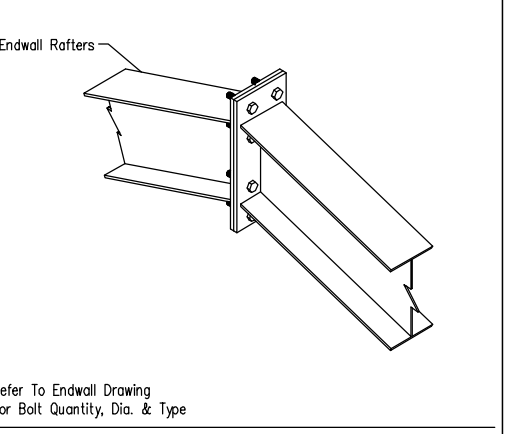
D17 WALL GIRTS TO ENDWALL CORNER COLUMN



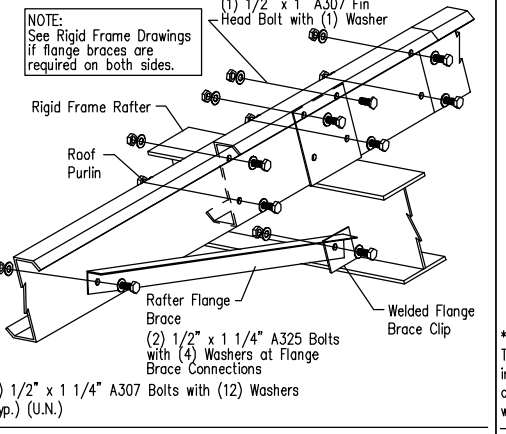
E3 ANCHOR BOLTS AT ENDWALL COLUMNS



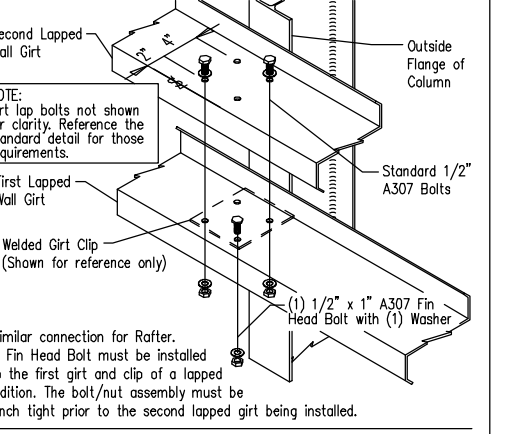
E6 BASE PLATE FOR DOOR JAMB



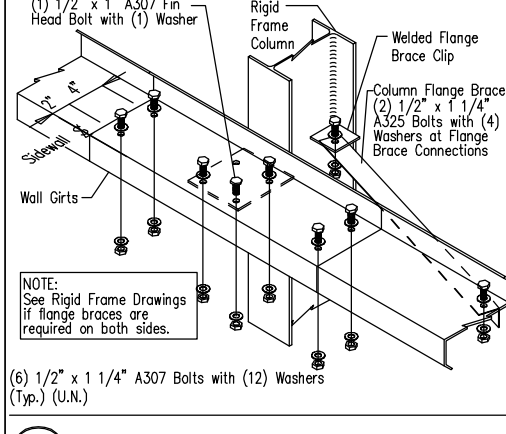
F12 RAFTER SPLICE ALONG SURFACE



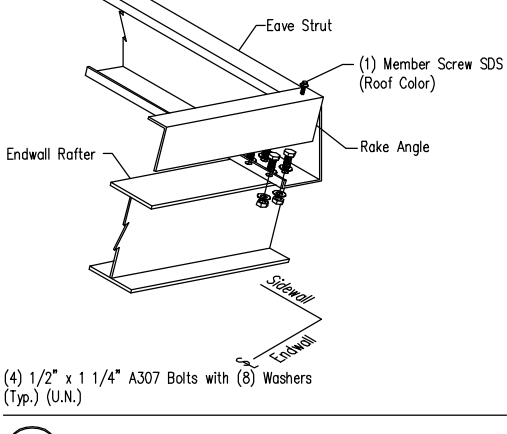
G2 ROOF PURLIN TO INTERIOR RIGID FRAME



