GENERAL NOTES

1.1 Fabrication shall be in accordance with A.S.C. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".

1.2 MATERIALS

	ASTM DESIGNATION	MIN. YIELD STRENGTH
Hot Rolled Steel Shapes (W, & C)	A572	Fy = 50 KSI
Hot Rolled Steel Angles (L)	A36	Fy = 36 KSI
Steel Pipes	A500	Fy = 42 KSI
Structural Tubing	A500	Fy = 42 KSI
Structural Steel Web Plate	A572/A1011	Fy = 50 KSI
Structural Steel Flange Plates/Bars	A529/A572	Fy = 55 KSI
Cold Formed Light Gage	A653/A1011	Fy = 55 KSI
Roof and Wall Sheets	A792/A653	Fy = 50, 80 KSI
Cable Brace	A475 – TYPE 1	Extra High Strength
Rod Brace	A529	Fy = 50 KSI
		MIN. TENSILE STRENGTH
Machine Bolts & Nuts	A307	Fu = 60 KSI
High Strength Bolts (1"ø and less)	A325–TYPE 1	Fu = 120 KSI
High Strength Bolts (>1"ø to 1 1/2"ø)	A325–TYPE 1	Fu = 105 KSI
Anchor Bolts (Not supplied by A.S.C.)	A36/A307/F1554	Fu = 60 KSI

1.3 PRIMER
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is A.S.C. Gray Oxide color. This paint is not intended for long term exposure to the elements. A.S.C. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. A.S.C. shall not be responsible for any field applied paint and/or coatings. (AISC Code of Standard Practice, Latest Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.

1.4 GALVANIZED OR SPECIAL COATINGS:
See Contract Documents

1.5 ALL BOLTS ARE 1/2"ø x 0'–1 1/4" A307 EXCEPT :
a) Endwall rafter splice – 5/8"ø x 0'–1 3/4" A325–N
b) Endwall column to rafter connection – 1/2"ø x 0'–1 1/4" A325 MIN.(SEE WALL ELEVATION)
c) Main frame connections – SEE CROSS SECTION
d) Flange Brace connections – 1/2"ø x 0'–1 1/4" A325

NOTE: Washers are not supplied unless noted otherwise on drawing

1.6 A325 BOLT TIGHTENING REQUIREMENTS

All high strength bolts are A325–N unless specifically noted otherwise. Holes are not slotted and design is bearing connection. Structural bolts shall be tightened by the turn–of–the–nut method in accordance with the Latest Edition AISC "Specification For Structural Joints " using ASTM A325 or A490 Bolts, when specifically required. A325–N bolts are supplied without washer unless otherwise noted on the drawings.

All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE– Under roof panels & base of wall panels
OUTSIDE – Between roof panels & ridge cap
– Between wall panels & eave/gable trim

1.8 ERECTION NOTE:
All bracing, strapping, & bridging shown and provided by A.S.C. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.9 ERECTION AND UNLOADING NOT BY A.S.C.

1.10 SHORTAGES
Any claims or shortages by buyer must be made to A.S.C. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
Claims for correction of alleged misfits will be disallowed unless A.S.C. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of A.S.C.

BUYER/END USE CUSTOMER RESPONSIBILITIES

2.1 **It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release A.S.C. to fabricate upon receiving such.**

2.2 Armstrong Steel Corp (hereafter referred to as A.S.C.) standard specifications apply unless stipulated otherwise in the Contract Documents. A.S.C. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.

2.3 In case of discrepancies between A.S.C. structural steel plans and plans for other trades, A.S.C. plans shall govern. (Section. 3 AISC Code of Standard Practices, Latest Edition)

2.4 Approval of A.S.C. drawings and calculations indicates that A.S.C. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the A.S.C. design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)

2.5 Once the BUYER/END USE CUSTOMER has signed A.S.C. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/ END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.

2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by A.S.C. and A.S.C. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or A.S.C. assumptions will govern (AISC Code of Standard Practice, Latest Edition)

ARMSTRONG STEEL

PHONE: 800–345–4610
www.armstrongsteel.com

JOB NO. : 58146

CUSTOMER :
END USER :
END USE :
LOCATION :
:
:
PH. NO. :

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

Design Code / Wind Code	: IBC–18
Building Risk Category	: II – Normal
Enclosure	: Enclosed
Dead Load (psf)	: 2.00
Collateral Load (psf)	: 1.00
Wind Load	
Ultimate Wind Speed, (Vult) (mph)	: 115.00
Wind Exposure	: C
Internal Pressure Coefficient, GCpi	: 0.18 /–0.18
Wall Panel Design Wind Pressure (psf)	: 25.70 /–27.80
Live Load	
Primary Framing (psf)	: 20.00
Trib. Area Reduction	: Yes
Secondary Framing (psf)	: 20.00
Snow Load	
Ground Snow Load, Pg (psf)	: 5.00
Roof Snow Load, Pf (psf)	: 5.00
Sloped Roof Snow Load, Ps (psf)	: 5.00
Snow Exposure Factor, Ce	: 1.00
Snow Importance Factor, Is	: 1.00
Thermal Factor, Ct	: 1.20
Sloped Factor, Cs	:
Seismic Load	
Seismic Importance Factor, Ie	: 1.00
Site Class	: D
Mapped Spectral Response Acceleration	: Ss = 0.205 :S1 = 0.071
Spectral Response Coefficients	: Sds = 0.218 :Sd1 = 0.114
Seismic Design Category	: B
Basic Force Resisting Systems Used	: Steel System Not Specifically Detailed For Resistance
	: Rigid Frames (OMF)
	: Braced Frames (OCBF/OMF)
Total Design Base Shear, V (kips)	: Longitudinal = 1.70
	: Transverse = 1.70
Response Modification Factors, R	: Rigid Frames = 3.00 Ω = 3.00
	: SW X–Bracing = 3.00 Ω = 3.00
Seismic Response Coefficient, Cs	: Rigid Frames = 0.0728
	: SW X–Bracing = 0.0728
Analysis Procedure Used	: Equivalent Lateral Force Procedure
Other Loads/Requirements	

BUILDING DESCRIPTION:

Width (ft)	: 50
Length (ft)	: 80
Eave Ht. at BSW (ft)	: 12
Eave Ht. at FSW (ft)	: 12
Roof Slope at BSW	: 2.0:12
Roof Slope at FSW	: 2.0:12
Bay Spacing (ft)	: 4 at 20

COVERING AND TRIMS:

Roof Panels & Trims

Panel Type	: 26 Ga. R–Loc
Panel Color	: Galvalume Plus
Trim Colors	
Gable/Eave Trim	: Charcoal 40 yr

Wall Panel & Trims

Panel Type	: 26 Ga. R–Loc
Panel Color	: Hawaiian 40 yr
Trim Colors	
Corner Trims	: Charcoal 40 yr
Opening Trims	: Charcoal 40 yr
Base Trim	: Hawaiian 40 yr

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT ARMSTRONG STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY A.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ARMSTRONG ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

Drawing Index

BUYER/END USE CUSTOMER RESPONSIBILITIES CONTINUED

2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that A.S.C. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that A.S.C. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by A.S.C.

2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with A.S.C. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (AISC Code of Standard Practice, Latest Edition.)

2.9 Armstrong Steel Corp is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete.
Unless otherwise provided in the Order Documents, A.S.C. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Latest MBMA Low Rise Building Systems Manual)

2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to A.S.C. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (AISC Code of Standard Practice Latest Edition)

2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (AISC Code of Standard Practice Latest Edition)

2.12 WARNING In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run–off from copper flashing, wiring, or tubing onto Galvalume should be avoided.

2.13 SAFETY COMMITMENT Armstrong Steel Corp has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of A.S.C.
It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees.
Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.

2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.

2.15 It is recommended by Factory Mutual (Reference: B2.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

NOTE:
For Snow/Ice Removal Procedure, Refer to Metal Building System Manual 2002 Edition, Section A8.4, Page XI–A8–2,

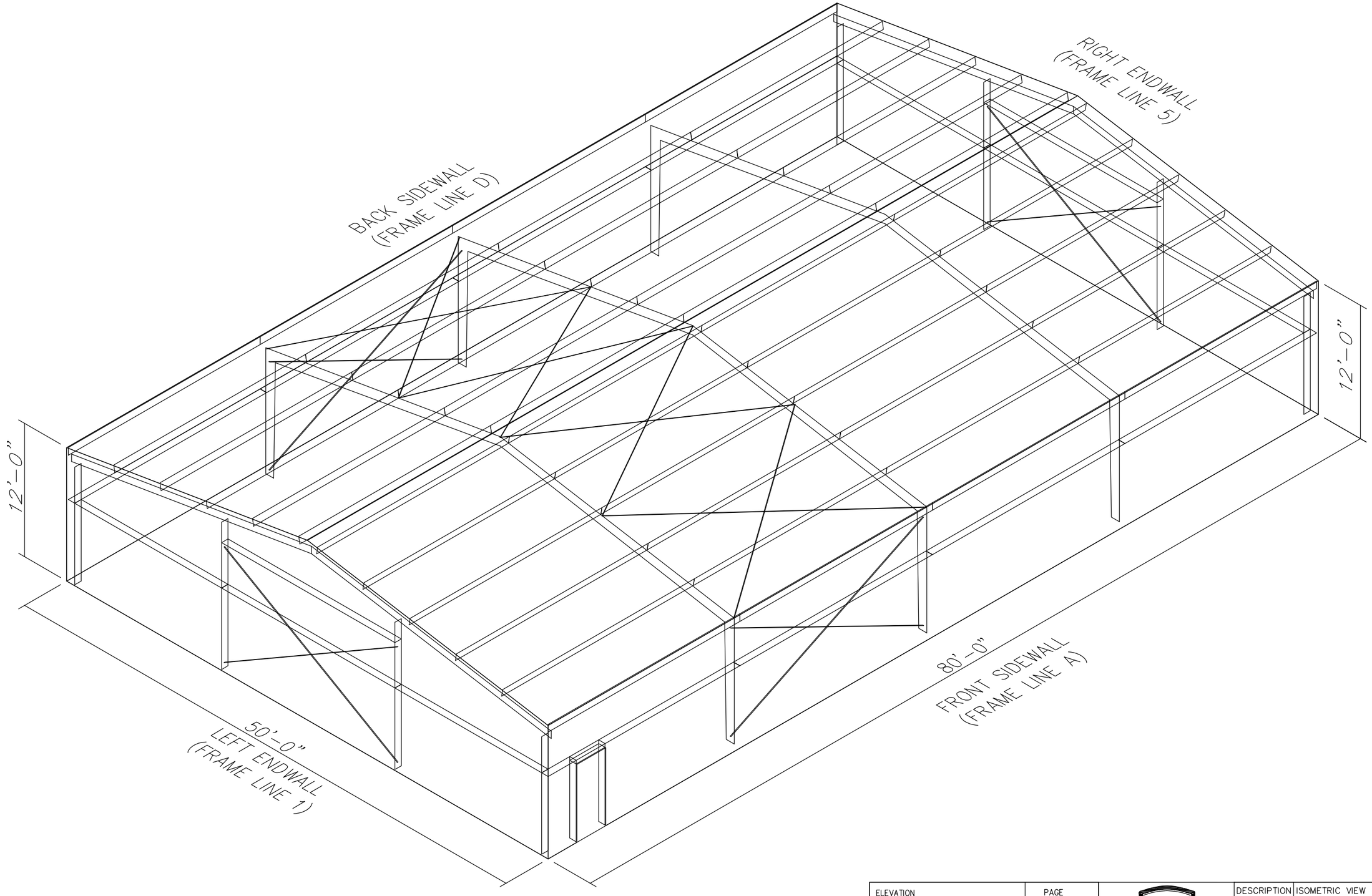
Drawing Status

☐ APPROVAL: ☐ REVISED APPROVAL:
These drawings, being for approval, are by definition not final, and are for conceptual representation only. their purpose is to confirm proper interpretation of the project documents. Only drawings issued "Construction" can be considered as complete.