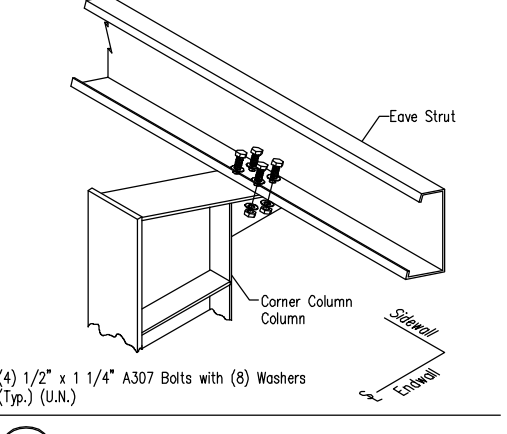
H0 BYPASS LAPPED WALL GIRTS



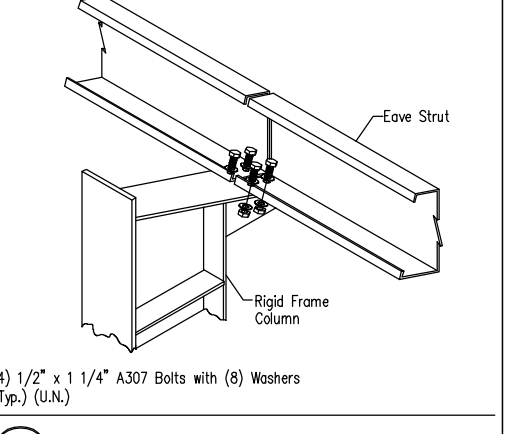
H2 WALL GIRTS TO RIGID FRAME COLUMN



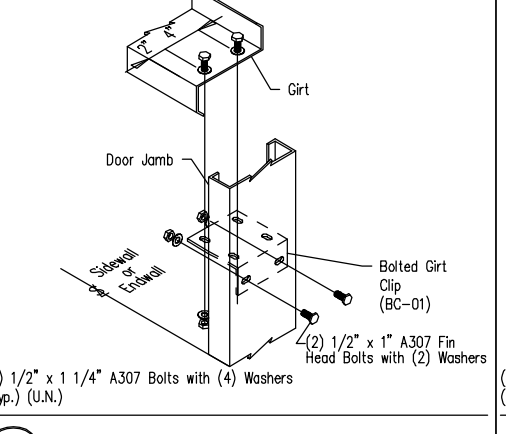
I8 EAVE STRUT TO ENDWALL RAFTER



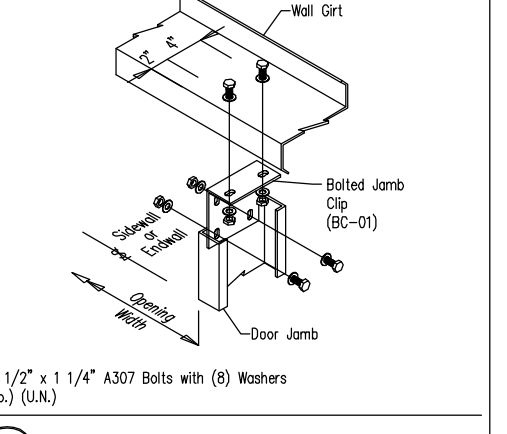
I14 LOWSIDE EAVE STRUT TO BYPASS ENDWALL COLUMN



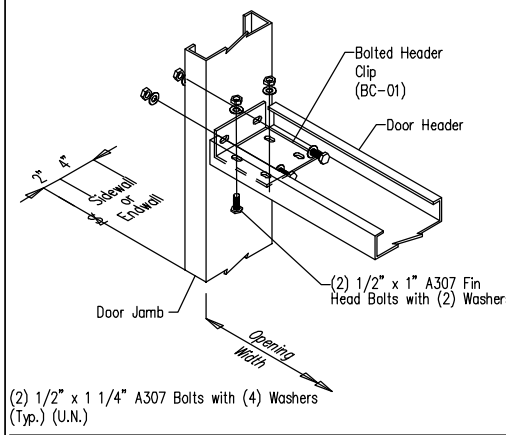
J2 LOWSIDE EAVE STRUT TO BYPASS RIGID FRAME



K2 WALL GIRTS TO DOOR JAMB



L6 DOOR JAMB TO WALL GIRTS

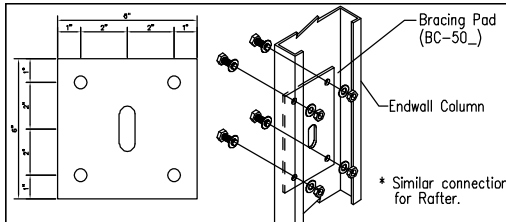


M1 HEADER TO JAMB

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

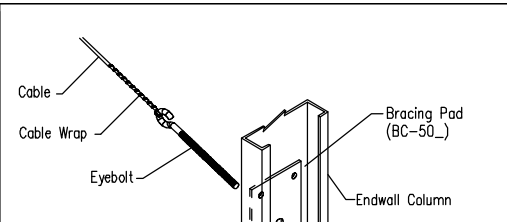
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58143	ENG. BY: MZ DATE: 4/25/24
	DWG. NO.: 10 OF 13 ISSUE: C



DIAGONAL BRACE PAD INSTALLATION INSTRUCTIONS

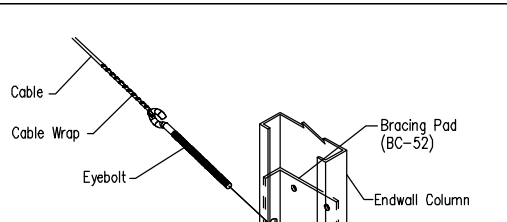
- STEP 1** Line up brace pad with pre existing hole punches in the member.
- STEP 2** Bolt the brace pad down using (4) 1/2" A307 bolts.
- STEP 3** Field cut out the slot, using the brace pad slot as a template.
- STEP 4** Install cable brace as normal, still leaving the brace pad installed along with the 1/2" A307 bolts.

Q2 DIAGONAL BRACE PAD TO WEB OF CEE COLUMN



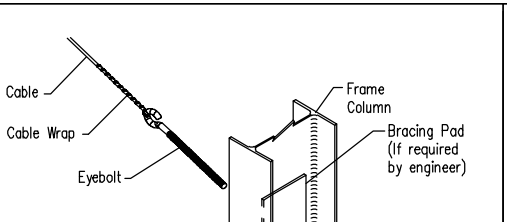
* Similar connection for Rafter. Insert Eyebolt through slot in web. Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO WEB OF CEE COLUMN



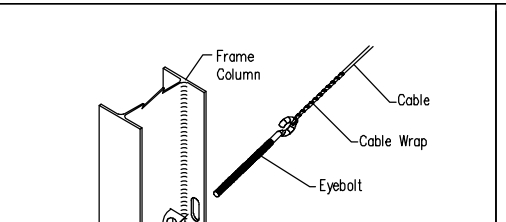
* Similar connection for Rafter. Insert Eyebolt through slot in flange. Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO FLANGE OF CEE COLUMN



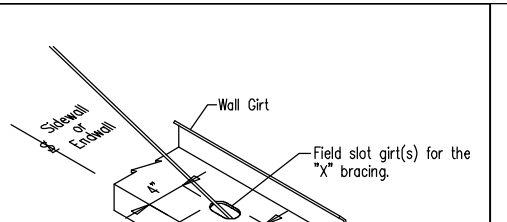
* Similar connection for Rafter. Insert Eyebolt through slot in web. Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO WEB OF FRAME COLUMN

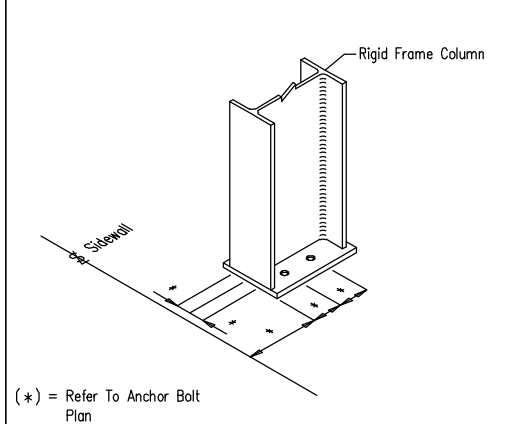


Insert Eyebolt through slot in flange. Then assemble Hillside Washer, Flat Washer, and Nut.