☐ PERMIT: ☐ REVISED PERMIT:
These drawings, being for permit, are by definition not final. Only drawings issued "Construction" can be considered as complete.

☒ CONSTRUCTION:
Final drawings to be used in the erection of the building.

JOB NO : 58146



NOTE:
3D IS A GENERAL REPRESENTATION OF BUILDING.
SOME MEMBERS MAY CHANGE IN FINAL ERECTION DRAWINGS

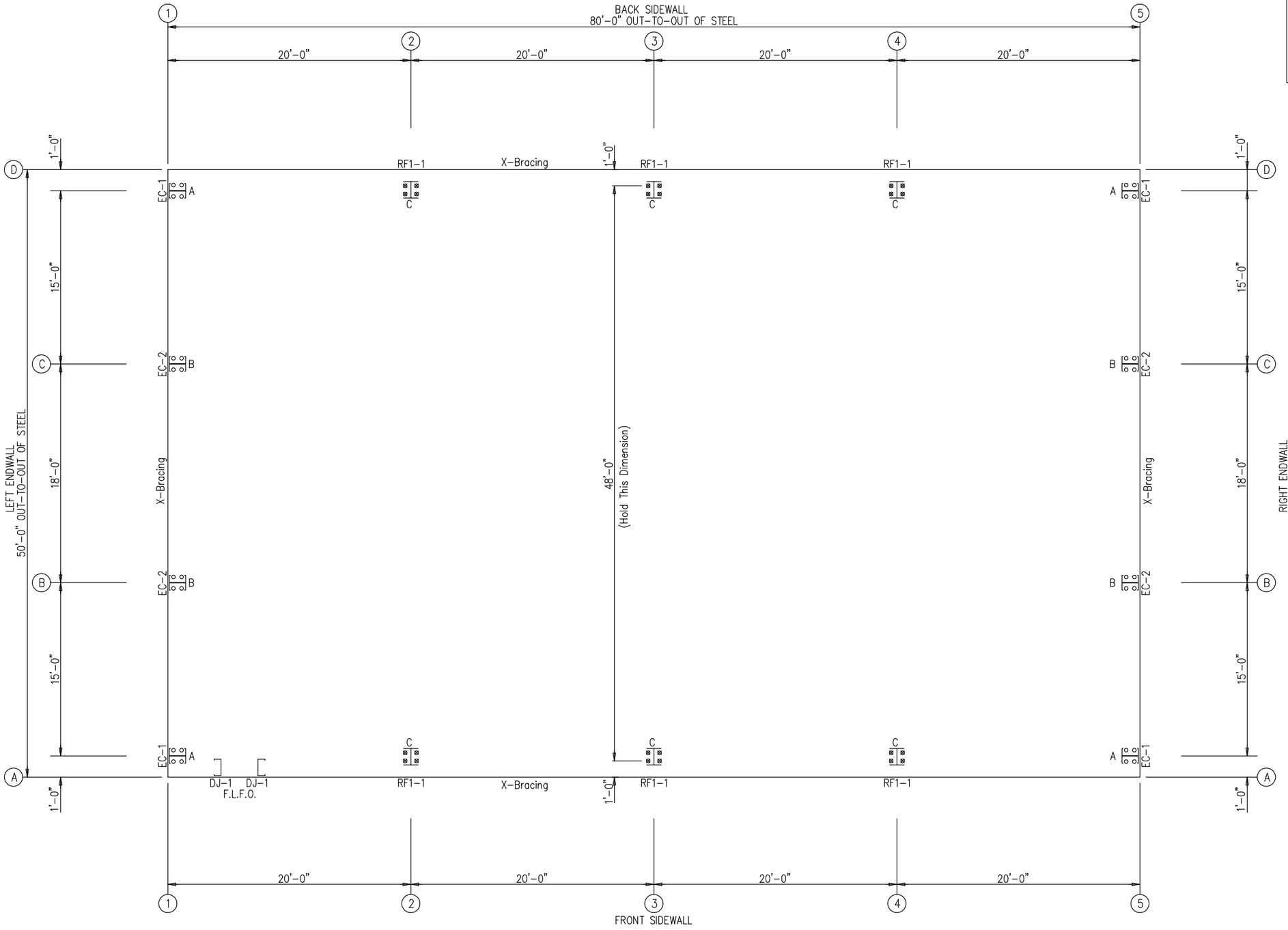
ELEVATION	PAGE
FRONT SIDEWALL (FRAME LINE A)	5 OF 13
BACK SIDEWALL (FRAME LINE D)	6 OF 13
LEFT ENDWALL (FRAME LINE 1)	7 OF 13
RIGHT ENDWALL (FRAME LINE 5)	8 OF 13

ARMSTRONG STEEL
2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ISOMETRIC VIEW
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.:	3D REFERENCE
DATE:	4/22/22
ISSUE:	C

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Type
○	32	Endwall	5/8"	A307
⊠	24	Frame	3/4"	A307



ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)
F.L.F.O = FIELD LOCATED FRAMED OPENING

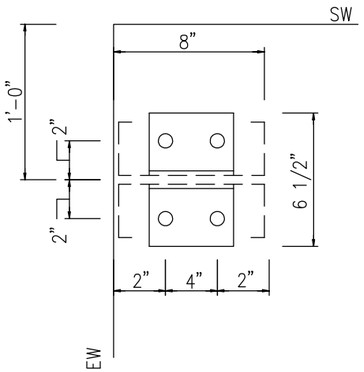
NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

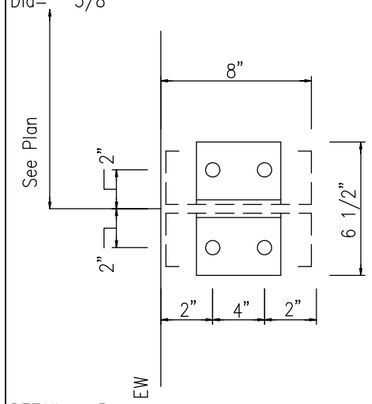
DESCRIPTION	ANCHOR BOLT PLAN
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.: 1 OF 13	DATE: 4/30/24
	ISSUE: C

Dia= 5/8"



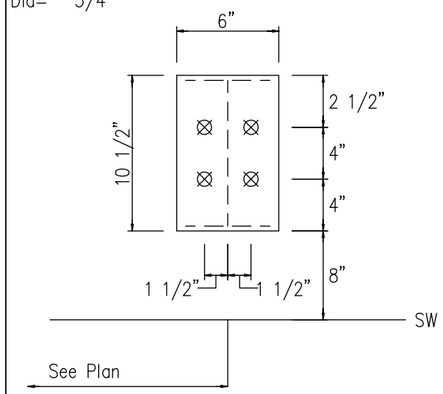
DETAIL A

Dia= 5/8"



DETAIL B

Dia= 3/4"



DETAIL C

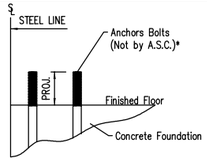
NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING MANUFACTURER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

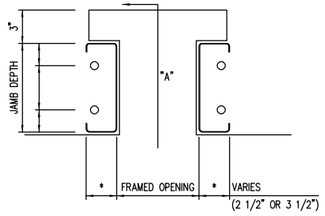
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.

Anchor Bolt Diameter	Projection
1/2"	1 1/2"
5/8"	2"
3/4"	2 1/2"
7/8"	3 1/2"
1"	3 1/2"
1 1/8"	3 1/2"
1 1/4"	3 1/2"

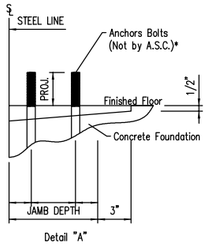


ANCHOR BOLT PROJECTION



DETAIL AT FRAMED OPENING

SLOPED CONCRETE NOTCH TO KEEP WATER OUT OF THE BUILDING



*Jamb not shown for clarity.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ANCHOR BOLT DETAILS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE

JOB NO.: 58146 | ENG. BY: MZ | DATE: 4/30/24 | DWG. NO.: 2 OF 13 | ISSUE: C

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	D	0.3	0.1	1.3	0.3	0.0	−1.8	0.0	−1.3	0.0	−1.0	0.0	−0.6	−0.9
1	C	0.6	0.2	3.8	0.9	−1.3	−6.0	0.0	−2.1	−1.3	−4.6	0.0	−0.6	−2.3
1	B	0.6	0.2	3.8	0.9	0.0	−2.1	1.3	−6.0	0.0	−0.6	1.3	−4.6	−2.3
1	A	0.3	0.1	1.3	0.3	0.0	−1.3	0.0	−1.8	0.0	−0.6	0.0	−1.0	−0.9

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Vert	E1UNB_SL_L− Horz	E1UNB_SL_L− Vert
1	D	1.1	0.0	−2.1	0.0	−1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.3
1	C	2.5	0.0	−4.4	−0.5	−3.3	−0.2	−0.2	0.0	0.2	0.0	0.0	1.2
1	B	2.5	0.5	−3.3	0.0	−4.4	0.0	0.2	0.2	−0.2	0.0	0.0	0.5
1	A	1.1	0.0	−1.3	0.0	−2.1	0.0	0.0	0.0	0.0	0.0	0.0	0.1

Frm Line	Col Line	E1UNB_SL_R− Horz	E1UNB_SL_R− Vert
1	D	0.0	0.1
1	C	0.0	0.5
1	B	0.0	1.2
1	A	0.0	0.3

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
5	A	0.3	0.1	1.3	0.3	0.0	−1.8	0.0	−1.3	0.0	−1.0	0.0	−0.6	−0.9
5	B	0.6	0.2	3.8	0.9	−1.3	−6.0	0.0	−2.1	−1.3	−4.6	0.0	−0.6	−2.3
5	C	0.6	0.2	3.8	0.9	0.0	−2.1	1.3	−6.0	0.0	−0.6	1.3	−4.6	−2.3
5	D	0.3	0.1	1.3	0.3	0.0	−1.3	0.0	−1.8	0.0	−0.6	0.0	−1.0	−0.9

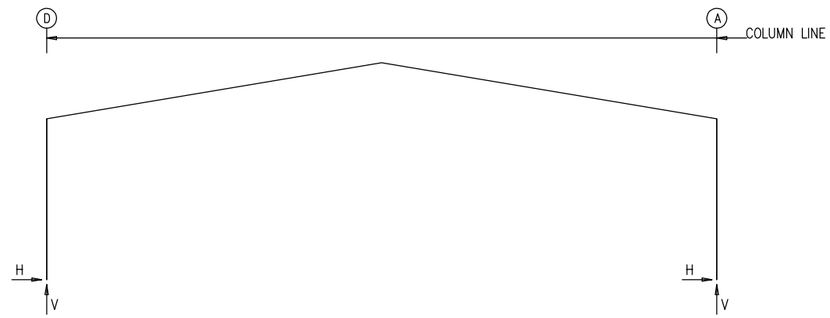
Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Vert	E2UNB_SL_L− Horz	E2UNB_SL_L− Vert
5	A	1.1	0.0	−2.1	0.0	−1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.3
5	B	2.5	0.0	−4.4	−0.5	−3.3	−0.2	−0.2	0.0	0.2	0.0	0.0	1.2
5	C	2.5	0.5	−3.3	0.0	−4.4	0.0	0.2	0.2	−0.2	0.0	0.0	0.5
5	D	1.1	0.0	−1.3	0.0	−2.1	0.0	0.0	0.0	0.0	0.0	0.0	0.1

Frm Line	Col Line	E2UNB_SL_R− Horz	E2UNB_SL_R− Vert
5	A	0.0	0.1
5	B	0.0	0.5
5	C	0.0	1.2
5	D	0.0	0.3

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
1	D	6	0.6	−1.1	7	−0.6	−1.1	4	0.625	6.500	7.750	0.250	0.0
		1	0.0	1.6	6	0.6	−1.1						
1	C	8	1.5	−3.2	7	−1.4	−2.3	4	0.625	6.500	7.750	0.250	0.0
		1	0.0	4.6	8	1.5	−3.2						
1	B	9	1.5	−3.2	10	−1.4	−2.3	4	0.625	6.500	7.750	0.250	0.0
		1	0.0	4.6	9	1.5	−3.2						
1	A	11	0.6	−1.1	10	−0.6	−1.1	4	0.625	6.500	7.750	0.250	0.0
		1	0.0	1.6	11	0.6	−1.1						
5	A	6	0.6	−1.1	7	−0.6	−1.1	4	0.625	6.500	7.750	0.250	0.0
		1	0.0	1.6	6	0.6	−1.1						
5	B	8	1.5	−3.2	7	−1.4	−2.3	4	0.625	6.500	7.750	0.250	0.0
		1	0.0	4.6	8	1.5	−3.2						
5	C	9	1.5	−3.2	10	−1.4	−2.3	4	0.625	6.500	7.750	0.250	0.0
		1	0.0	4.6	9	1.5	−3.2						
5	D	11	0.6	−1.1	10	−0.6	−1.1	4	0.625	6.500	7.750	0.250	0.0
		1	0.0	1.6	11	0.6	−1.1						

FRAME LINES: 2 3 4



RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
2*	D	0.8	1.5	0.3	0.5	3.5	6.0	1.5	2.5	−6.8	−10.0	−2.2	−7.2
2*	A	−0.8	1.5	−0.3	0.5	−3.5	6.0	−1.5	2.5	2.2	−7.2	6.8	−10.0

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
2*	D	−5.5	−5.7	−1.1	−3.0	−2.6	−10.7	−3.3	−9.1	−0.2	−0.1	0.2	0.1
2*	A	1.1	−3.0	5.5	−5.7	3.3	−9.1	2.6	−10.7	−0.2	0.1	0.2	−0.1

Frame Line	Column Line	Seismic_Long Horz	Seismic_Long Vert	F1UNB_SL_L− Horz	F1UNB_SL_L− Vert	F1UNB_SL_R− Horz	F1UNB_SL_R− Vert
2*	D	0.0	−0.4	1.3	2.5	1.3	1.5
2*	A	0.0	−0.4	−1.3	1.5	−1.3	2.5

2* Frame lines: 2 3 4

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
2*	D	1	4.6	8.0	2	−3.6	−5.1	4	0.750	6.000	10.50	0.500	0.0
					4	−1.1	−5.5						
2*	A	3	3.6	−5.1	1	−4.6	8.0	4	0.750	6.000	10.50	0.500	0.0
		1	−4.6	8.0	5	1.1	−5.5						

2* Frame lines: 2 3 4

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
○ 32	Endwall	5/8"	A307
⊠ 24	Frame	3/4"	A307

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Panel_Shear (lb/ft) Seis
L_EW	1	C,B	1.3	1.0	0.3	0.2		
F_SW	A	2,3	2.9	1.4	0.8	0.4		
R_EW	5	B,C	1.3	1.0	0.3	0.2		
B_SW	D	3,2	2.9	1.4	0.8	0.4		

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	50.0
Length	(ft)	=	80.0
Eave Height	(ft)	=	12.0/ 12.0
Roof Slope	(rise/12)	=	2.00/ 2.00
Roof Dead Load	(psf)	=	2.0
Wall Dead Load			
Left Endwall	(psf)	=	2.0
Right Endwall	(psf)	=	2.0
Front Sidewall	(psf)	=	2.0
Back Sidewall	(psf)	=	2.0
Roof Live Load	(psf)	=	20.0
Frame Live Load	(psf)	=	12.0
Collateral Load	(psf)	=	1.0
Snow Load	(psf)	=	5.0
Wind Speed	(mph)	=	115.0
Wind Code		=	IBC 18
Exposure		=	C
Closure		=	Enclosed
Internal Wind Coeff		=	−0.18, +0.18
Risk Category		=	II − Normal
Importance − Wind		=	1.00
Importance − Seismic		=	1.00
Seismic Design Category		=	B
Seismic Coeff	(Sms)	=	0.33

ID	Description
1	Dead+Collateral+Live
2	0.6Dead+0.6Wind_Left1
3	0.6Dead+0.6Wind_Right1
4	0.6Dead+0.6Wind_Long1L
5	0.6Dead+0.6Wind_Long2L
6	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
7	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
8	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
9	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
10	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
11	0.6Dead+0.6Wind_Suction+0.6Wind_Long2L

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

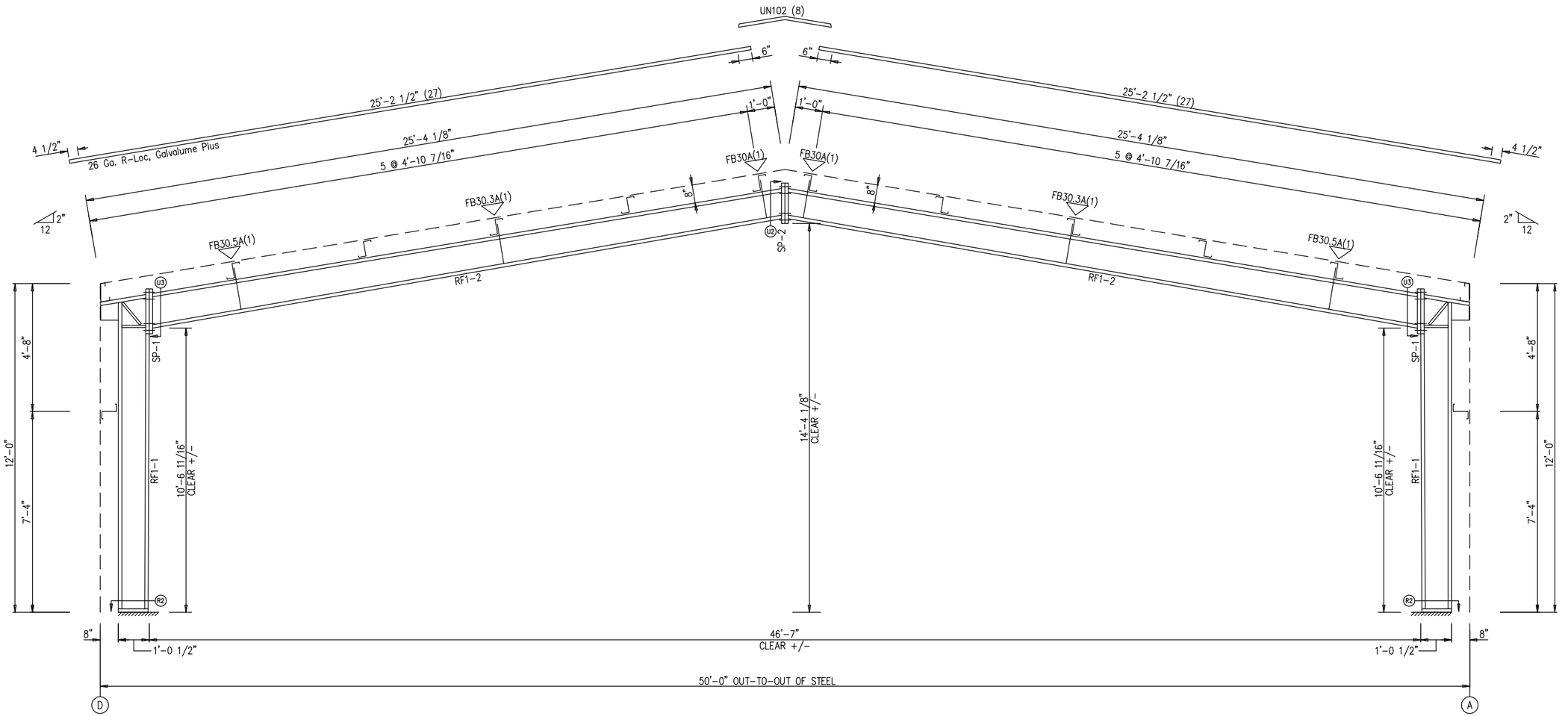
2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800−345−4610
www.armstrongsteel.com

DESCRIPTION	ANCHOR BOLT REACTIONS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.:	58146
ENG. BY:	MZ
DATE:	4/30/24
DWG. NO.:	3 OF 13
ISSUE:	C

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	0	A325	0.750	2.00
SP-2	4	4	0	A325	0.625	1.75

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	10.0/12.0	0.135	11'-6 7/16"	5 x 1/4" x 11'-4 3/8" 5 x 5/16" x 1'-8 1/2"	5 x 1/4" x 10'-3"
RF1-2	12.0/10.3 10.3/10.0	0.135 0.135	19'-10" 3'-10 3/16"	5 x 1/4" x 23'-6 3/16"	5 x 1/4" x 23'-6 1/2"