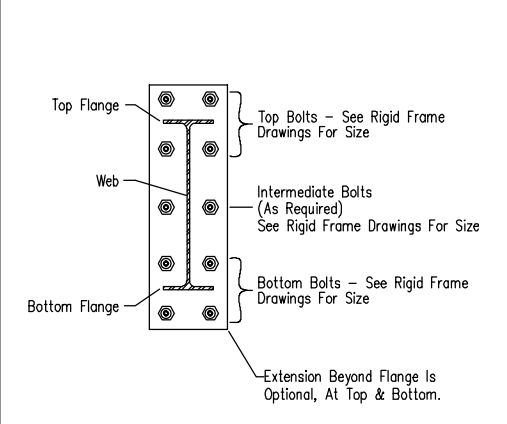
DIAGONAL CABLE BRACE TO FLANGE OF FRAME COLUMN



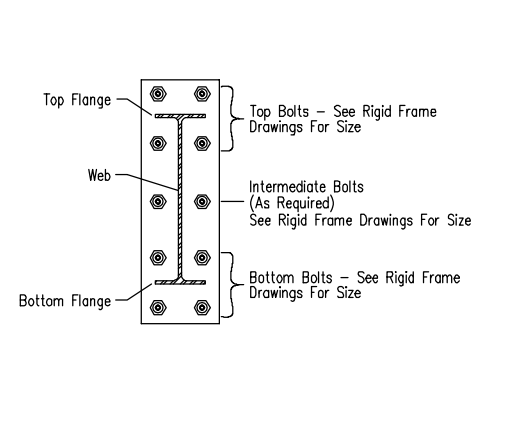
DIAGONAL CABLE BRACE AT FLUSH WALL GIRTS



R2 ANCHOR BOLTS AT SIDEWALL COLUMNS



U2 BOLTS FOR RIGID FRAME RAFTER AT BUILDING PEAK

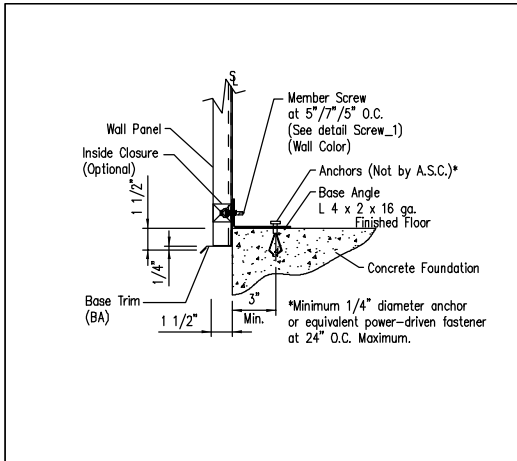


U3 BOLTS FOR RIGID FRAME RAFTER TO COLUMN CONNECTION

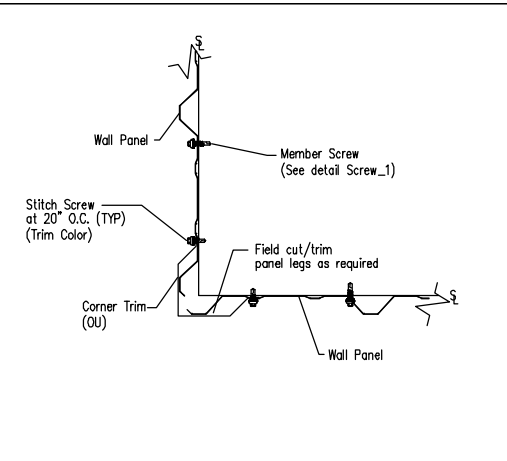
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

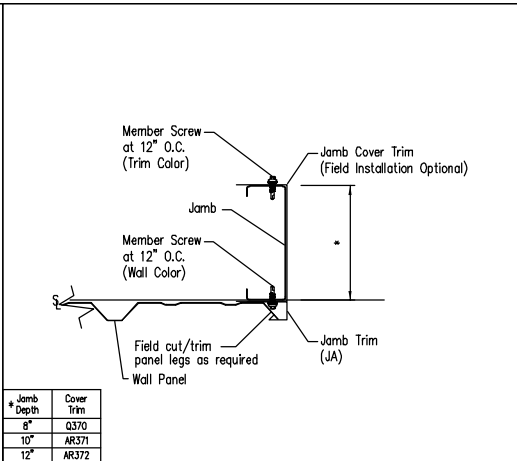
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58143	
ENG. BY: MZ	DATE: 4/25/24
DWG. NO.: 11 OF 13	ISSUE: C