▽FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - B316



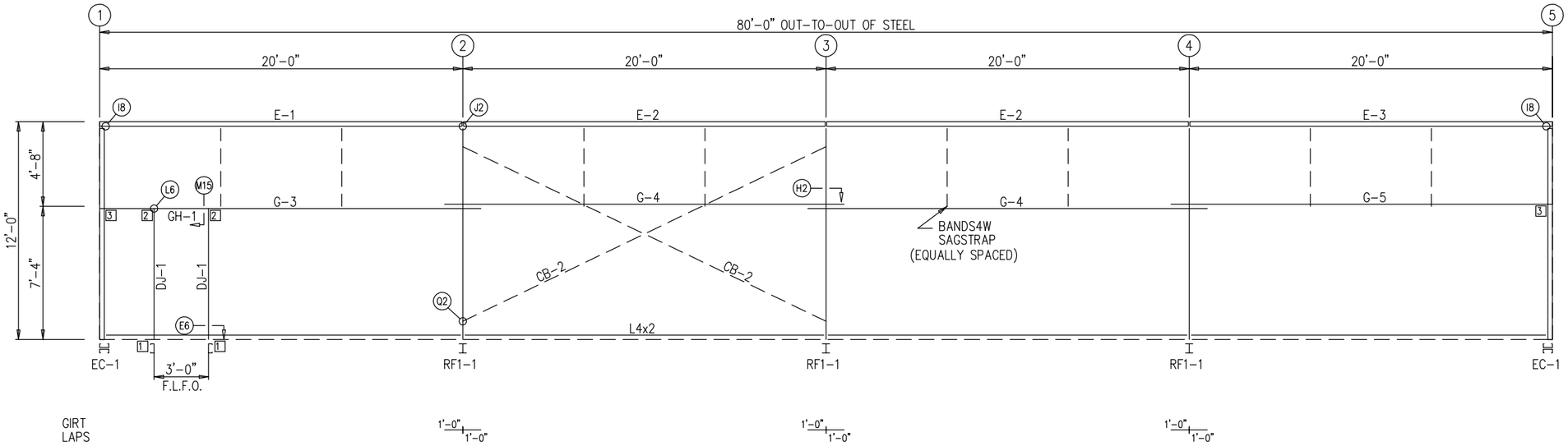
RIGID FRAME ELEVATION: FRAME LINE 2 3 4

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	RIGID FRAME ELEVATION
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.: 4 OF 13	DATE: 4/30/24
	ISSUE: C

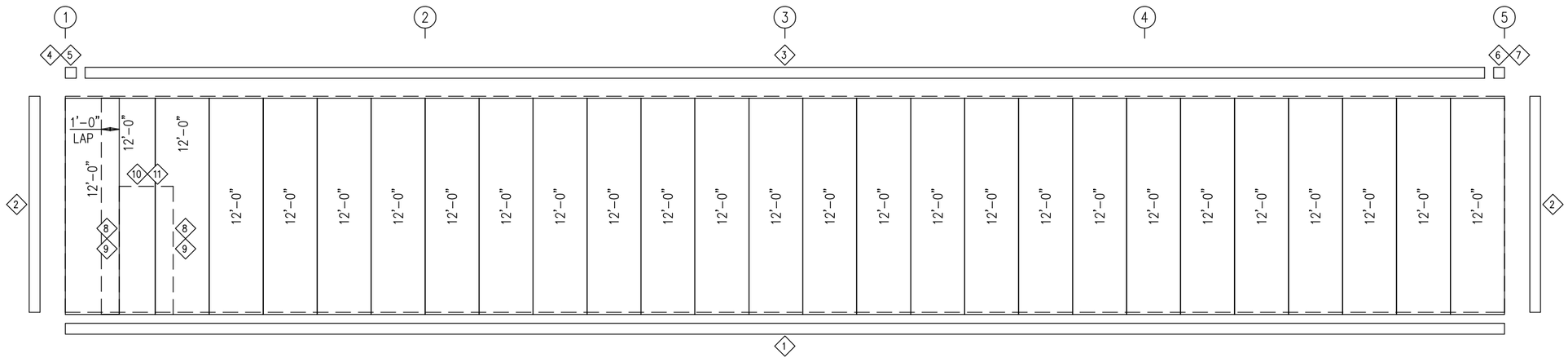


FRONT SIDEWALL FRAMING: FRAME LINE A

TRIM TABLE FRAME LINE A				
◇ID	QUAN	PART	LENGTH	DETAIL
1	4	BA6	20'-4"	TRIM_1
2	2	OU6	12'-2"	TRIM_30
3	4	Q7726	20'-4"	TRIM_61
4	1	Q773L6	6"	
5	1	AR961L6	7 7/16"	TRIM_60
6	1	Q773R6	6"	
7	1	AR961R6	7 7/16"	TRIM_60
8	2	Q3706	7'-4"	TRIM_50
9	2	JA6	7'-2"	TRIM_50
10	1	AR3806	3'-7"	TRIM_51
11	1	HE6	3'-6"	TRIM_51

MEMBER TABLE FRAME LINE A			
QUAN	MARK	PART	LENGTH
2	DJ-1	8X25C16	7'-0"
1	E-1	08536DU2	19'-11"
2	E-2	08536DU2	19'-11"
1	E-3	08536DU2	19'-11"
1	G-3	8X25Z16	20'-11 1/2"
2	G-4	8X25Z16	22'-0"
1	G-5	8X25Z16	20'-11 1/2"
2	CB-2	GS1716	22'-6 1/4"
1	GH-1	HW816	3'-0"

CONNECTION PLATES FRAME LINE A		
□ID	QUAN	MARK/PART
1	2	BC-05
2	2	BC-01
3	1	BC-09



FRONT SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. R-Loc – Hawaiian 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

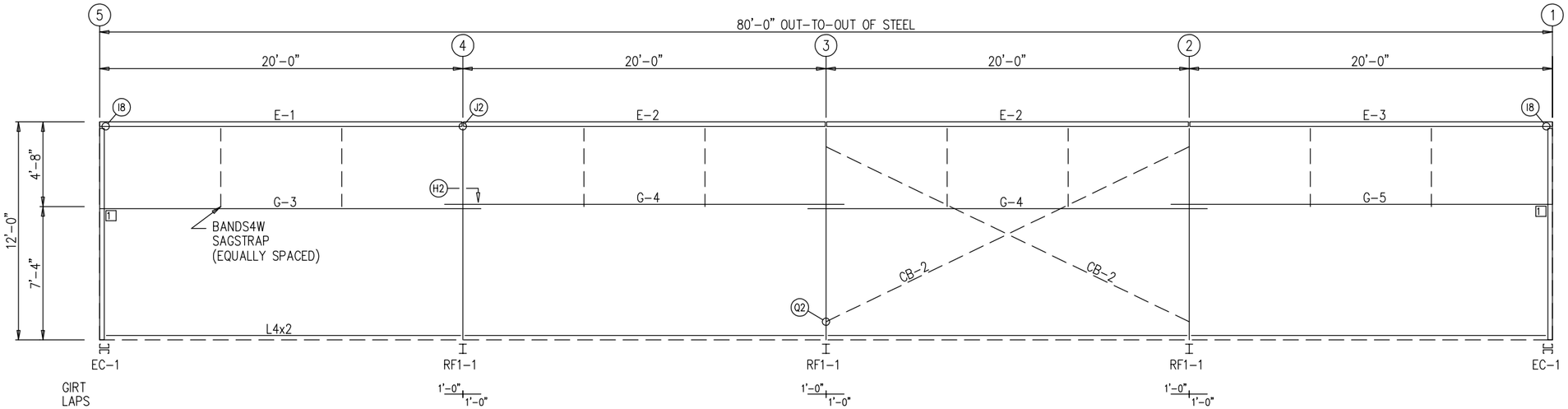
2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	SIDEWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.: 5 OF 13	DATE: 4/30/24
	ISSUE: C

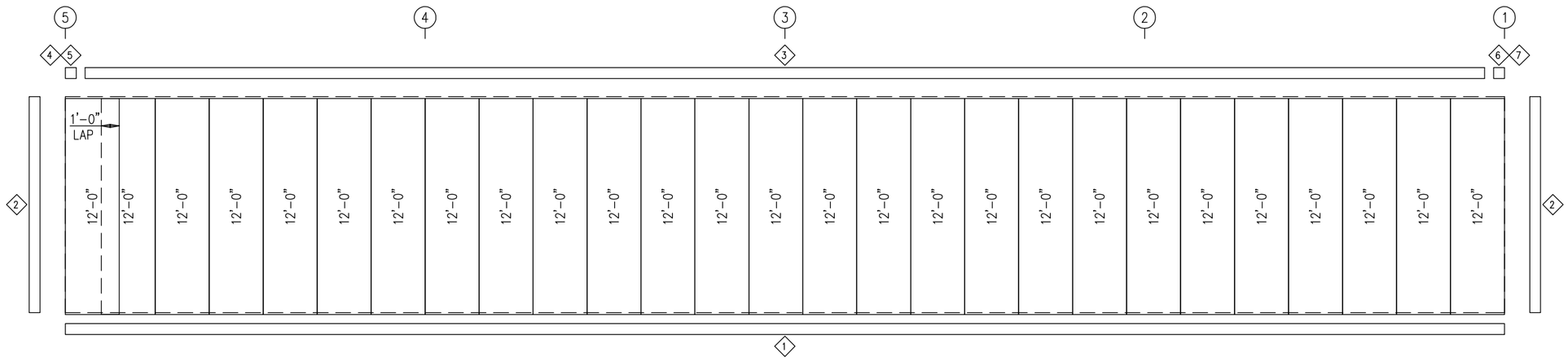
TRIM TABLE FRAME LINE D				
◇ID	QUAN	PART	LENGTH	DETAIL
1	4	BA6	20'-4"	TRIM_1
2	2	OU6	12'-2"	TRIM_30
3	4	Q7726	20'-4"	TRIM_61
4	1	Q773L6	6"	
5	1	AR961L6	7 7/16"	TRIM_60
6	1	Q773R6	6"	
7	1	AR961R6	7 7/16"	TRIM_60

MEMBER TABLE FRAME LINE D			
QUAN	MARK	PART	LENGTH
1	E-1	08536DU2	19'-11"
2	E-2	08536DU2	19'-11"
1	E-3	08536DU2	19'-11"
1	G-3	8X25Z16	20'-11 1/2"
2	G-4	8X25Z16	22'-0"
1	G-5	8X25Z16	20'-11 1/2"
2	CB-2	GS1716	22'-6 1/4"

CONNECTION PLATES FRAME LINE D		
□ID	QUAN	MARK/PART
1	1	BC-09



BACK SIDEWALL FRAMING: FRAME LINE D



BACK SIDEWALL SHEETING & TRIM: FRAME LINE D

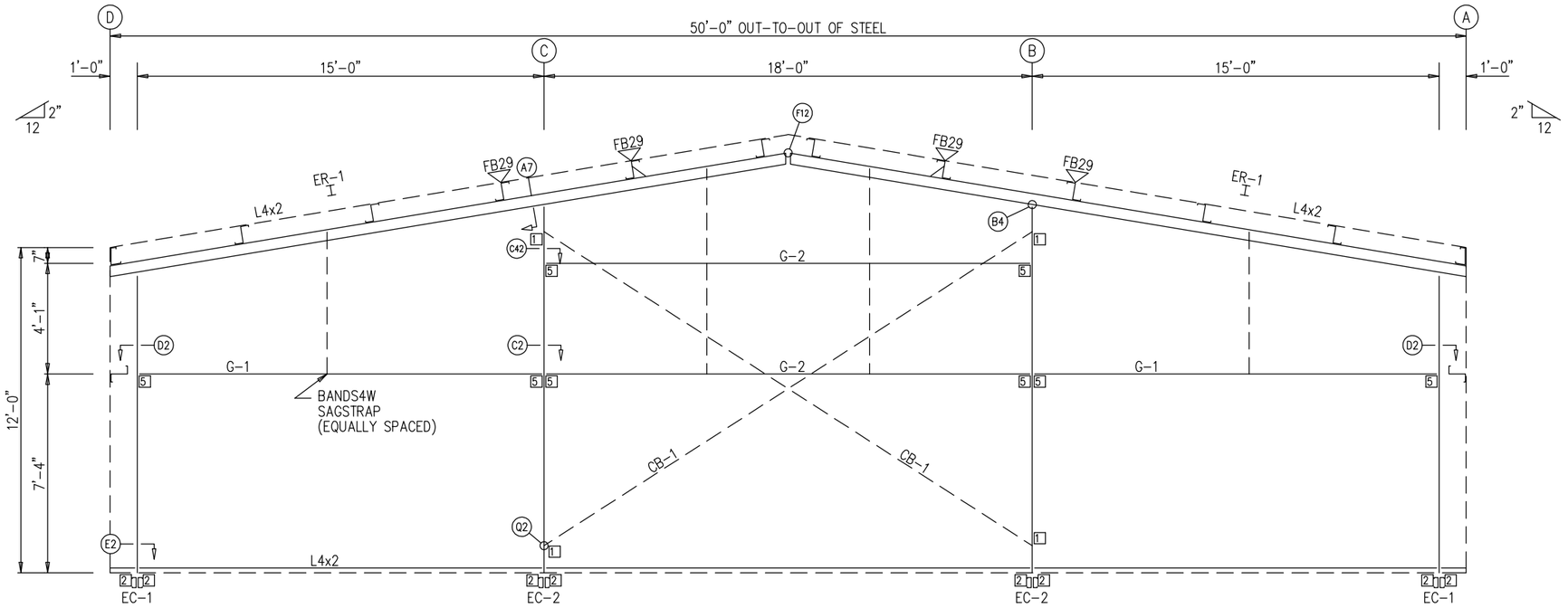
PANELS: 26 Ga. R-Loc – Hawaiian 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