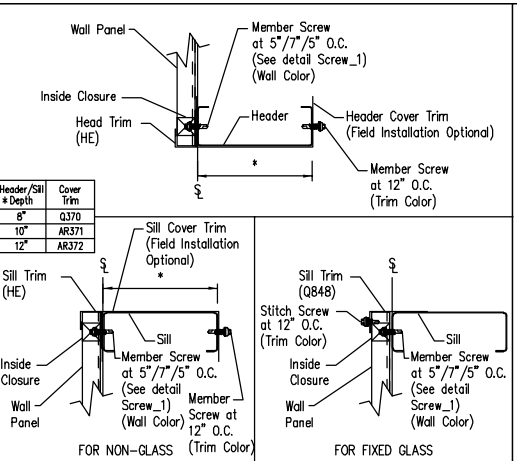
DRAWING NO. TRIM_1
 BASE ANGLE DETAIL WITH TRIM



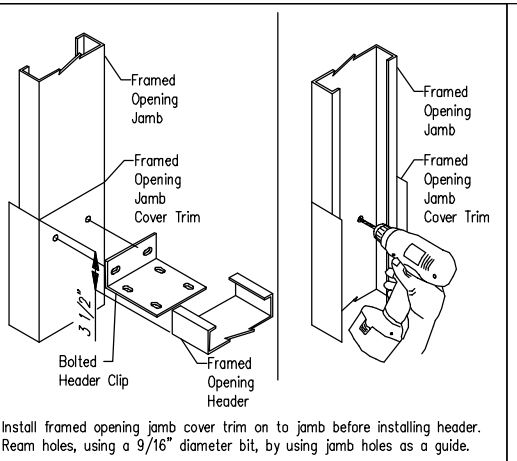
DRAWING NO. TRIM_30
 OUTSIDE CORNER DETAIL



DRAWING NO. TRIM_50
 FRAMED OPENING JAMB TRIM DETAIL



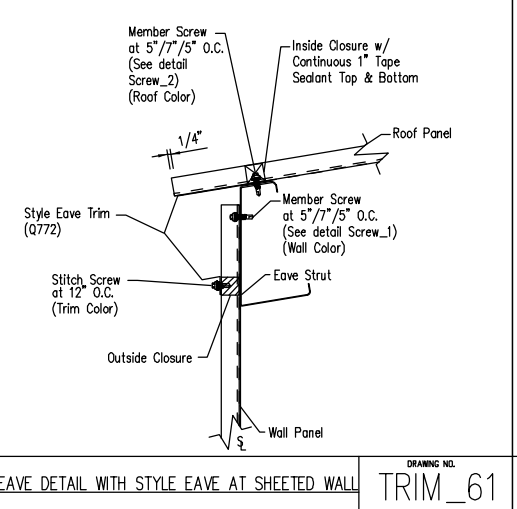
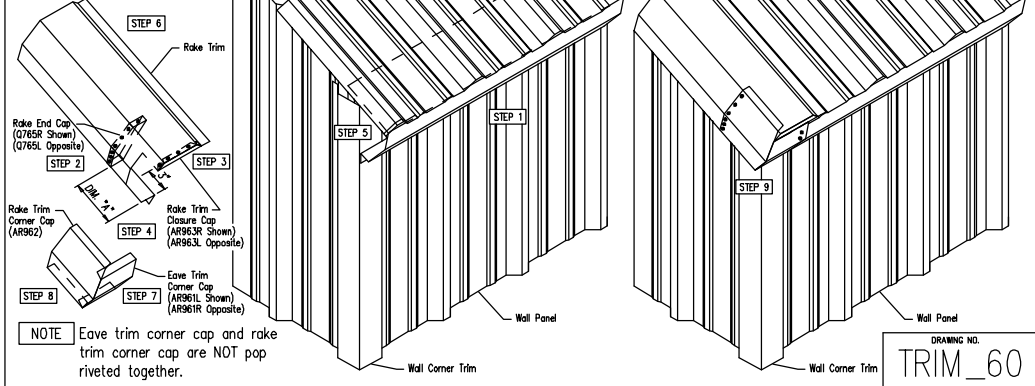
DRAWING NO. TRIM_51
 FRAMED OPENING HEAD & SILL TRIM DETAILS