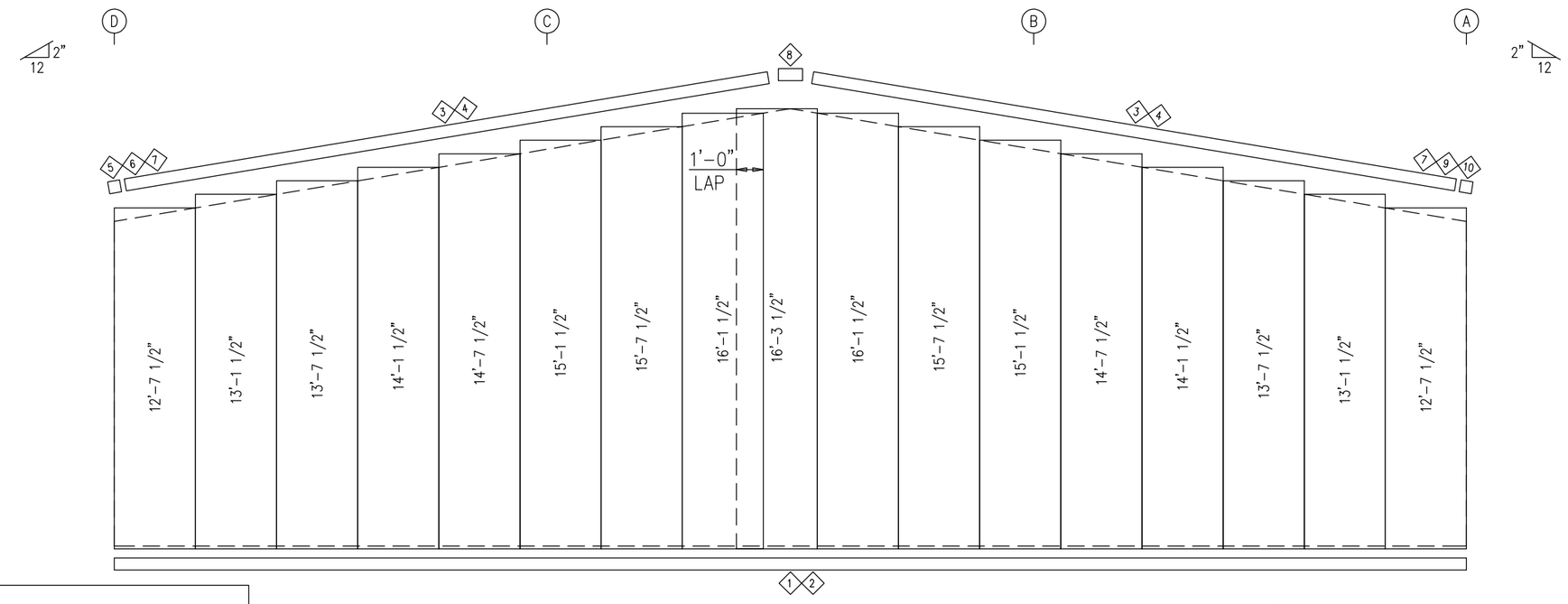
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	SIDEWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.: 6 OF 13	DATE: 4/30/24
	ISSUE: C



LEFT ENDWALL FRAMING: FRAME LINE 1



LEFT ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. R-Loc - Hawaiian 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

TRIM TABLE
FRAME LINE 1

◇ID	QUAN	PART	LENGTH	DETAIL
1	1	BA6102	10'-2"	TRIM_1
2	2	BA6	20'-4"	TRIM_1
3	2	Q7646	12'-2"	TRIM_66
4	2	Q7646	14'-2"	TRIM_66
5	1	Q765L6	6"	
6	1	AR963L6	9 1/8"	
7	2	AR9626	8 1/16"	
8	1	Q7676	1'-4"	TRIM_100
9	1	Q765R6	6"	
10	1	AR963R6	9 1/8"	

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 1/2"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE
FRAME LINE 1

QUAN	MARK	PART	LENGTH
2	EC-1	8X50D16	10'-7 7/8"
2	EC-2	8X50D16	13'-1 7/8"
2	ER-1	W8X10	25'-3 5/8"
2	G-1	8X25Z16	14'-4"
2	G-2	8X25Z16	17'-4"
2	CB-1	GS1716	21'-8 1/4"

FLANGE BRACE TABLE
FRAME LINE 1

▽ID	QUAN	MARK	LENGTH
1	4	FB29	2'-5"

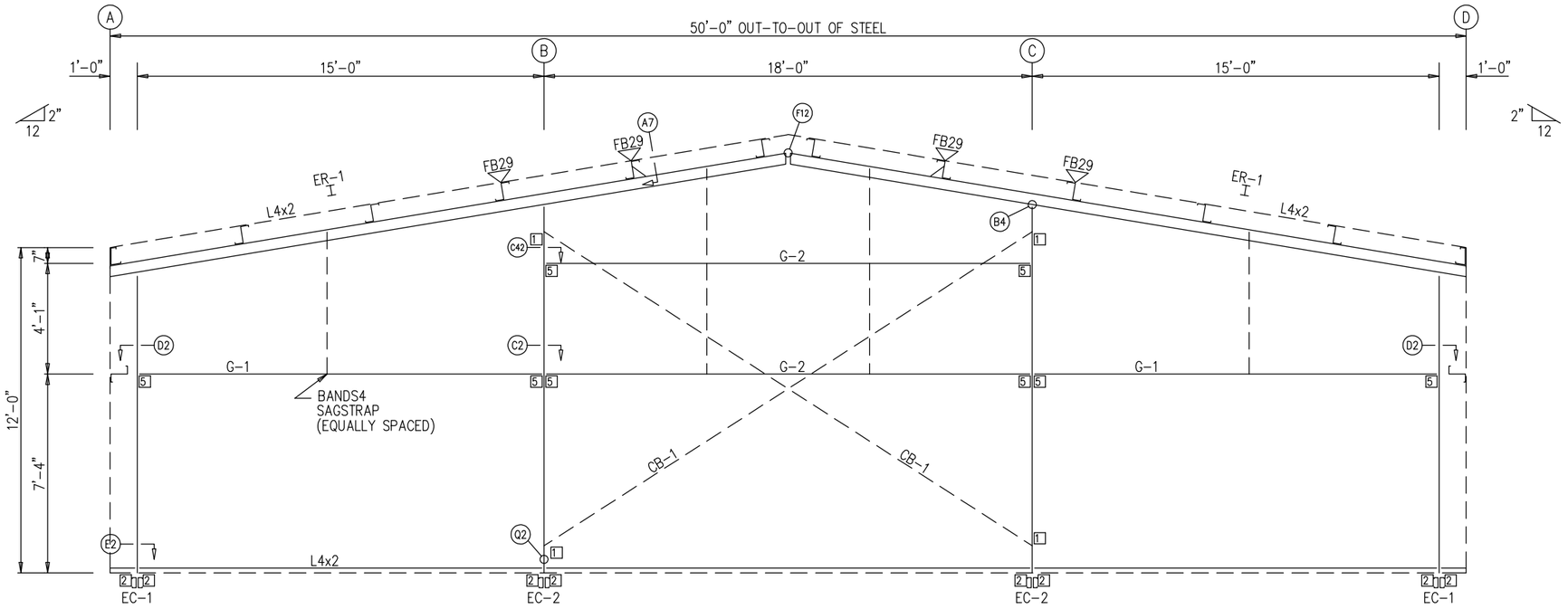
CONNECTION PLATES
FRAME LINE 1

□ID	QUAN	MARK/PART
1	4	BC-50A
2	8	BC-04
5	8	BC-01

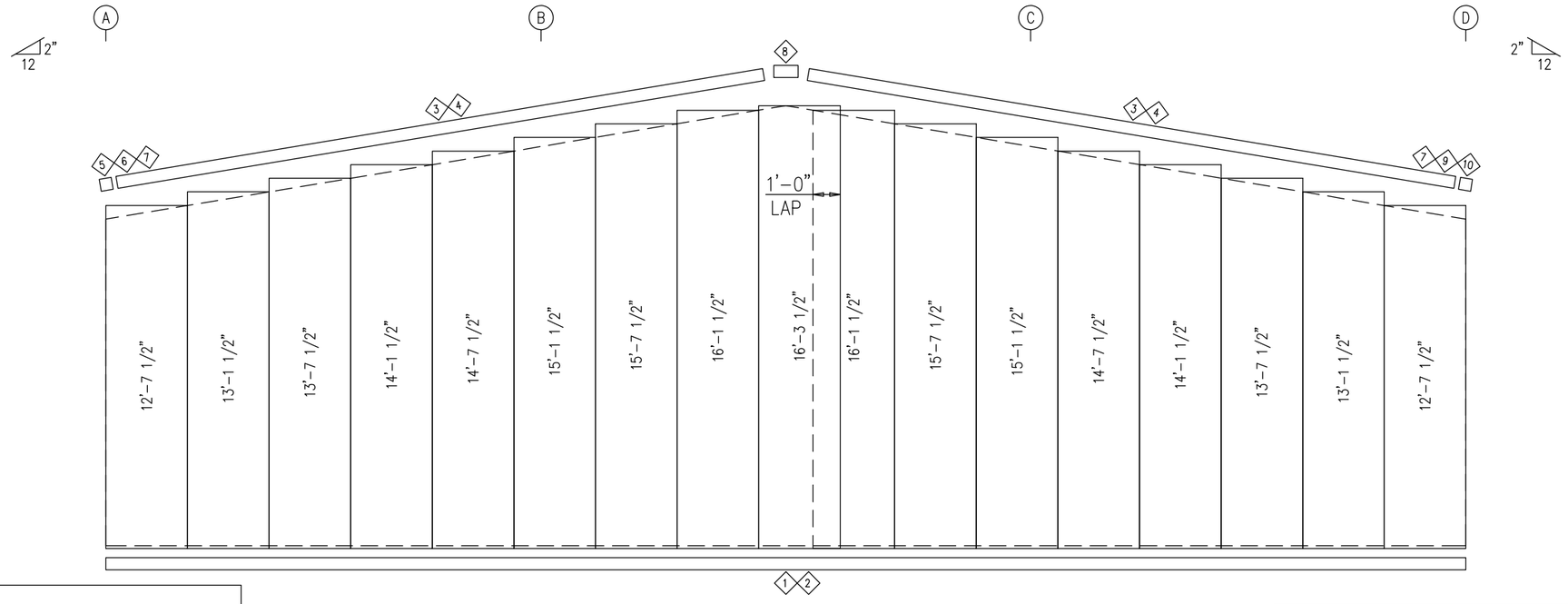
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ENDWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.: 7 OF 13	DATE: 4/30/24
	ISSUE: C



RIGHT ENDWALL FRAMING: FRAME LINE 5



RIGHT ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. R-Loc – Hawaiian 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

TRIM TABLE
FRAME LINE 5

◇ID	QUAN	PART	LENGTH	DETAIL
1	1	BA6102	10'-2"	TRIM_1
2	2	BA6	20'-4"	TRIM_1
3	2	Q7646	12'-2"	TRIM_66
4	2	Q7646	14'-2"	TRIM_66
5	1	Q765L6	6"	
6	1	AR963L6	9 1/8"	
7	2	AR9626	8 1/16"	
8	1	Q7676	1'-4"	TRIM_100
9	1	Q765R6	6"	
10	1	AR963R6	9 1/8"	

BOLT TABLE
FRAME LINE 5

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 1/2"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE
FRAME LINE 5

QUAN	MARK	PART	LENGTH
2	EC-1	8X50D16	10'-7 7/8"
2	EC-2	8X50D16	13'-1 7/8"
2	ER-1	W8X10	25'-3 5/8"
2	G-1	8X25Z16	14'-4"
2	G-2	8X25Z16	17'-4"
2	CB-1	GS1716	21'-8 1/4"

FLANGE BRACE TABLE
FRAME LINE 5

▽ID	QUAN	MARK	LENGTH
1	4	FB29	2'-5"

CONNECTION PLATES
FRAME LINE 5

□ID	QUAN	MARK/PART
1	4	BC-50A
2	8	BC-04
5	8	BC-01

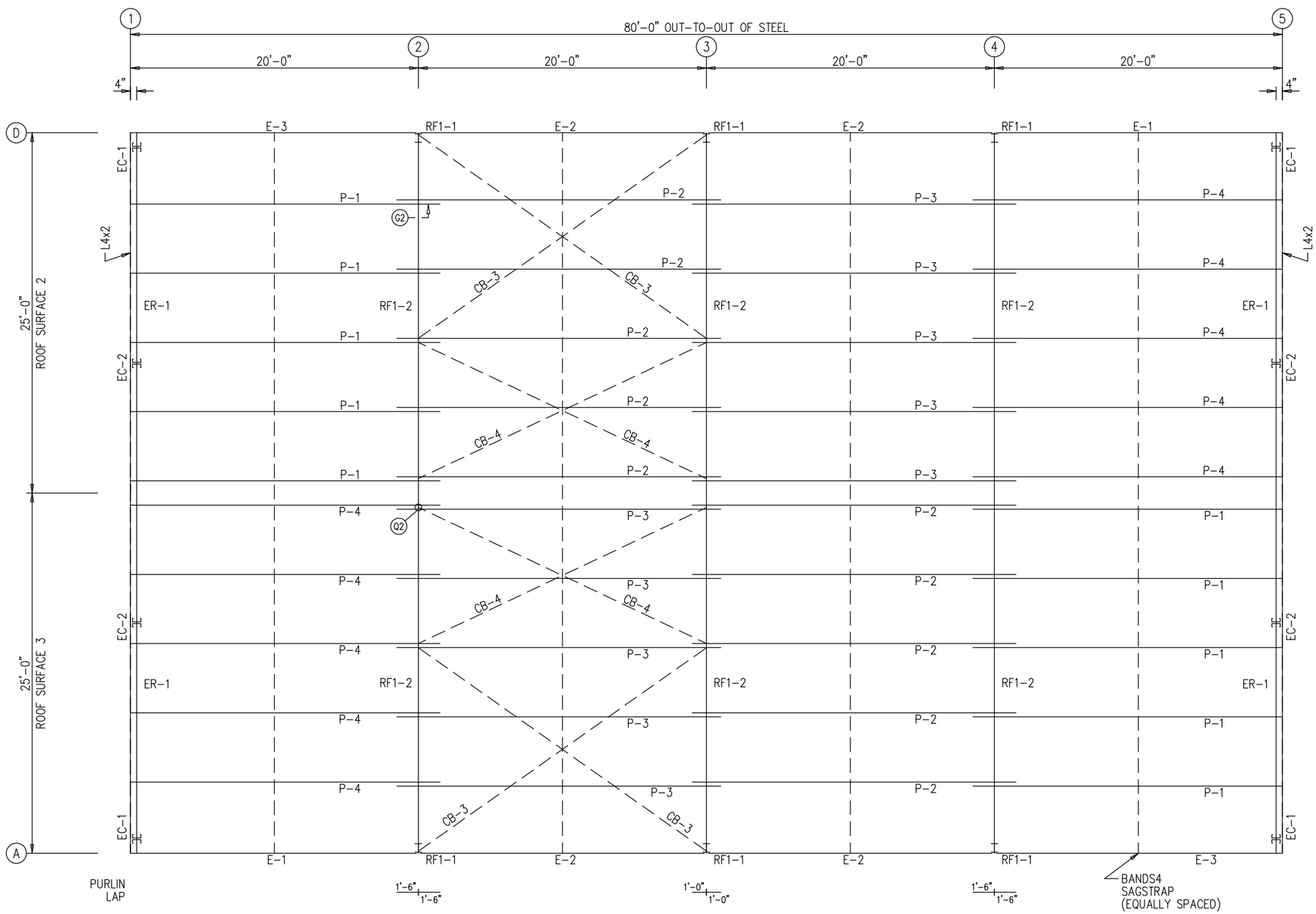
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ENDWALL FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.: 8 OF 13	DATE: 4/30/24
	ISSUE: C

TRIM TABLE ROOF PLAN				
◇ID	QUAN	PART	LENGTH	DETAIL
1	8	UN6102	10'-2"	TRIM_101

MEMBER TABLE ROOF PLAN			
QUAN	MARK	PART	LENGTH
10	P-1	8x25Z16	21'-5 1/2"
10	P-2	8x25Z16	22'-6"
10	P-3	8x25Z16	22'-6"
10	P-4	8x25Z16	21'-5 1/2"
2	E-1	08536DU2	19'-11"
4	E-2	08536DU2	19'-11"
2	E-3	08536DU2	19'-11"
4	CB-3	GS1716	23'-5 3/4"
4	CB-4	GS1716	22'-3 1/4"



ROOF FRAMING PLAN

ROOF SHEETING

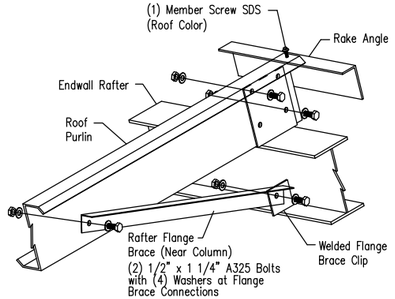
PANELS: 26 Ga. R-Loc
Galvalume Plus

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

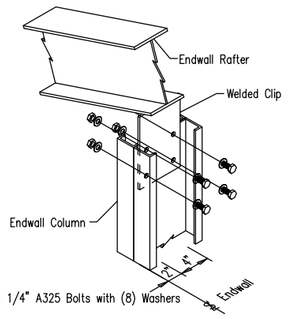
2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ROOF FRAMING & SHEETING
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.: 9 OF 13	DATE: 4/30/24
	ISSUE: C



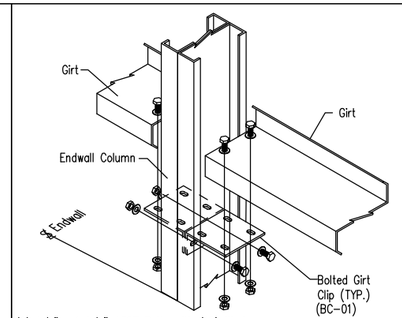
(2) 1/2" x 1 1/4" A307 Bolts with (4) Washers
(Typ.) (U.N.)

A7 ROOF PURLIN TO HOT-ROLLED RAFTER



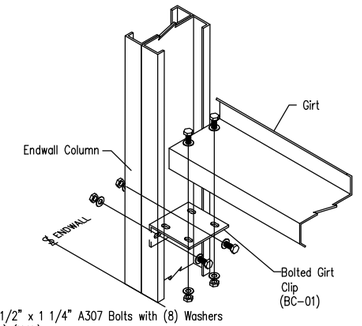
(4) 1/2" x 1 1/4" A325 Bolts with (8) Washers
(Typ.) (U.N.)

B4 ENDWALL COLUMN TO ENDWALL RAFTER



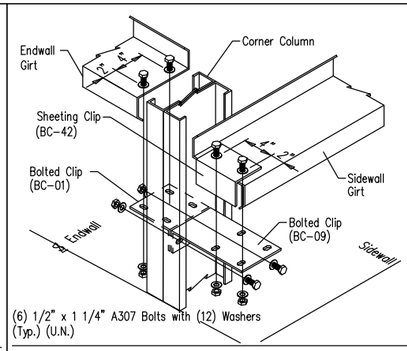
(6) 1/2" x 1 1/4" A307 Bolts with (12) Washers
(Typ.) (U.N.)

C2 WALL GIRTS TO ENDWALL COLUMN



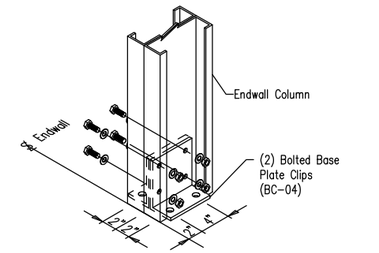
(4) 1/2" x 1 1/4" A307 Bolts with (8) Washers
(Typ.) (U.N.)

C42 WALL GIRT TO ENDWALL COLUMN



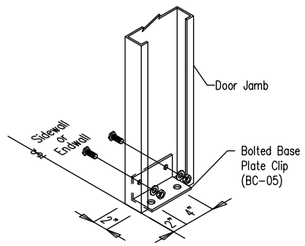
(6) 1/2" x 1 1/4" A307 Bolts with (12) Washers
(Typ.) (U.N.)