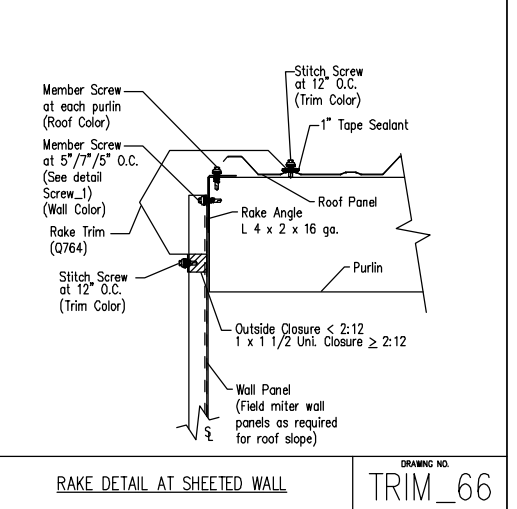
DRAWING NO. TRIM_52
 COVER TRIM INSTALLATION INSTRUCTIONS

- STYLE EAVE CORNER TRIM INSTALLATION INSTRUCTIONS**
- STEP 1** Install style eave trim in between the roof panel and low eave member. Be sure the end of the style eave trim is flush with the wall corner trim.
 - STEP 2** Install rake end cap, into rake trim using (8) pop rivets. Use chart to determine how far the rake end cap is positioned into the rake trim.
 - STEP 3** Install rake trim closure cap, flush with the end of the rake trim using (5) pop rivets.
 - STEP 4** Field cut/notch the face of the rake trim by 3". This is to prevent the rake trim from sticking out past the style eave trim upon final assembly.
 - STEP 5** Field cut/notch the end of the roof panel back 1". This is to allow the rake trim closure cap from hitting the roof panel.
 - STEP 6** Install rake trim. Be sure the end of the rake trim is flush with style eave trim.
 - STEP 7** Install the eave trim corner cap to the style eave trim using (6) pop rivets.
 - STEP 8** Install the rake trim corner cap to the rake trim using (7) pop rivets.
 - STEP 9** Field cut/notch the bottom legs of the rake trim. Horizontal leg flush with the eave trim corner cap. Vertical leg flush with the wall corner trim.

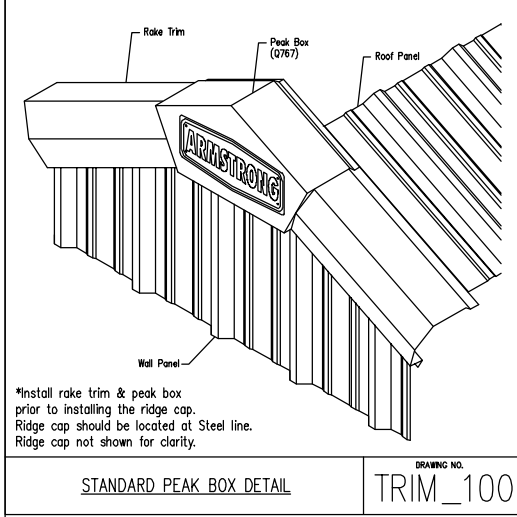
Slope	Dim. "A"
1/2 : 12	3 7/16"
1 : 12	3 11/16"
2 : 12	4 1/4"
3 : 12	4 3/4"
4 : 12	5 1/4"
5 : 12	5 13/16"
6 : 12	6 3/8"