D2 WALL GIRTS TO ENDWALL CORNER COLUMN



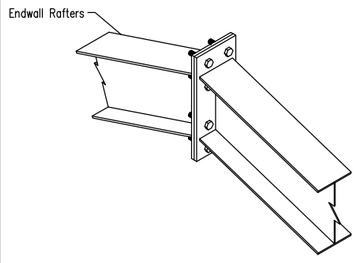
(4) 1/2" x 1 1/2" A307 Bolts with (8) Washers
(Typ.) (U.N.)
For Anchor Bolt Size: Refer to Anchor Bolt Plan

E2 BASE PLATE FOR ENDWALL COLUMN



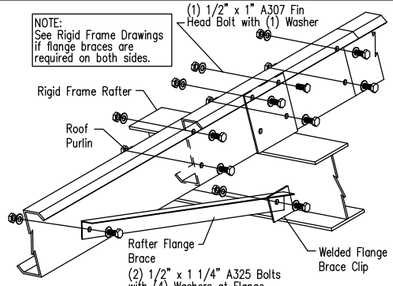
(2) 1/2" x 1" A307 Fin Head Bolts with (2) Washers
(Typ.) (U.N.)
For Anchor Bolt Size: Refer to Anchor Bolt Plan

E6 BASE PLATE FOR DOOR JAMB



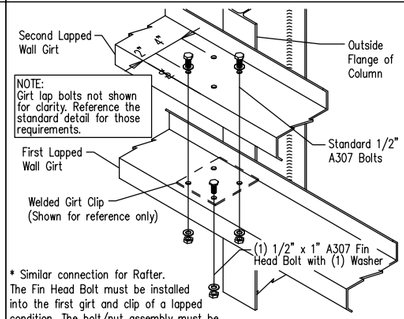
Refer To Endwall Drawing
For Bolt Quantity, Dia. & Type

F12 RAFTER SPLICE ALONG SURFACE



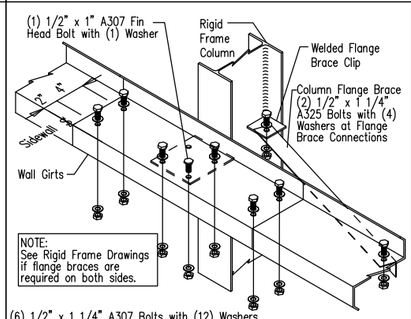
(6) 1/2" x 1 1/4" A307 Bolts with (12) Washers
(Typ.) (U.N.)

G2 ROOF PURLIN TO INTERIOR RIGID FRAME



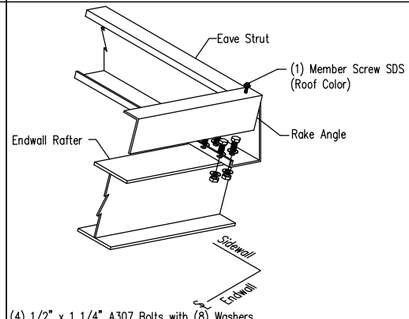
* Similar connection for Rafter.
The Fin Head Bolt must be installed into the first girt and clip of a lapped condition. The bolt/nut assembly must be wrench tight prior to the second lapped girt being installed.

H0 BYPASS LAPPED WALL GIRT



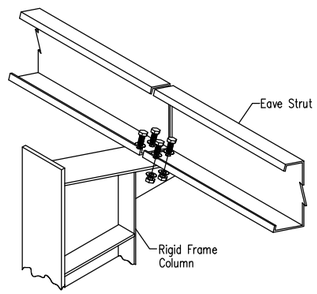
(6) 1/2" x 1 1/4" A307 Bolts with (12) Washers
(Typ.) (U.N.)

H2 WALL GIRT TO RIGID FRAME COLUMN



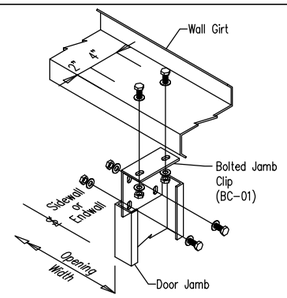
(4) 1/2" x 1 1/4" A307 Bolts with (8) Washers
(Typ.) (U.N.)

I8 EAVE STRUT TO ENDWALL RAFTER



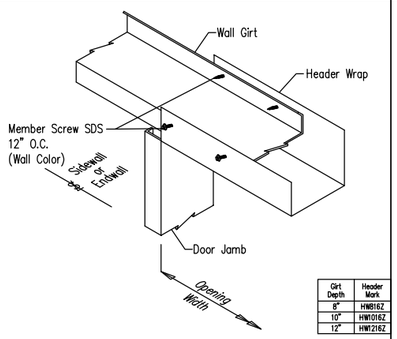
(4) 1/2" x 1 1/4" A307 Bolts with (8) Washers
(Typ.) (U.N.)

J2 LOWSIDE EAVE STRUT TO BYPASS RIGID FRAME



(4) 1/2" x 1 1/4" A307 Bolts with (8) Washers
(Typ.) (U.N.)

L6 DOOR JAMB TO WALL GIRT



Girt Depth	Header Mark
8"	HW816Z
10"	HW1016Z
12"	HW1216Z

M15 HEADER WRAP TO WALL GIRT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

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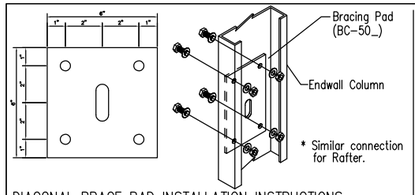
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE

JOB NO.: 58146 | ENG. BY: MZ | DATE: 4/30/24 | DWG. NO.: 10 OF 13 | ISSUE: C



DIAGONAL BRACE PAD INSTALLATION INSTRUCTIONS

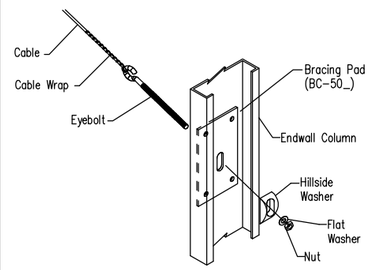
STEP 1 Line up brace pad with pre existing hole punches in the member.

STEP 2 Bolt the brace pad down using (4) 1/2" A307 bolts.

STEP 3 Field cut out the slot, using the brace pad slot as a template.

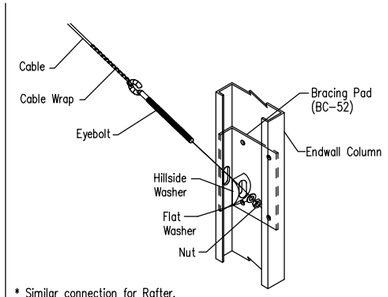
STEP 4 Install cable brace as normal, still leaving the brace pad installed along with the 1/2" A307 bolts.

Q2 DIAGONAL BRACE PAD TO WEB OF CEE COLUMN



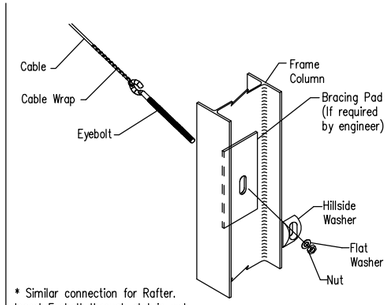
* Similar connection for Rafter.
Insert Eyebolt through slot in web.
Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO WEB OF CEE COLUMN



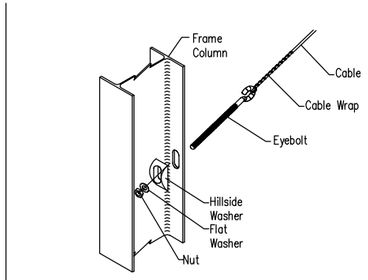
* Similar connection for Rafter.
Insert Eyebolt through slot in flange.
Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO FLANGE OF CEE COLUMN



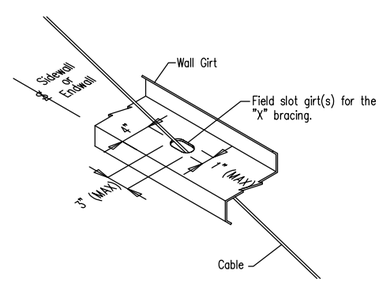
* Similar connection for Rafter.
Insert Eyebolt through slot in web.
Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO WEB OF FRAME COLUMN

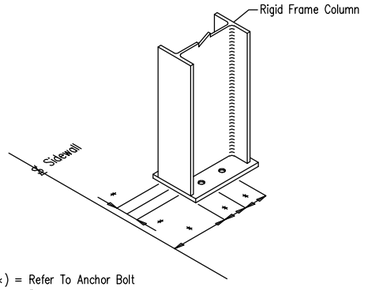


Insert Eyebolt through slot in flange.
Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO FLANGE OF FRAME COLUMN

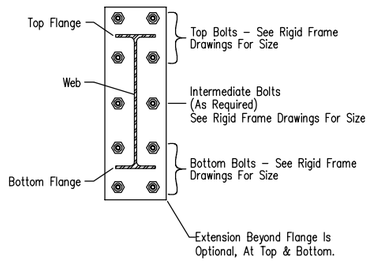


DIAGONAL CABLE BRACE AT FLUSH WALL GIRT

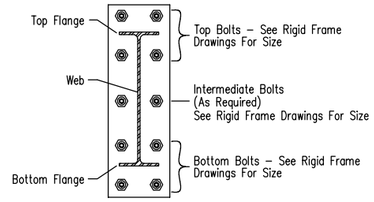


(*) = Refer To Anchor Bolt Plan

R2 ANCHOR BOLTS AT SIDEWALL COLUMNS



U2 BOLTS FOR RIGID FRAME RAFTER AT BUILDING PEAK



U3 BOLTS FOR RIGID FRAME RAFTER TO COLUMN CONNECTION

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

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DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
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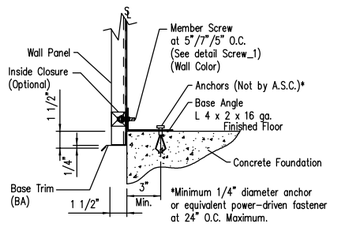
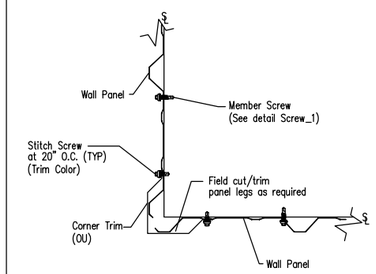
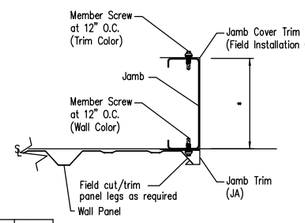
JOB NO.: 58146 | ENG. BY: MZ | DATE: 4/30/24 | DWG. NO.: 11 OF 13 | ISSUE: C