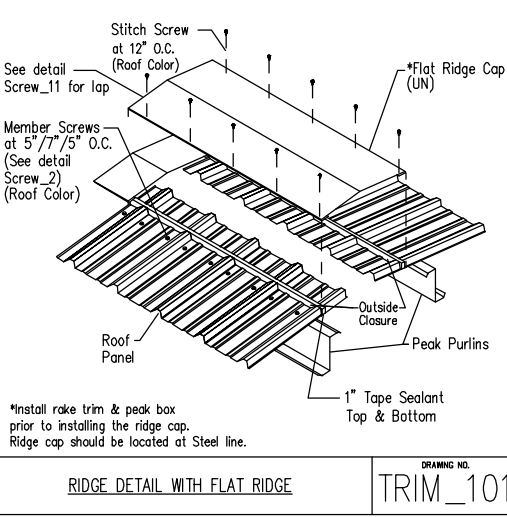
DRAWING NO. TRIM_60
 EAVE DETAIL WITH STYLE EAVE AT SHEETED WALL



DRAWING NO. TRIM_66
 RAKE DETAIL AT SHEETED WALL



DRAWING NO. TRIM_100
 STANDARD PEAK BOX DETAIL

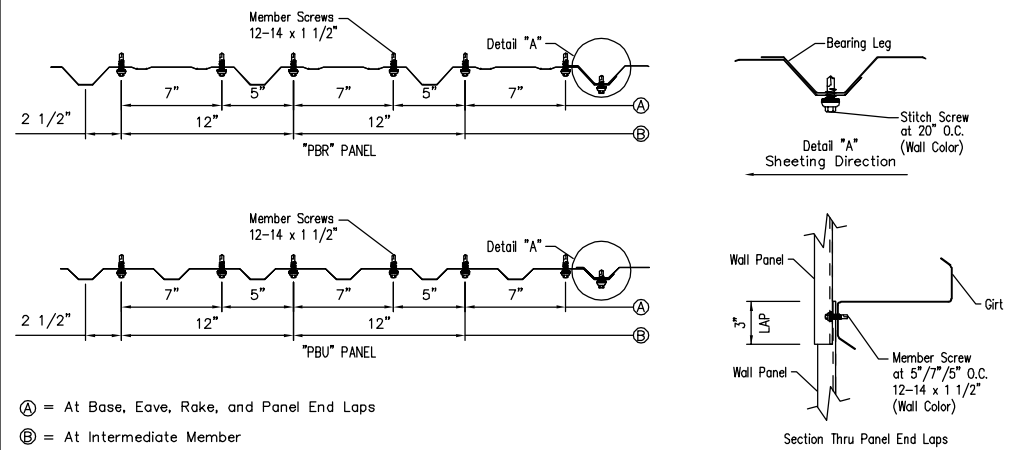


DRAWING NO. TRIM_101
 RIDGE DETAIL WITH FLAT RIDGE

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

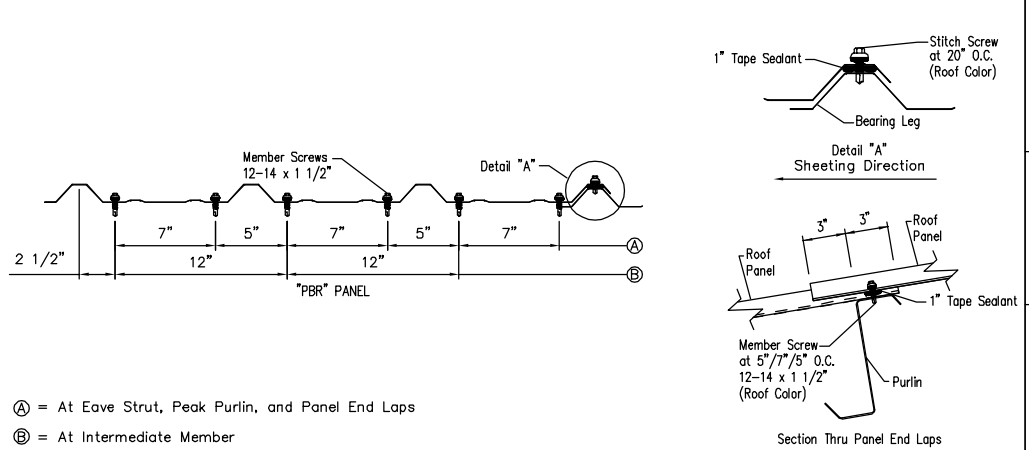
2 Inverness Drive East, Ste#200
 Englewood, Colorado 80112
 PHONE: 800-345-4610
 www.armstrongsteel.com

DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58143	ENG. BY: MZ DATE: 4/25/24 DWG. NO.: 12 OF 13 ISSUE: C



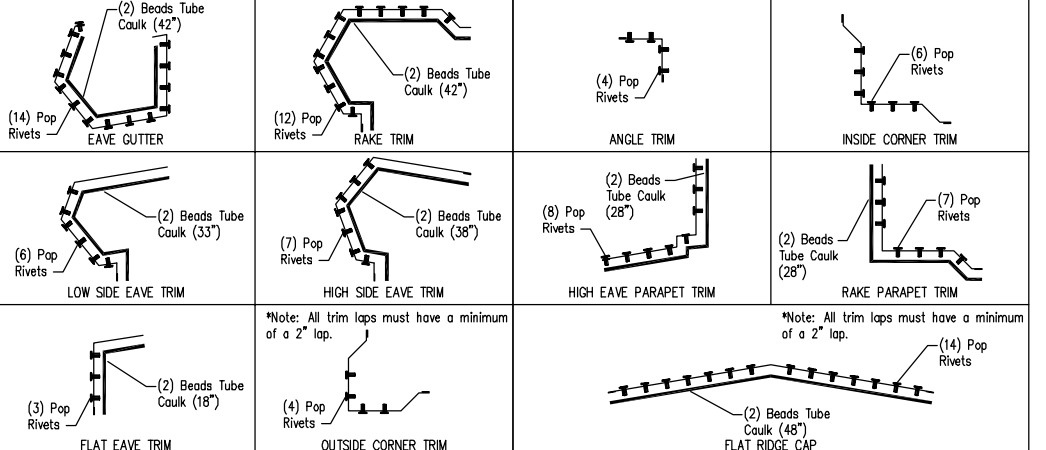
FASTENER LOCATION FOR WALL PANELS

SCREW_1



FASTENER LOCATION FOR ROOF PANELS

SCREW_2

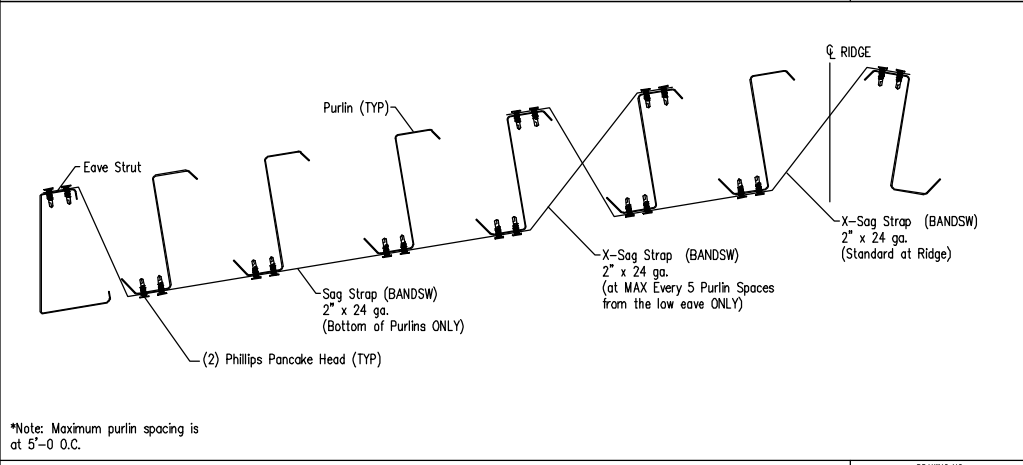


TRIM LAPS

SCREW_10

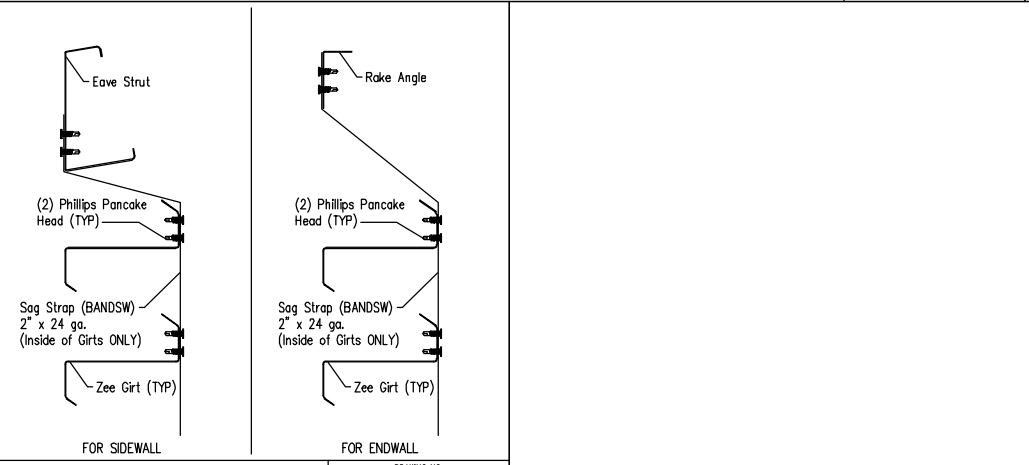
TRIM LAPS

SCREW_11



TYPICAL SAG STRAP AT GABLED ROOF

SCREW_15



TYPICAL SAG STRAP AT WALLS

SCREW_17

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	05.07.24	SS	SW	MZ
C	CONSTRUCTION	05.08.24	SS	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58143	ENG. BY: MZ DATE: 4/25/24 DWG. NO.: 13 OF 13 ISSUE: C