BASE ANGLE DETAIL WITH TRIM — DRAWING NO. TRIM_1

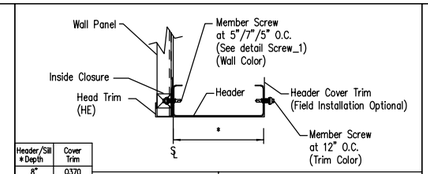


OUTSIDE CORNER DETAIL — DRAWING NO. TRIM_30

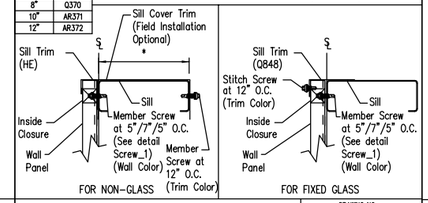


*Jamb Depth	Cover Trim
8"	Q370
10"	AR371
12"	AR372

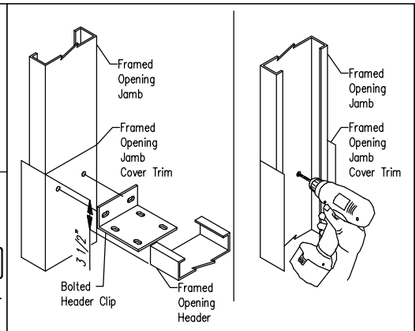
FRAMED OPENING JAMB TRIM DETAIL — DRAWING NO. TRIM_50



Header/Sill *Depth	Cover Trim
8"	Q370
10"	AR371
12"	AR372



FRAMED OPENING HEAD & SILL TRIM DETAILS — DRAWING NO. TRIM_51



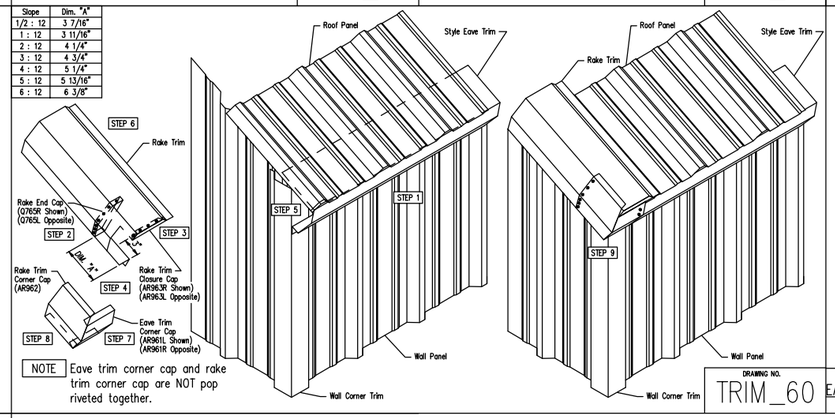
Install framed opening jamb cover trim on to jamb before installing header. Ream holes, using a 9/16" diameter bit, by using jamb holes as a guide.

COVER TRIM INSTALLATION INSTRUCTIONS — DRAWING NO. TRIM_52

STYLE EAVE CORNER TRIM INSTALLATION INSTRUCTIONS

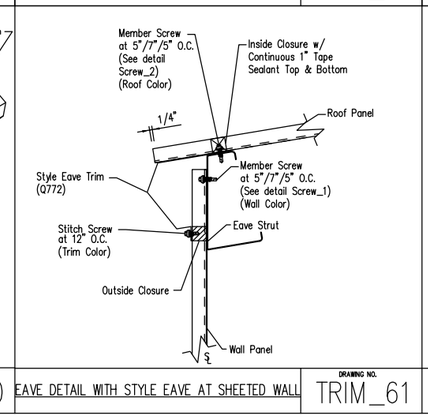
- STEP 1 Install style eave trim in between the roof panel and low eave member. Be sure the end of the style eave trim is flush with the wall corner trim.
- STEP 2 Install rake end cap, into rake trim using (8) pop rivets. Use chart to determine how far the rake end cap is positioned into the rake trim.
- STEP 3 Install rake trim closure cap, flush with the end of the rake trim using (5) pop rivets.
- STEP 4 Field cut/notch the face of the rake trim by 3". This is to prevent the rake trim from sticking out past the style eave trim upon final assembly.
- STEP 5 Field cut/notch the end of the roof panel back 1". This is to allow the rake trim closure cap from hitting the roof panel.
- STEP 6 Install rake trim. Be sure the end of the rake trim is flush with style eave trim.
- STEP 7 Install the eave trim corner cap to the style eave trim using (6) pop rivets.
- STEP 8 Install the rake trim corner cap to the rake trim using (7) pop rivets.
- STEP 9 Field cut/notch the bottom legs of the rake trim. Horizontal leg flush with the eave trim corner cap. Vertical leg flush with the wall corner trim.

Slope	Dim. "A"
1/2 : 12	3 7/16"
1 : 12	3 11/16"
2 : 12	4 1/4"
3 : 12	4 3/4"
4 : 12	5 1/4"
5 : 12	5 13/16"
6 : 12	6 3/8"

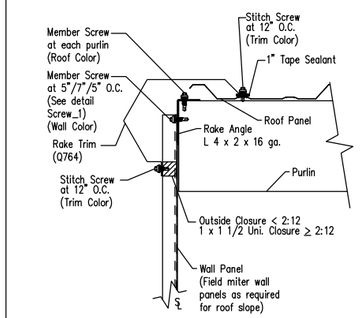


NOTE Eave trim corner cap and rake trim corner cap are NOT pop riveted together.

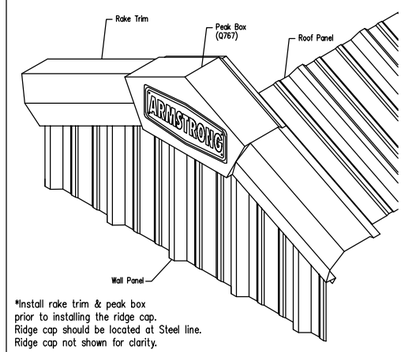
EAVE DETAIL WITH STYLE EAVE AT SHEETED WALL — DRAWING NO. TRIM_60



RAKE DETAIL AT SHEETED WALL — DRAWING NO. TRIM_61

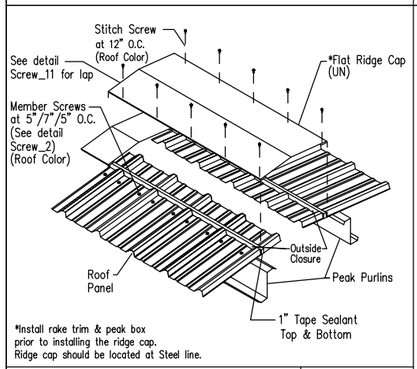


RAKE DETAIL AT SHEETED WALL — DRAWING NO. TRIM_66



*Install rake trim & peak box prior to installing the ridge cap. Ridge cap should be located at Steel line. Ridge cap not shown for clarity.

STANDARD PEAK BOX DETAIL — DRAWING NO. TRIM_100



*Install rake trim & peak box prior to installing the ridge cap. Ridge cap should be located at Steel line.

RIDGE DETAIL WITH FLAT RIDGE — DRAWING NO. TRIM_101

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

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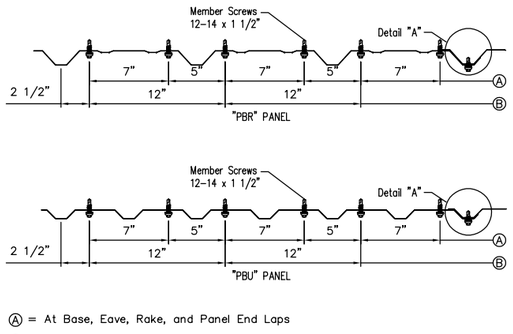
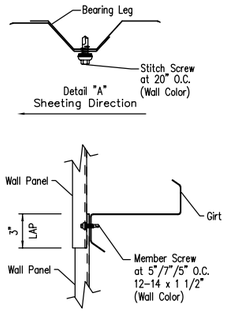
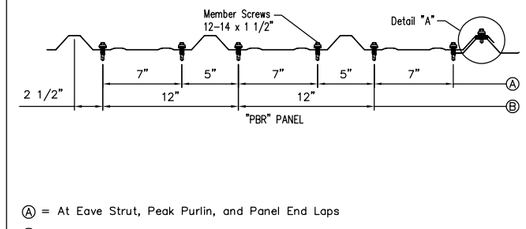
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE
JOB NO.: 58146	ENG. BY: MZ
DWG. NO.: 12 OF 13	DATE: 4/30/24
	ISSUE: C

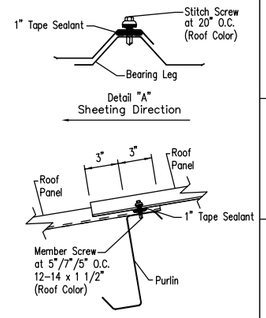




FASTENER LOCATION FOR WALL PANELS

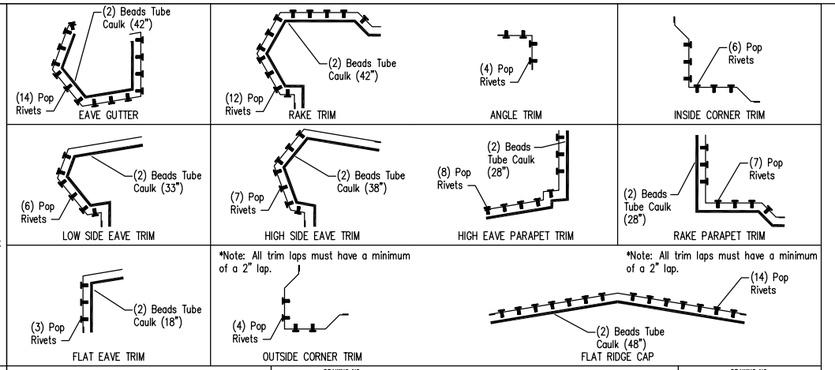
DRAWING NO. SCREW_1





FASTENER LOCATION FOR ROOF PANELS

DRAWING NO. SCREW_2

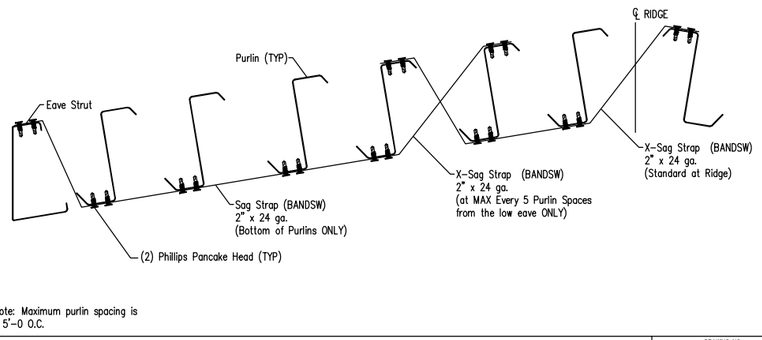


TRIM LAPS

DRAWING NO. SCREW_10

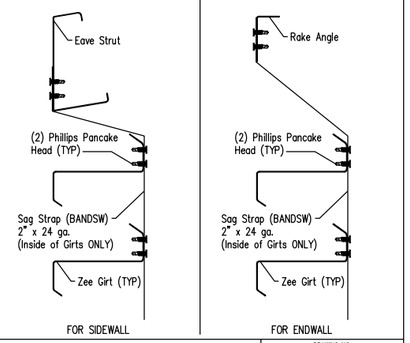
TRIM LAPS

DRAWING NO. SCREW_11



TYPICAL SAG STRAP AT GABLED ROOF

DRAWING NO. SCREW_15



TYPICAL SAG STRAP AT WALLS

DRAWING NO. SCREW_17

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/30/24	JW	SW	MZ
C	CONSTRUCTION	05.01.24	JW	SW	MZ

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PHONE: 800-345-4610
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DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	
END USER	
SCALE	NOT TO SCALE

JOB NO.:	ENG. BY:	DATE:
58146	MZ	4/30/24
	DWG. NO.: 13 OF 13	ISSUE: C